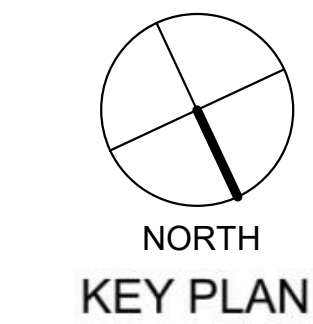


1 PROPOSED SECOND LEVEL FLOOR PLAN -
SECTION A
1/8" = 1'-0"



SHEET NOTES

1. ALL DIMENSIONS TO BE VERIFIED IN THE FIELD BY THE G.C. PRIOR TO ANY INSTALLATION OF NEW WORK
2. ALL EXPOSED GYPSUM BOARD IN LEASE AREA SHALL BE SEALED AND PRIMED ONLY
3. GENERAL CONTRACTOR SHALL BE REQUIRED TO VISIT PREMISE TO INSPECT EXISTING CONDITIONS AND VERIFY WORK TO BE PERFORMED.
4. ALL WORK PERFORMED SHALL BE IN STRICT COMPLIANCE WITH GOVERNING FEDERAL, STATE AND LOCAL BUILDING CODES REQUIREMENTS.
5. FIRE SEAL ALL PENETRATIONS THRU RATED WALLS, FLOORS, AND CEILINGS WITH APPROVED FIRE RATED CAULK AS DIRECTED BY LOCAL JURISDICTIONS.
6. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE REMOVAL OF TRASH AND DEBRIS FROM THE JOB SITE ON A DAILY BASIS.
7. REFER TO MECHANICAL, ELECTRICAL AND PLUMBING DRAWINGS FOR EQUIPMENT AND RELATED DETAILS.
8. REFER TO STRUCTURAL DRAWINGS FOR FOUNDATION DETAILS.
9. SEE STRUCTURAL DRAWINGS FOR ALL SLAB ON GRADE DETAILS AND SPECIFICATIONS
10. FOR LOCATION OF MOP SINKS AND FLOOR DRAINS SEE PLUMBING DRAWINGS.
11. ALL FRAMING DIMENSIONS ARE ROUGH FRAMING, AND DO NOT INCLUDE ANY FINISH MATERIALS UNLESS OTHERWISE NOTED.
12. ALL WALLS SHALL BE OF 6" OR 4" METAL STUDS UNLESS OTHERWISE NOTED.
13. APPLY BORIC ACID ON TOP OF THE MUD SILL OR BOTTOM PLATE, PRIOR TO CLOSING ALL WALLS.
14. PROVIDE BLOCKING FOR RESTROOM ACCESSORIES, DOOR STOPS, EXP. JOINTS, CABINETS, ETC.
15. INSTALL OCCUPANT LOAD SIGN PER LOCAL FIRE MARSHALL. COORDINATE LOCATION WITH OWNER/FIRE MARSHALL.
16. PROVIDE MASTIC OR APPROVED SEALANT UNDER ALL EXT. PLATES
17. SEE DRAWING A601 FOR DOOR TYPES, SCHEDULE AND HARDWARE SCHEDULE.
18. SEE MECHANICAL DRAWING FOR NEW COOLER AND COLD DOCK SPECIFICATIONS.

WALL TYPES

1. EXTERIOR - BRICK VENEER TO 4' A.F.F. / 26 GAUGE PBR MTL. PANELS STARTING AT 4' A.F.F. ON EXTERIOR SHEATHING ON WALL GIRTS W/ R-20 BATT INSULATION AND WATERPROOF COVER
2. EXTERIOR - 26 GAUGE PBR MTL. PANELS ON EXTERIOR SHEATHING ON WALL GIRTS W/ R-20 BATT INSULATION AND WATERPROOF COVER
3. INTERIOR - 3 5/8" MTL. STUD AT 16" O.C. ON 1 LAYER 1/2" GWB (BOTH SIDES) W/ SOUND ATTENUATION INSULATION
4. INTERIOR - 25 GAUGE 6" MTL STUD AT 24" O.C. ON 1 LAYER 5/8" GWB (BOTH SIDES) W/ SOUND ATTENUATION INSULATION



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Drawing Revisions:		
A	ADDENDUM #3	12.13.22

Drawing Date:	09/20/2022
Project Number:	220001
Drawn By:	AP
Checked By:	GLP

GEORGIA MOUNTAIN FOOD BANK
1642 CALVARY INDUSTRIAL DRIVE SW
GAINESVILLE, GA 30607

Drawing Description:
PROPOSED
SECOND LEVEL
FLOOR PLAN

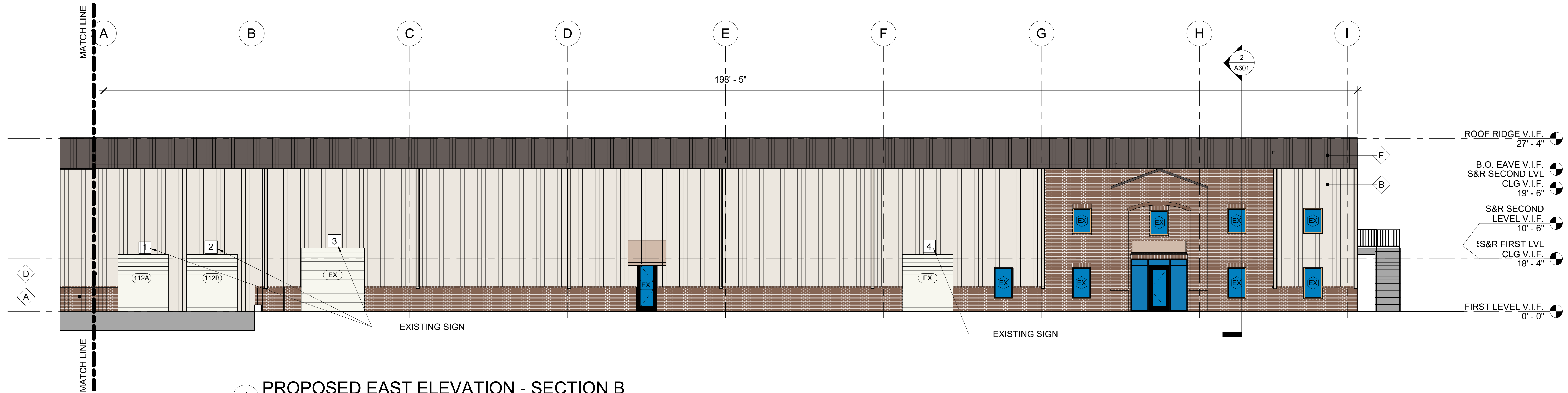
A105-A

EXTERIOR FINISHES SCHEDULE

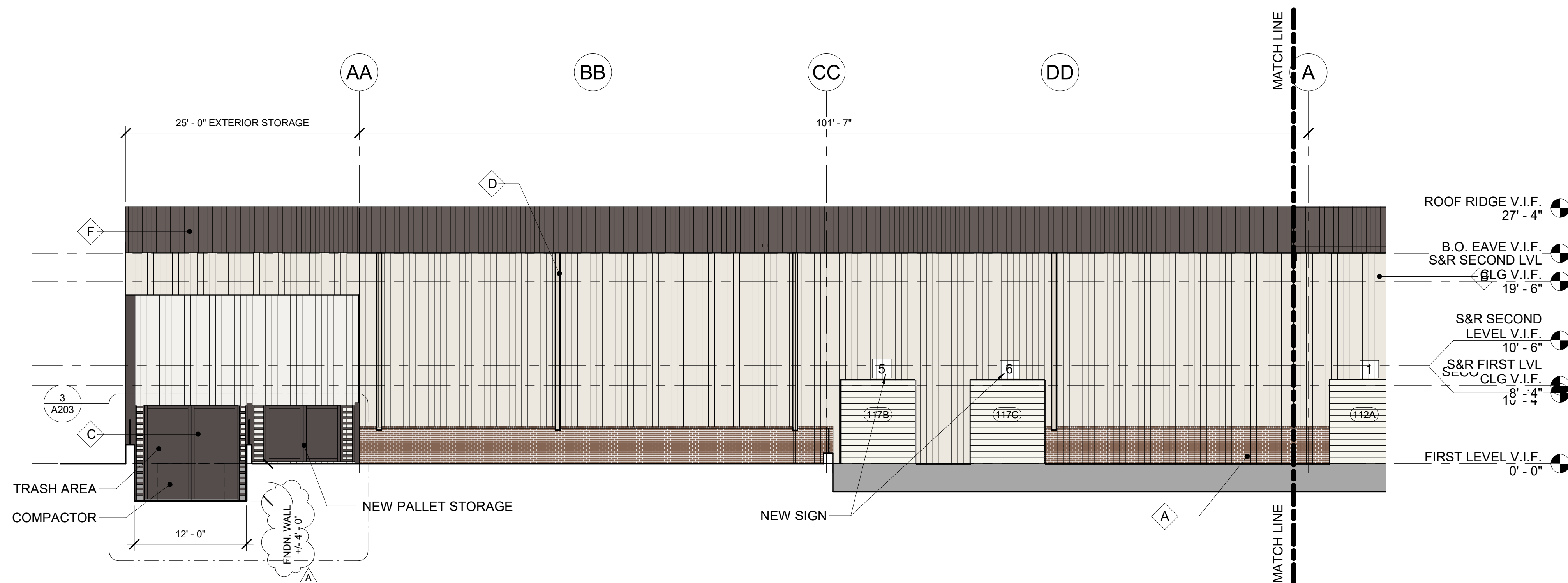
SYMBOL	MATERIAL	DESCRIPTION
A	BRICK	MFR/TYPE: CHEROKEE BRICK, SHELMAN'S BLUFF
B	METAL PANELING	26 GAUGE PBR PANELS, COLOR TO MATCH EXISTING
C	METAL GATE	20 GAUGE CORRUGATED METAL DECK WELDED TO FRAME
D	SEAMLESS GUTTER AND DOWNSPOUT	6" SEAMLESS GUTTER & DOWNSPOUT SYSTEM, COLOR TO MATCH EXISTING
E	METAL FENCE	
F	ROOF METAL PANELING	24 GAUGE PBR PANELS, COLOR TO MATCH EXISTING

NOTES:

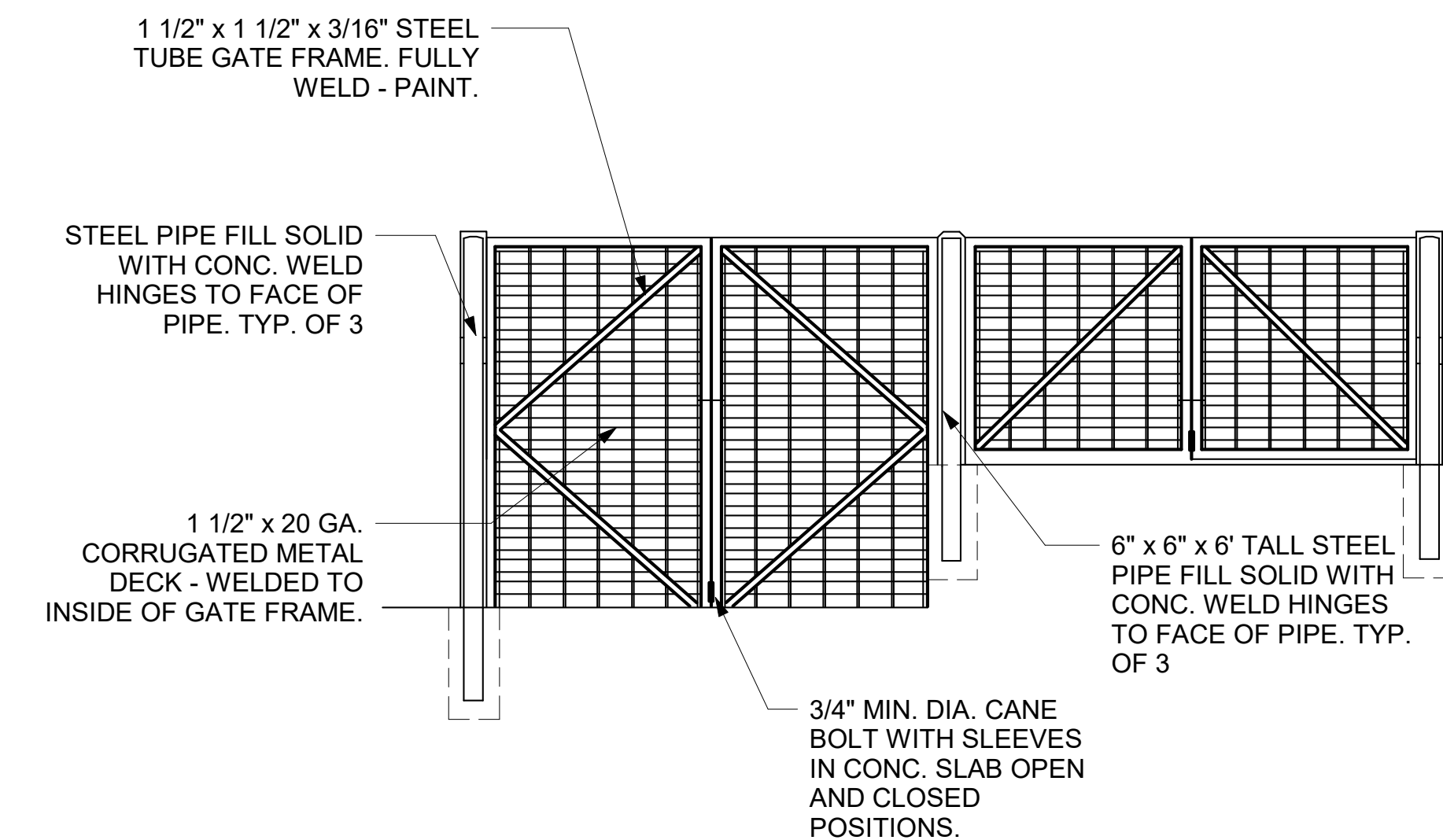
- PREPARE ALL SURFACES TO BE PAINTED AS PER MANUFACTURERS
RECOMMENDATIONS
- TEST S.W. COLORS TO COLORS ON SITE - ADJUST SW COLORS AS REQ'D TO
MATCH
- ALL COLORS AND FINISHES SHALL BE PRESENTED TO OWNER FOR APPROVAL
PRIOR TO INSTALLATION
- BUILDING DOOR NUMBER SIGNAGE TO MATCH EXISTING.



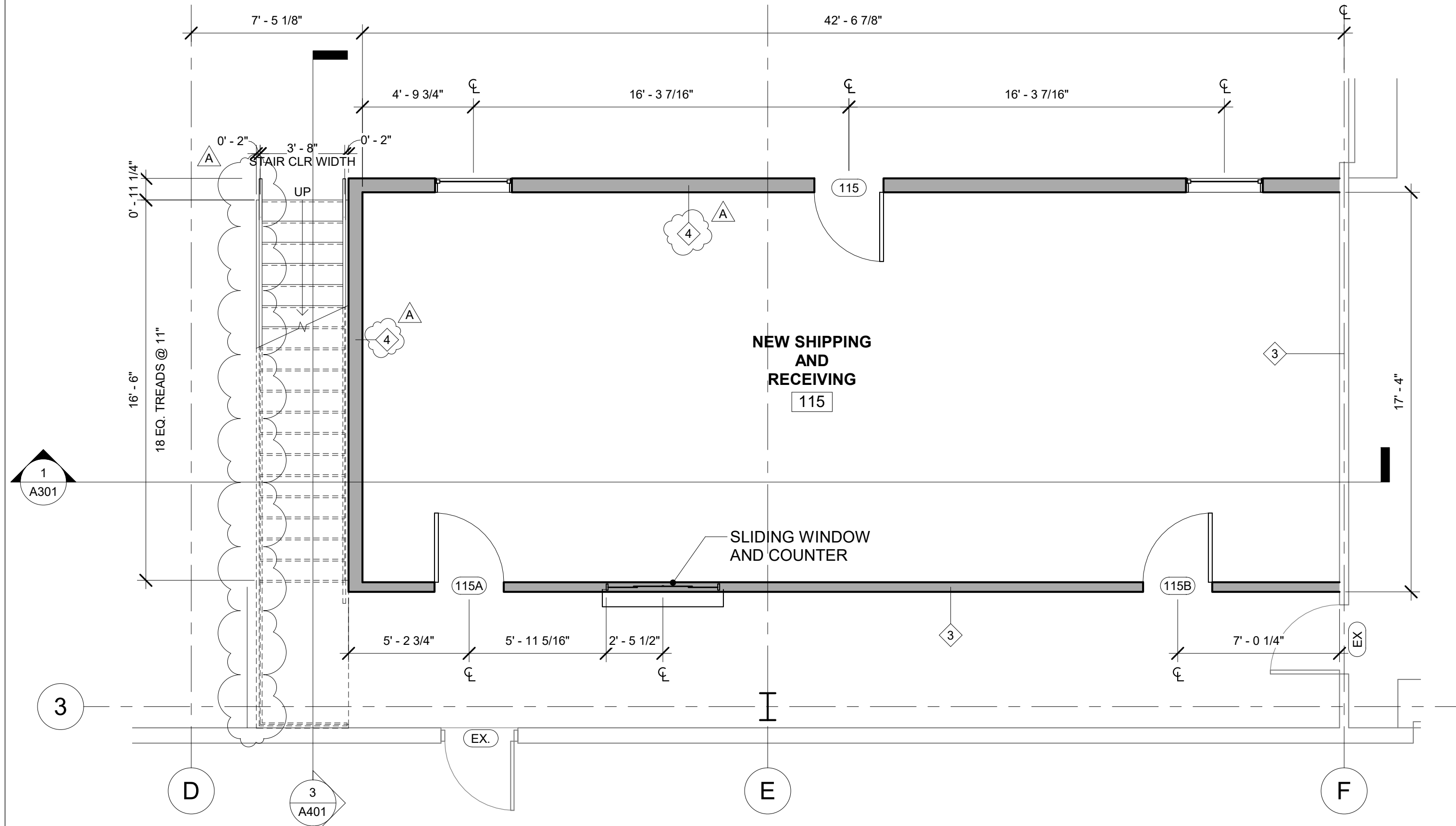
1 PROPOSED EAST ELEVATION - SECTION B
1/8" = 1'-0"



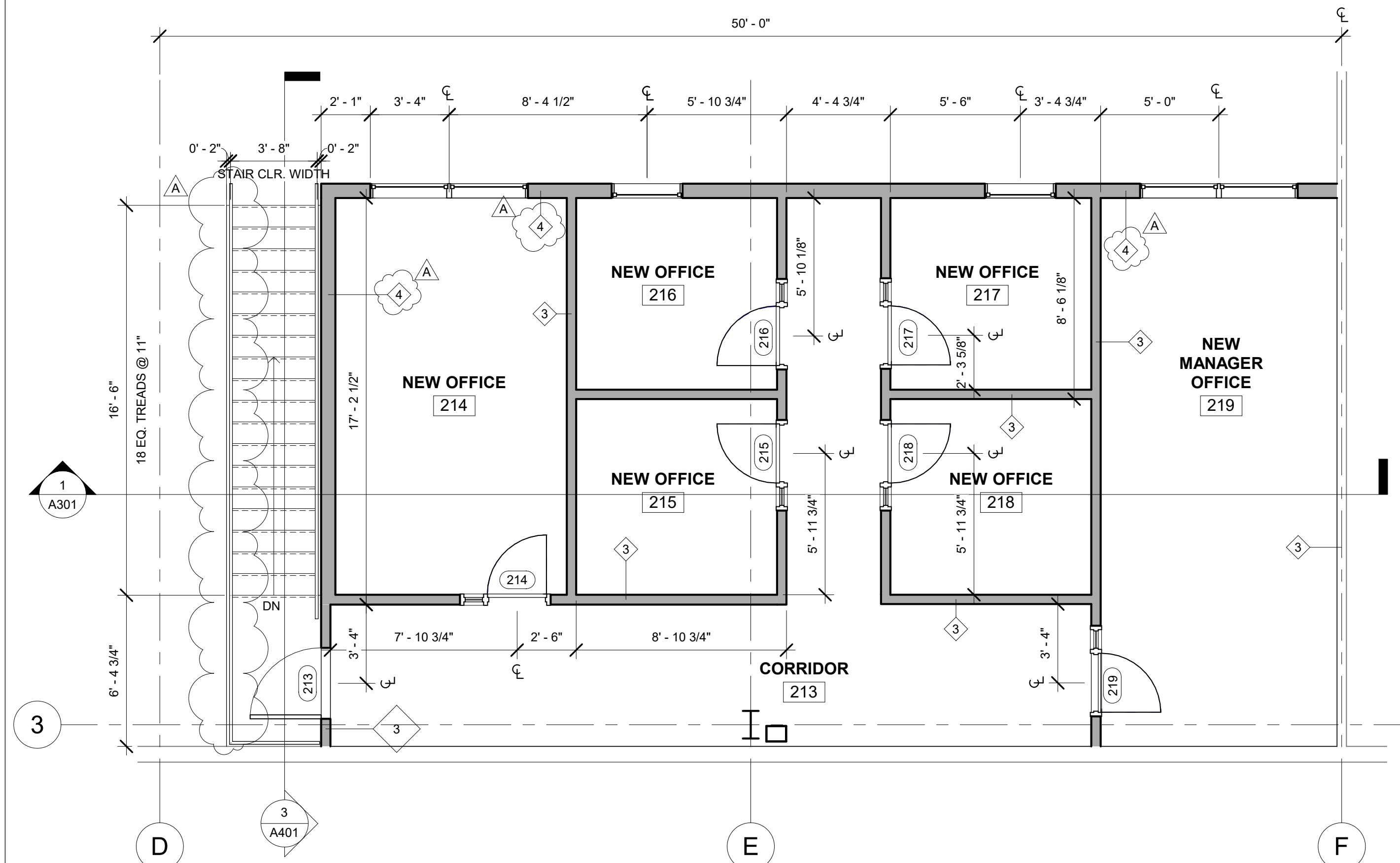
2 PROPOSED EAST ELEVATION - SECTION A
1/8" = 1'-0"



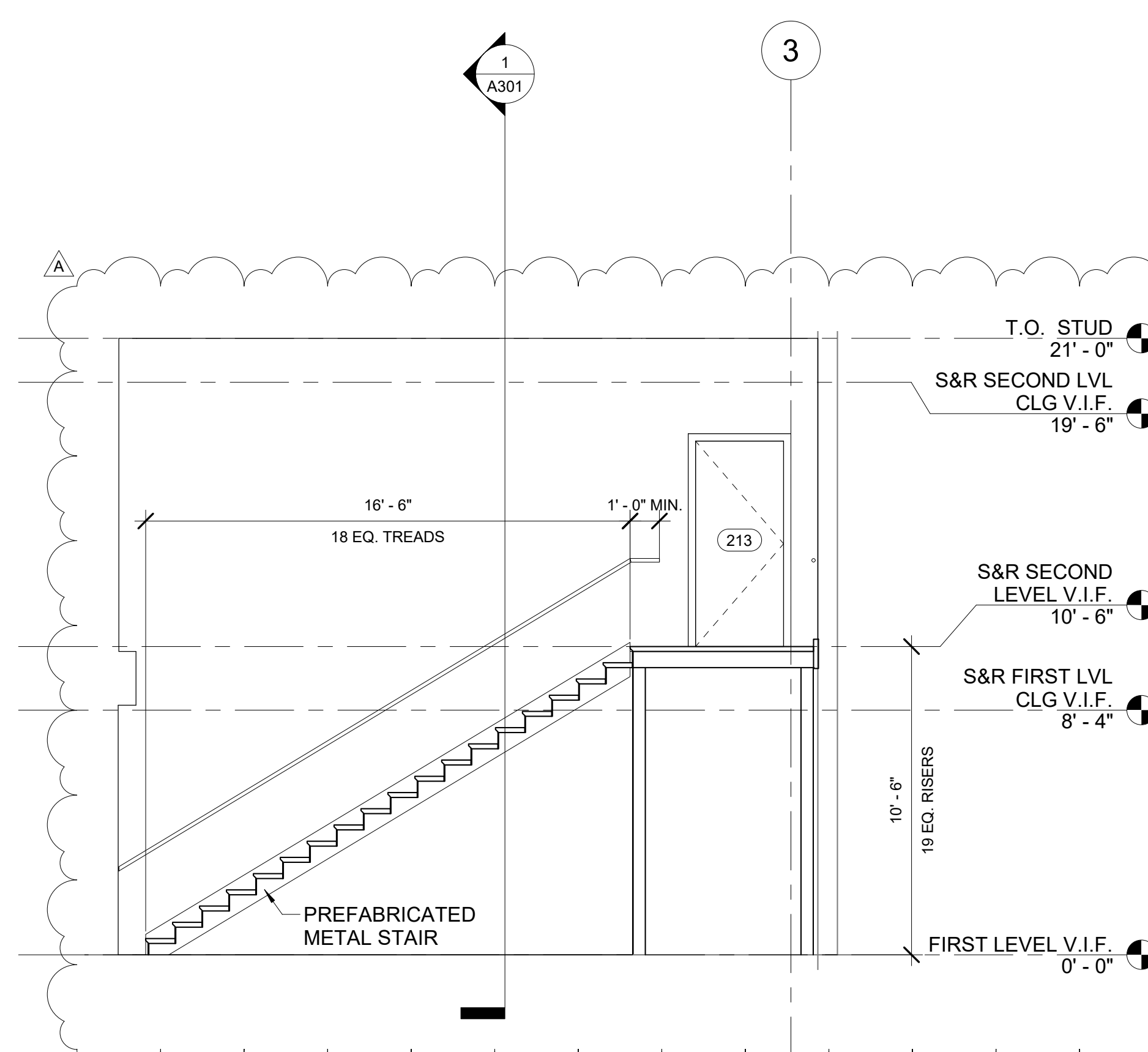
3 EXTERIOR STORAGE GATE ELEVATION
1/4" = 1'-0"



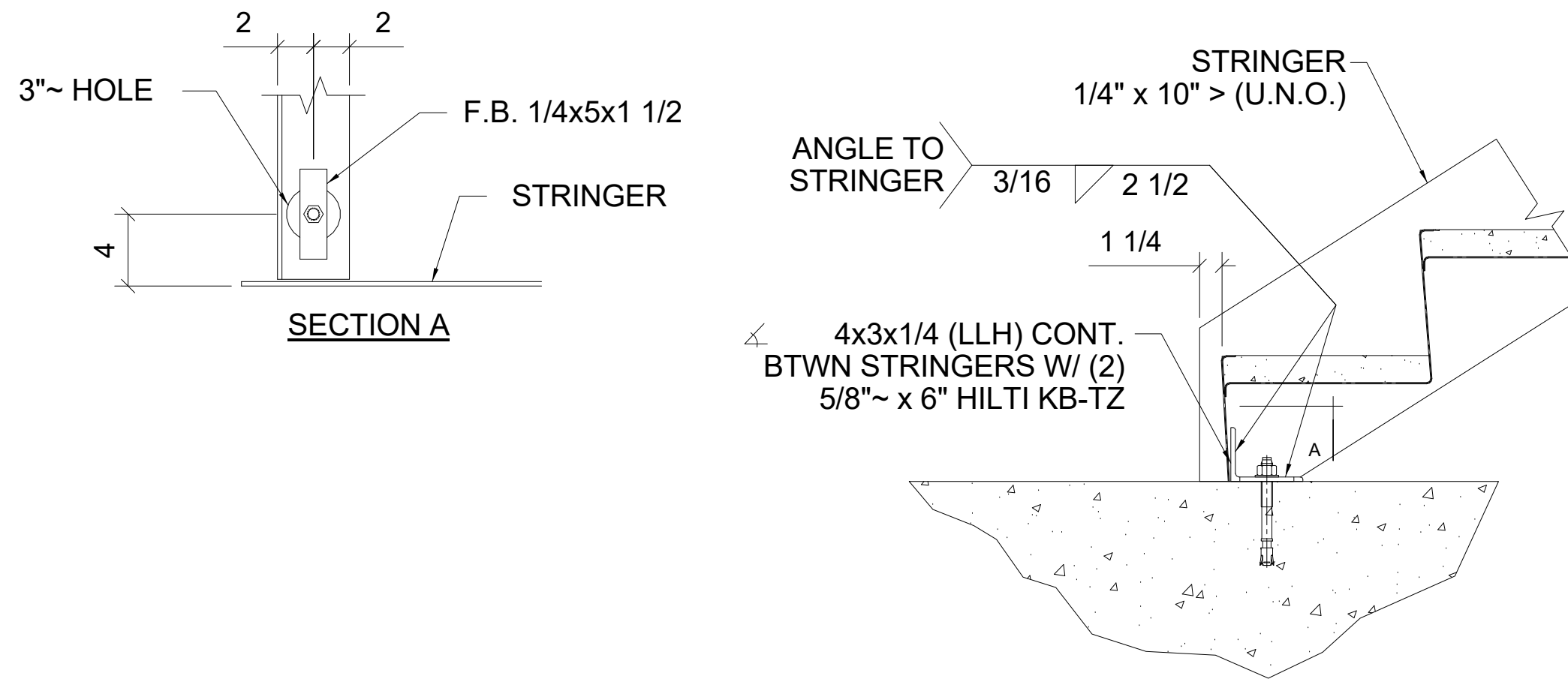
1
ENLARGED FIRST LEVEL PLAN - SHIPPING AND RECEIVING
1/4" = 1'-0"



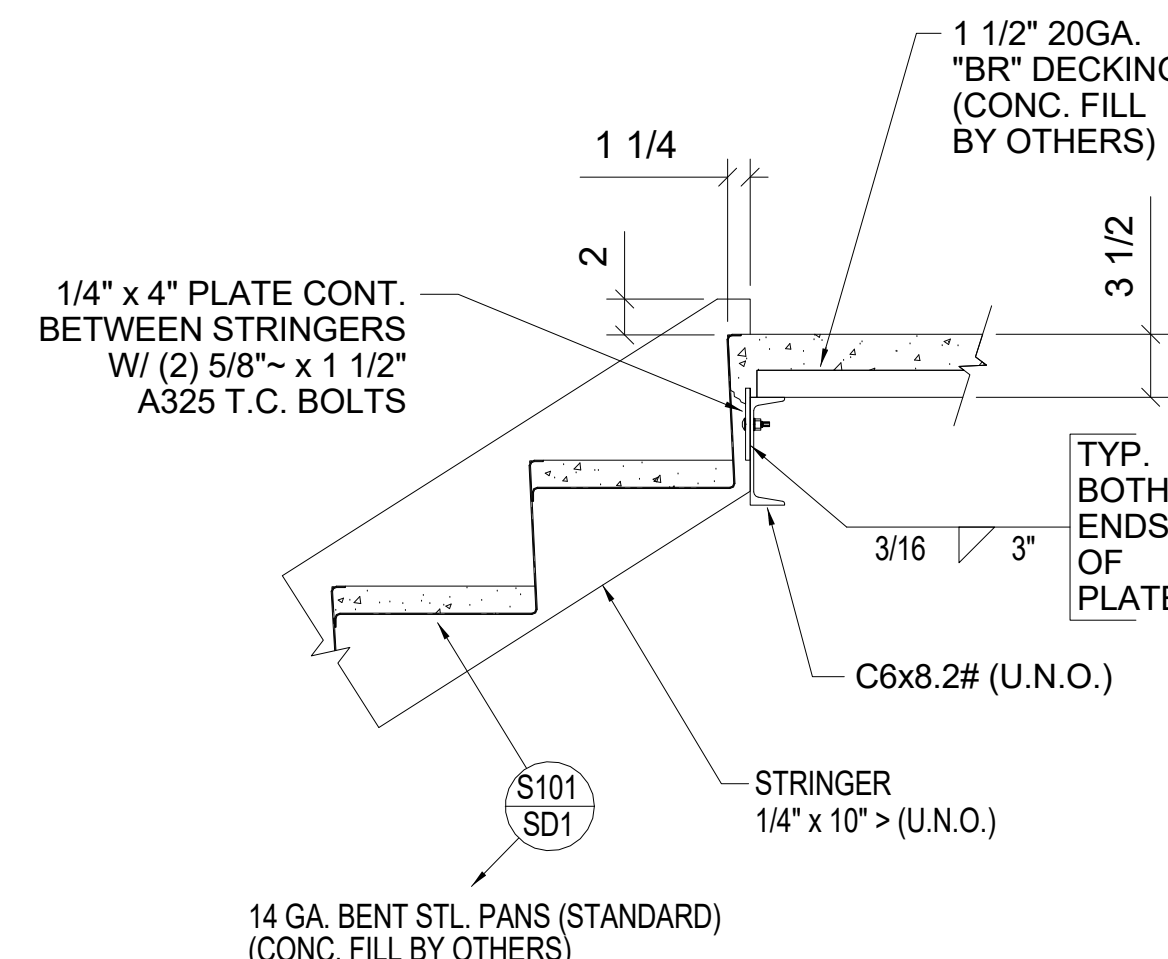
2
ENLARGED SECOND LEVEL PLAN - SHIPPING AND RECEIVING
1/4" = 1'-0"



3
STAIR SECTION - SHIPPING AND RECEIVING
1/4" = 1'-0"



5
STAIR DETAIL - CONNECTION TO SLAB
1 1/2" = 1'-0"



6
STAIR DETAIL - CONNECTION TO STEEL LANDING
N.T.S.

KEY PLAN

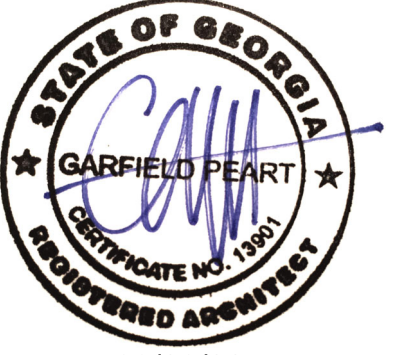


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- SEE DRAWING A601 FOR DOOR TYPES, SCHEDULE INTERIOR FINISH PLAN AND HARDWARE SCHEDULE.

WALL TYPES

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Drawing Revisions:

A ADDENDUM #3 12.13.22

Drawing Date: 09/20/2022

Project Number: 220001

Drawn By: AP

Checked By: GLP

GEORGIA MOUNTAIN FOOD BANK
1642 CALVARY INDUSTRIAL DRIVE SW
GAINESVILLE, GA 30507

Drawing Description:

ENLARGED PLANS

A401-A

STRUCTURAL GENERAL NOTES

FOUNDATIONS

1.

ALL FOOTINGS SHALL BEAR ON NATURAL UNDISTURBED SOIL HAVING A MINIMUM SAFE BEARING CAPACITY OF 2,000 PSF, OR ENGINEERED CONTROLLED COMPACTED FILL AS REQUIRED HAVING ALLOWABLE BEARING CAPACITY OF 2,000 PSF. THERE WAS NO GEOTECHNICAL REPORT DURING THE DESIGN OF THIS BUILDING. IT'S ABSOLUTELY NECESSARY THAT A GEOTECHNICAL ENGINEER IS RETAINED DURING THE CONSTRUCTION PHASE TO ANALYZED THE SOIL WITHIN THE BUILDING FOOTPRINT AND PROVIDE THE STRUCTURAL ENGINEER OF THE PROJECT WITH THE SOIL PROFILE INDICATING THE TYPE OF SOIL ENCOUNTERED AT THE SITE, THE DENSITY, THE SOIL FRICTION ANGLE, THE ACTIVE, PASSIVE AND STATIC SOIL PRESSURES. ALSO A GEOTECHNICAL ENGINEER SHALL VERIFY THE SOIL COMPACTION BEFORE INSTALLATION OF ANY FOUNDATION SYSTEM.
2.

ISOLATED FOOTING SHALL BE LOWERED AND PIERS SHALL BE ADDED IN HEIGHT, ALL AS APPROVED, WHERE SOIL OF THE SPECIFIED BEARING CAPACITY IS FOUND AT A LOWER ELEVATION THAN SHOWN ON THE PLANS.
3.

BACKFILLING AGAINST FOUNDATION WALLS SHALL NOT BE DONE UNTIL WALL HAS BEEN CURED TO A SUFFICIENT STRENGTH (7 DAYS MINIMUM) AND WALLS ARE PROPERLY STORED OR BRACED FOR ADDITIONAL 7 DAYS.
4.

THE CONTRACTOR SHALL SAFEGUARD AND PROTECT ALL EXCAVATIONS, AND EXCAVATIONS SHALL BE KEPT FREE OF WATER.
5.

EDGES OF FOOTINGS SHALL NOT BE PLACED AT A SLOPE GREATER THAN 1 (VERTICAL) TO 2 (HORIZONTAL) WITH RESPECT TO ANY ADJACENT FOOTINGS U.N.O.
6.

THE FOUNDATION CONTRACTOR SHALL REFER TO THE MECHANICAL, CIVIL AND ELECTRICAL CONTRACT DRAWINGS FOR ALL LOCATIONS OF TRENCHES, PITTS, CONDUITS, UNDERFLOOR AND PERIMETER DRAINAGE SYSTEMS ETC.

CONCRETE

1.

ALL CONCRETE WORK SHALL CONFORM WITH THE REQUIREMENTS OF THE LATEST EDITION OF THE AMERICAN CONCRETE INSTITUTE A.C.I. 318 AND OTHER A.C.I. CONCRETE CODES.
2.

ALL CONCRETE USED FOR THIS PROJECT SHALL BE AS FOLLOWS:

a.

ALL CONCRETE FOR FOOTINGS AND BUILDING SLAB ON GRADE SHALL BE STONE CONCRETE HAVING A MINIMUM COMPRESSIVE STRENGTH OF 3,000 PSI AT 28 DAYS.

b.

ALL CONCRETE FOR EXTERIOR WORK LIKE THE RAMP, SIDEWALKS, ETC. SHALL BE STONE CONCRETE WITH 4,000 PSI MINIMUM COMPRESSIVE STRENGTH AT 28 DAYS.
3.

REPRESENTATIVE TEST CYLINDERS WILL BE TAKEN FROM THE CONCRETE PLACED EACH DAY IN ACCORDANCE WITH A.C.I. CODE AND SPECIFICATIONS IN THIS PROJECT.
4.

REINFORCING STEEL SHALL BE DEFORMED BARS OF INTERMEDIATE GRADE NEW BILLET STEEL CONFORMING TO CURRENT REQUIREMENTS OF A.S.T.M. A-615 GRADE 60. DESIGN STRESS = 24,000 PSI. LAP BARS, 40 BAR DIAM. UNLESS OTHERWISE SHOWN.
5.

WELDED WIRE FABRIC SHALL HAVE A MINIMUM ULTIMATE STRENGTH OF 70,000 PSI AND SHALL CONFORM TO A.S.T.M. A-185.
8.

EXCAVATIONS SHALL BE KEPT FREE OF WATER. NO CONCRETE SHALL BE PLACED IN WATER.
9.

ALL SLABS ON GRADE SHALL HAVE THICKENINGS AT DEPRESSIONS, OPENINGS, ETC. AS REQUIRED OR AS SHOWN HERIN OR ON ARCHITECTURAL DRAWINGS.
10.

ALL MESH SHALL BE SPLICED SO THAT THE OVERLAP BETWEEN OUTERMOST CROSS WIRES OF EACH SHEET IS NOT LESS THAN THE SPACING OF THE CROSS WIRES PLUS TWO INCHES, UNLESS OTHERWISE SHOWN ON DRAWINGS.
11.

FOR ALL SLABS ON GRADE WHERE NOT OTHERWISE SPECIFIED, USE STYLE 4 x 4 - W4 x W4 W.W.F.
12.

ALL SLABS ON GRADE SHALL BE TROWELLED IN AT EVERY COLUMN LINE AND ELSEWHERE SHOWN ON FOUNDATION PLAN. SEE TYPICAL SLAB ON GRADE DETAIL FOR CONTROL JOINTS.

STRUCTURAL AND MISCELLANEOUS STEEL

1.

ALL STRUCTURAL AND MISCELLANEOUS STEEL SHALL CONFORM TO THE NINTH EDITION OF THE AISC "SPECIFICATION FOR THE DESIGN, FABRICATION AND ERECTION OF STRUCTURAL STEEL FOR BUILDINGS" AND ALL ITS SUPPLEMENTS, AND TO THE AISC "CODE OF STANDARD PRACTICE FOR STEEL BUILDINGS AND BRIDGES".
2.

ALL STRUCTURAL AND MISCELLANEOUS STEEL SHALL CONFORM TO ASTM A992 HAVING A MINIMUM YIELD STRENGTH OF 50,000 PSI.
3.

STEEL PIPES SHALL CONFORM TO ASTM A53, TYPE E OR S GRADE B.
4.

ALL WELDED CONNECTIONS SHALL BE DONE WITH E70XX ELECTRODES. ALL BOLTED CONNECTIONS SHALL USE 3/4" ASTM A325N HIGH STRENGTH BOLTS UNLESS NOTED OTHERWISE. ALL BEAM CONNECTIONS SHALL DEVELOP THE FULL UNIFORM LOAD CAPACITY THE MEMBER CAN CARRY WITH DUE CONSIDERATION TO CONCENTRATED LOADS AT THE ENDS OF THE MEMBER. NO CONNECTION SHALL BE MADE USING LESS THAN TWO BOLTS. ALL ANCHOR BOLTS SHALL BE ASTM A-307.
5.

SHOP AND FIELD WELDS SHALL BE MADE BY APPROVED CERTIFIED WELDERS AND SHALL CONFORM TO THE AMERICAN WELDING SOCIETY CODE FOR BUILDINGS AWS D11.1. WELDS SHALL DEVELOP THE FULL STRENGTH OF MATERIALS BEING WELDED, UNLESS OTHERWISE NOTED.
6.

NO OPENINGS IN BEAMS SHALL BE PERMITTED WITHOUT THE WRITTEN PERMISSION OF THE ENGINEER.
7.

THE USE OF A GAS-CUTTING TORCH IN THE FIELD FOR CUTTING HOLES OR FOR CORRECTING FABRICATION ERRORS WILL NOT BE PERMITTED ON NEW STRUCTURAL FRAMING MEMBERS EXCEPT WITH THE WRITTEN APPROVAL OF THE ENGINEER FOR EACH SPECIFIC CONDITION.

7.

THE USE OF A GAS-CUTTING TORCH IN THE FIELD FOR CUTTING HOLES OR FOR CORRECTING FABRICATION ERRORS WILL NOT BE PERMITTED ON NEW STRUCTURAL FRAMING MEMBERS EXCEPT WITH THE WRITTEN APPROVAL OF THE ENGINEER FOR EACH SPECIFIC CONDITION.
8.

ALL STRUCTURAL STEEL SHALL BE SHOP PAINTED WITH AN APPROVED CORROSION RESISTANT PRIMER. ALL STEEL SHALL BE PAINTED IN STRICT ACCORDANCE WITH THE AISC SPECIFICATIONS AND MANUFACTURER'S RECOMMENDATIONS. PRIOR TO PAINTING, ALL STEEL SURFACES SHALL BE PREPARED IN ACCORDANCE WITH SSPC. REFER TO SPECIFICATIONS SECTION 05120.
9.

ERECTED STRUCTURAL STEEL SHALL BE INSPECTED BY QUALIFIED INSPECTORS AS STIPULATED IN THE SPECIFICATIONS. FIELD INSPECTION REPORTS TO BE FILED WITH THE STRUCTURAL ENGINEER WITHIN 5 DAYS OF TIME OF ACTUAL INSPECTION. INSPECTORS MUST BE NOTIFIED OF ALL PHASES OF CONSTRUCTION AND WELDING BY GENERAL CONTRACTOR.

METAL DECKS

1.

ALL METAL DECKING SHALL BE DESIGNED, FABRICATED AND ERECTED IN ACCORDANCE WITH THE LATEST EDITION OF THE "SPECIFICATION OF THE STEEL DECK INSTITUTE".
2.

ALL METAL DECK SHALL BE GALVANIZED. ALL GALVANIZING SHALL CONFORM TO THE REQUIREMENTS OF ASTM A525, G60.
3.

METAL DECK SHALL BE AS FOLLOWS:

A:

NON-COMPOSITE FLOOR DECK SHALL BE SIMILAR TO 1" 22 GAUGE STEEL FORM DECK EQUIVALENT TO VULCRAFT 1.0C22 CSV CONFORM DECK SHALL HAVE THE FOLLOWING MINIMUM PROPERTIES PER FOOT WIDTH:

Ip = 0.073 IN4/FT.
In = 0.073 IN4/FT.
Sp = 0.130 IN3/FT.
Sn = 0.134 IN3/FT.
Fy = 60 Ksi

4.

ALL METAL DECKS HAVE BEEN DESIGNED TO BE CONTINUOUS OVER 2 SPANS MINIMUM, AND SHALL BEAR AT LEAST 2" ON STEEL SUPPORTS.
5.

DECK SHALL BE WELDED TO SUPPORTING STEEL AT ENDS OF UNITS AND ALL INTERMEDIATE SUPPORTS IN ACCORDANCE WITH MANUFACTURER'S RECOMMENDATIONS (UNLESS NOTED OTHERWISE ON PLAN).
6.

STANDARD CLOSURE SHALL BE PROVIDED AS REQUIRED.
7.

SHOP DRAWINGS SHALL INDICATE ALL INFORMATION NECESSARY FOR A COMPLETE FABRICATION AND ERECTION OF THE METAL DECK.
8.

PROVIDE STEEL HEADER FRAMES FOR SUPPORT OF METAL DECKING FOR ALL OPENINGS GREATER THAN 10" SQUARE.
9.

PROVIDE 12" x 16 GAUGE CONTINUOUS GALVANIZED STEEL BUTT STRIP WHERE DECK CHANGES DIRECTION.
10.

PROVIDE STEEL LEDGER ANGLE 2 1/2" x 2 1/2" x 1/4" AT STEEL COLUMNS AS REQUIRED FOR SUPPORT OF METAL DECKING.

LIGHT GAUGE METAL STUDS

1.

THE ENGINEERING OF ALL STRUCTURAL LIGHT GAUGE METAL STUD WALL INCLUDING THEIR CONNECTIONS AND LINTELS SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR.
2.

ALL LIGHT GAGE METAL INFORMATION SHOWN ON THE DRAWINGS ARE BASED ON DIETRICH INDUSTRIES, INC. PRODUCTS.
3.

ALL LIGHT GAGE STUDS MUST BE MANUFACTURED WITH CORROSION RESISTANT STEEL OR GALVANIZED IN ACCORDANCE WITH ASTM A525.
4.

ALL MEMBERS SHALL MEET THE FOLLOWING REQUIREMENTS:

ALL 54 mil & HEAVIER MEMBERS - ASTM A446 GRADE D (50 KSI MIN.)

ALL 43 mil & LIGHTER MEMBERS - ASTM A446 GRADE A (33 KSI MIN.)
5.

ALL LOAD BEARING STUDS MUST BE SEATED TIGHT, WITHIN 1/16" AGAINST TRACK, WEB. ALSO, STUD MUST BE SCREW ATTACHED OR WELDED TO EACH TRACK LEG.
6.

ERECT FRAMING AND PANELS PLUMB, LEVEL AND SQUARE IN STRICT ACCORDANCE WITH THE APPROVED SHOP DRAWINGS.
7.

HANDLING AND LIFTING OF PREFABRICATED PANELS SHALL BE DONE IN A MANNER AS TO NOT CAUSE DISTORTION IN ANY MEMBER.
8.

TRACK SHALL BE SECURELY ANCHORED TO THE SUPPORTING STRUCTURE AS SHOWN ON THE ERECTION DRAWINGS. CONCRETE ANCHORS SHALL BE INSTALLED AFTER FULL COMPRESSIVE STRENGTH HAS BEEN ACHIEVED.
9.

ALL TRACK BUTT JOINTS, ABUTTING PIECES OF TRACK SHALL BE SECURELY ANCHORED TO A COMMON STRUCTURAL ELEMENT, OR THEY SHALL BE BUTT-WELDED OR SPLICED TOGETHER.
10.

STUDS SHALL BE PLUMBED, ALIGNED AND SECURELY ATTACHED TO THE FLANGE OR WEBS OF BOTH UPPER AND LOWER TRACKS.
11.

JACK STUDS OR CRIPPLES SHALL BE INSTALLED BELOW WINDOW SILLS, ABOVE WINDOW AND DOOR HEADS, AT FREE STANDING STAIR RAILS AND ELSEWHERE TO FURNISH SUPPORT AND SHALL BE SECURELY ATTACHED TO SUPPORTING MEMBERS.
12.

FRAMED OPENINGS SHALL INCLUDE HEADERS AND SUPPORTING STUDS AS SHOWN ON SHOP DRAWINGS.
13.

TEMPORARY BRACING SHALL BE PROVIDED UNTIL ERECTION IS COMPLETED.
14.

PROVIDE HORIZONTAL BRIDGING FOR ALL STUDS AT 48 INCH. ON CENTER, OR PER MANUFACTURER'S RECOMMENDATION.

DESIGN CODE

2018 EDITION OF THE INTERNATIONAL BUILDING CODE.

FLOOR LIVE LOAD

SLAB ON GRADE = 150 PSF
MECHANICAL ROOM = 150 PSF, SECOND FLOOR = 70 PSF.

ROOF LIVE LOAD

30 PSF

ROOF SNOW LOAD

GROUND SNOW LOAD, Pg = 25 PSF
FLAT ROOF SNOW LOAD, Pf = 18 PSF
SNOW EXPOSURE FACTOR, Ce = 1.0
SNOW LOAD IMPORTANCE FACTOR, Is = 1.0
THERMAL FACTOR, Ct = 1.0

WIND LOAD

BASIC WIND SPEED = 120 MPH
WIND LOAD IMPORTANCE FACTOR, Iw = 1.0
WIND EXPOSURE FACTOR = C
INTERNAL PRESSURE COEFFICIENTS = +0.18, -0.18
COMPONENTS AND CLADDING = 25 PSF

EARTHQUAKE DESIGN DATA

SEISMIC IMPORTANCE FACTOR, Ie = 1.0
OCCUPANCY CATEGORY = GROUP II
MAPPED SPECTRAL RESPONSE ACCELERATIONS Ss = 0.184g, S1 = 0.063g
SITE CLASS = D
SPECTRAL RESPONSE COEFFICIENTS, Sds = 0.196, Sd1 = 0.101
SEISMIC DESIGN CATEGORY = B
BASIC SEISMIC FORCE RESISTING SYSTEM = STRUCTURAL STEEL SYSTEM NOT SPECIFICALLY DETAILED FOR SEISMIC RESISTANCE
DESIGN BASE SHEAR = 30 KIPS
SEISMIC RESPONSE COEFFICIENT = 0.065
RESPONSE MODIFICATION FACTOR, R = 3.5
ANALYSIS PROCEEDURE USED = EQUIVALENT LATERAL FORCE PROCEDURE

ASSUMED SOIL BEARING CAPACITY

2,000 PSF

SUBMITALS

SHOP DRAWINGS FOR ALL STRUCTURAL ITEMS MENTIONED ABOVE ARE PART OF THE STRUCTURAL DESIGN OF THIS PROJECT AND MUST BE SUBMITTED AND REVIEWED BY OUR OFFICE. IF THE CONTRACTOR OR OWNER FAILS TO SUBMIT SHOP DRAWINGS FOR OUR REVIEW PRIOR TO BEGINNING OF CONSTRUCTION WORK, EMMANUEL ANSA, P.E. WILL NOT BE RESPONSIBLE FOR THE STRUCTURAL DESIGN OF THIS PROJECT. THE CONTRACTOR OR OWNER IS REQUIRED TO SUBMIT SHOP DRAWINGS AND CALCULATIONS (IF ANY) FOR THE FOLLOWING ITEMS TO THE STRUCTURAL ENGINEER OF THE PROJECT FOR REVIEW PRIOR TO FABRICATION OR CONSTRUCTION OF THESE ITEMS:

1.

CONCRETE MIX DESIGN ACCORDING TO ACI REQUIREMENTS.
- * 2.

STRUCTURAL STEEL
3.

METAL DECK
- * 4.

EXTERIOR WALL LIGHT GAGE METAL STUD WALL AND PARAPET SYSTEM.

ITEMS IDENTIFIED WITH AN ASTERISK SHALL HAVE THEIR DRAWINGS AND CALCULATIONS SIGNED AND SEALED BY A PROFESSIONAL ENGINEER IN THE LOCAL CODE JURISDICTION.

THE REVIEW OF SHOP DRAWINGS BY THE STRUCTURAL ENGINEER DOES NOT RELIEVE THE CONTRACTOR, SUBCONTRACTOR OR FABRICATOR THE RESPONSIBILITY OF MEETING THE REQUIREMENTS OF THE ENTIRE DESIGN DOCUMENTS OF THIS PROJECT INCLUDING THE ARCHITECTURAL DRAWINGS AND OTHER DISCIPLINES. A MINIMUM OF TWO WEEKS SHALL BE ALLOWED FOR THE STRUCTURAL ENGINEER TO REVIEW ANY SUBMITTAL.

STEEL STAIRS

1.

SEE SPECIFICATIONS.
2.

STAIR SUPPLIER SHALL DESIGN ALL STAIR FRAMING INCLUDING RAILINGS TO SUPPORT THE FOLLOWING LOADS:

STAIRS - DEAD LOAD - AS REQUIRED BY CONSTRUCTION

LIVE LOAD - 100 PSF OR 300 POUND

CONCENTRATED LOAD AT CENTER OF TREND

RAILINGS - 50 POUNDS PER LINEAR FOOT OR 200 POUNDS

CONCENTRATED LOAD APPLIED IN ANY DIRECTION.
3.

SUBMIT COMPLETE SHOP AND ERECTION DRAWINGS FOR REVIEW PRIOR TO FABRICATION OR ERECTION. STAIR SUPPLIER'S SHOP DRAWINGS SHALL CONTAIN A CERTIFICATION SEALED BY A REGISTERED PROFESSIONAL ENGINEER STATING THAT THE STAIR AND RAILING COMPONENTS HAVE BEEN DESIGNED TO SUPPORT THE SPECIFIED LOADS.

EXISTING CONSTRUCTION

1.

EXISTING CONDITIONS INDICATED ON THE DRAWINGS ARE BASED ON INFORMATION FROM THE ARCHITECT OF THE PROJECT. CONTRACTOR SHALL VERIFY AND COORDINATE EXISTING CONDITIONS AND DIMENSIONS PRIOR TO PROCEEDING WITH WORK. SHOULD SIGNIFICANT DIFFERENCES BE FOUND BETWEEN GIVEN CONDITIONS AND ACTUAL FIELD CONDITIONS, THE ARCHITECT SHALL BE NOTIFIED BEFORE PROCEEDING WITH WORK. REMOVE EXISTING CONSTRUCTION AS REQUIRED TO INSTALL NEWWORK. THE CONTRACTOR SHALL BE RESPONSIBLE FOR ANY DAMAGE TO ADJACENT EXISTING ELEMENTS AND SHALL PROVIDE ADEQUATE REPAIRS NECESSARY TO RESTORE TO ORIGINAL CONDITION AT THE CONTRACTORS EXPENSE, WITHOUT ANY COST TO THE OWNER.
2.

DIMENSIONS SHOWN ON THE STRUCTURAL DRAWINGS ARE GENERALLY OBTAINED FROM THE ARCHITECT AND ARE INCLUDED AS INFORMATION COMPLIMENTARY TO THE ARCHITECTURAL DRAWINGS. GENERALLY, ARCHITECTURAL DIMENSIONS WILL GOVERN OVER STRUCTURAL DIMENSIONS. LAYOUT OF BUILDING FOUNDATIONS OR OTHER ITEMS MAY BE MADE USING THE DIMENSIONS SHOWN ON THE STRUCTURAL DRAWINGS ONLY IF THE CONTRACTOR HAS CAREFULLY STUDIED AND COMPARED THESE DRAWINGS WITH THE ARCHITECTURAL DRAWINGS AND HAS RECEIVED CLARIFICATION FROM THE ARCHITECT, REGARDING ANY ERRORS, INCONSISTENCIES OR OMISSIONS. ITEMS INSTALLED INCORRECTLY, DUE TO THE CONTRACTOR'S LACK OF SUCH VERIFICATION, SHALL BE CORRECTED AT THE CONTRACTORS

STRUCTURAL ENGINEER
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Drawing Revisions:

Drawing Date: 12/12/2022
Project number: 220001
Drawn By: TS
Checked By: EA

GEORGIA MOUNTAIN FOOD BANK
1042 CALVARY INDUSTRIAL DRIVE SW
GAINESVILLE, GA 30507

HALL COUNTY
P.O. BOX DRAWER 1435
GAINESVILLE, GA 30503

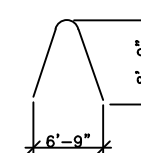
Drawing Description:

STRUCTURAL
GENERAL NOTES

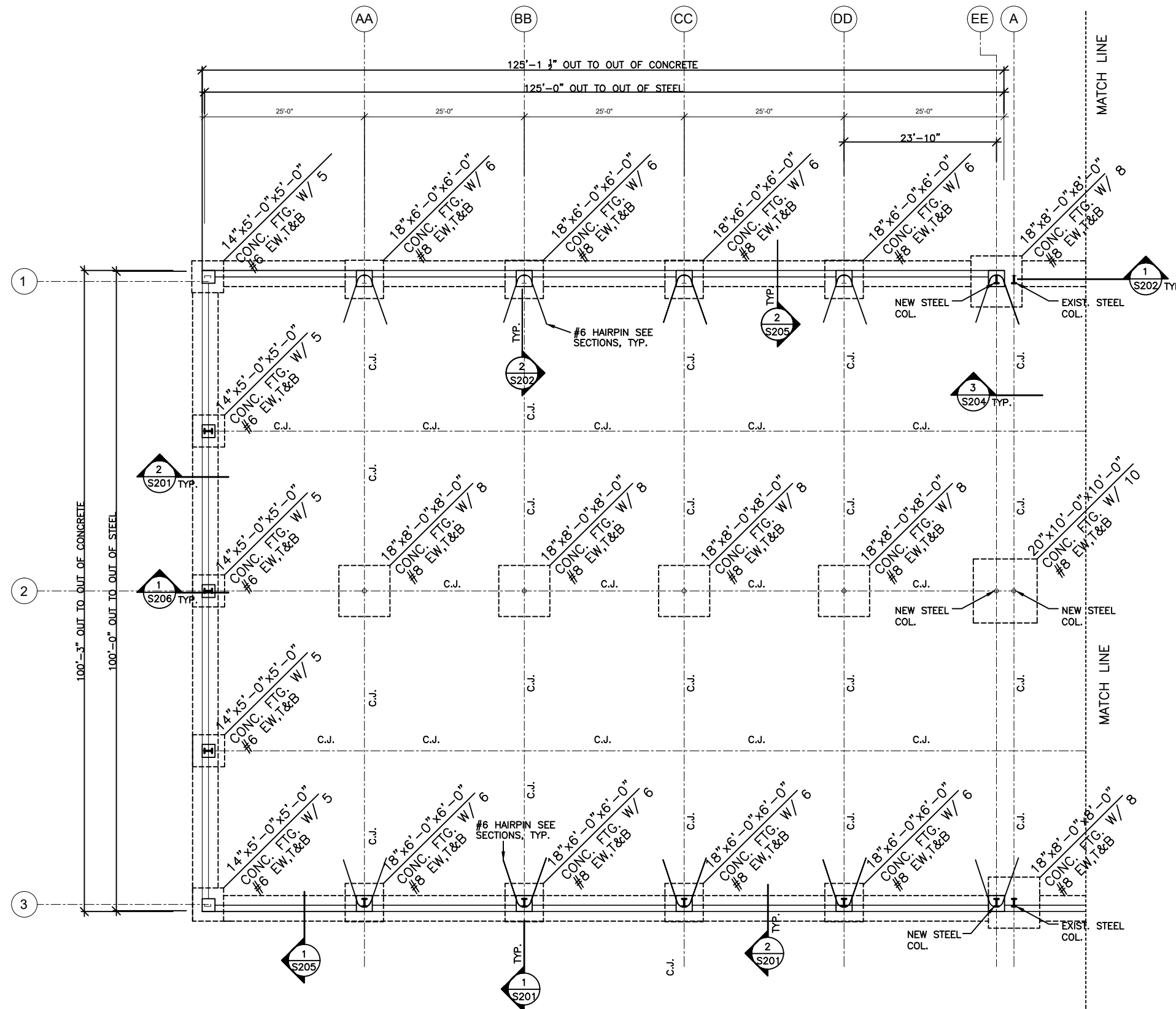
S100

PLAN NOTES :

1. THE TOP OF THE FIRST FLOOR ELEVATION SHALL BE AS INDICATED IN THE CIVIL AND ARCHITECTURAL DRAWINGS. THIS ELEVATION SHALL EQUAL 0.00', THE REFERENCED DATUM IN THE STRUCTURAL PLANS.
2. THE CONTRACTOR SHALL RETAIN A SOIL INSPECTION AGENCY TO PERFORM SOIL INSPECTION AND VERIFY THE COMPACTION WORK AND TYPE OF SOIL USED FOR THE FOUNDATION. THE FOOTING FOR THIS BUILDING WAS DESIGNED BASED ON AN ASSUMED SOIL BEARING CAPACITY OF 2000 PSF.
3. FOR SIZE AND LOCATION OF FLOOR OPENINGS, SLOPES, RAISED AREAS, ETC. NOT SHOWN ON THIS DRAWING, SEE ARCH./MECH. AND ELECT. DRAWINGS.
4. THE SLAB ON GRADE SHALL BE 5" THICK CONC. SLAB REINFORCED WITH 4 X 4 - W4 X W4 W.W.M. PLACED ON 15 MIL VAPOR BARRIER, OVER 6" CLEANED GRAVEL OR CRUSHED STONE, UNLESS NOTED OTHERWISE ON PLAN.
5. ALL UTILITY PIPES SHOULD PENITRATE THE FOUNDATION WALL ABOVE THE TOP OF THE FOOTING. LOWER FOOTING WHEN NECESSARY TO PLACE PIPE ON TOP OF FOOTING.
6. CONTRACTOR MUST FIRST VERIFY ALL DIMENSIONS OF THE BUILDING GEOMETRY FROM THE ARCHITECTURAL DIMENSION PLAN BEFORE STARTING THE FOUNDATION CONSTRUCTION.
7. THE CONTRACTOR MUST OBTAIN THE EXACT LOCATION OF ALL COLUMN ANCHOR BOLTS FROM THE STRUCTURAL STEEL MANUFACTURER'S PLAN.
8. USE THIS DRAWING IN CONJUNCTION WITH:
 - (a) STRUCTURAL NOTES ON DWG. S100
 - (b) SECTIONS AND DETAILS ON DWGS. S200 TO S206
 - (c) ENTIRE CONTRACT DOCUMENTS ON THIS PROJECT

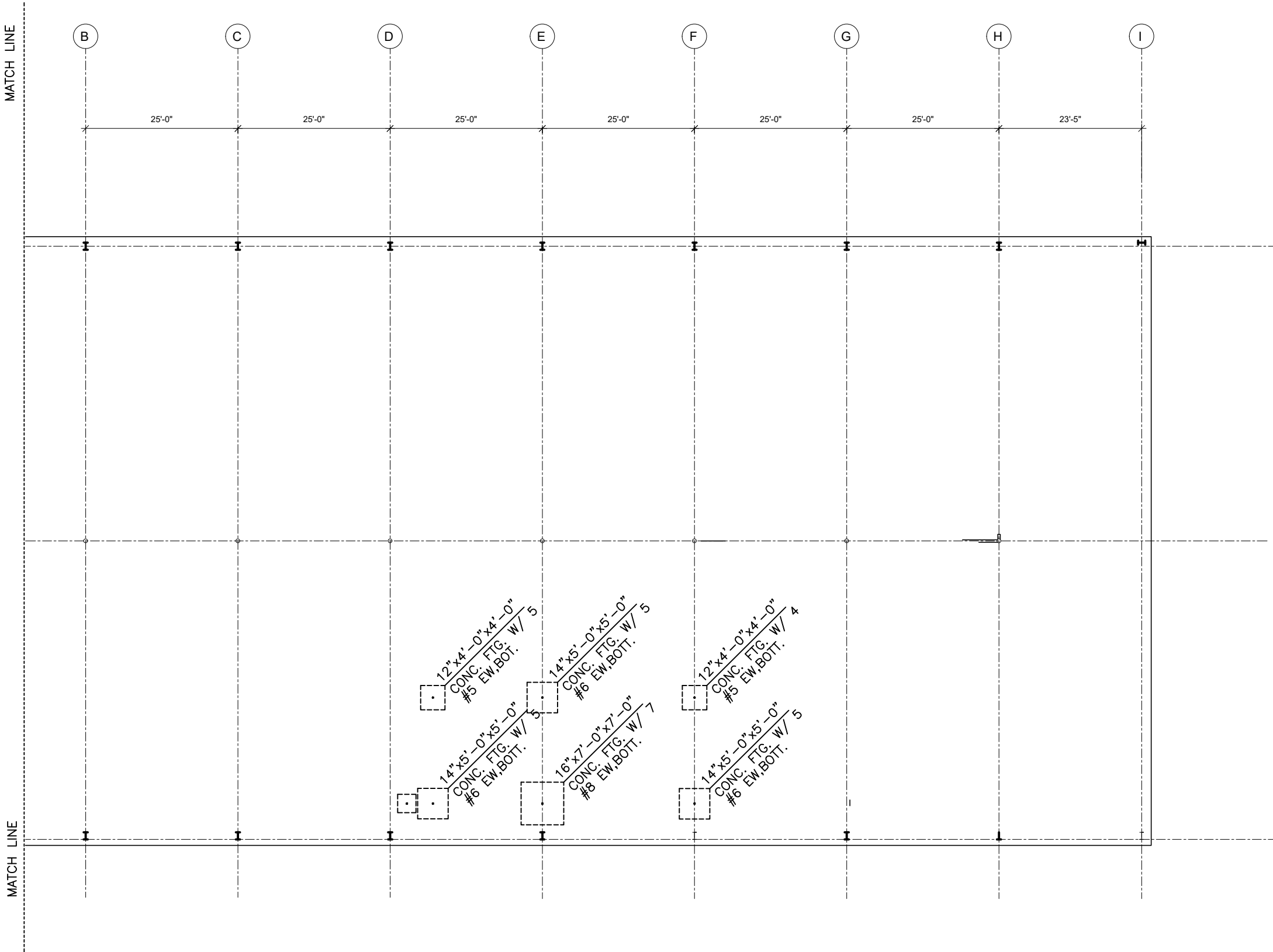


TYPICAL HAIRPIN



FOUNDATION / FIRST FLOOR PLAN

SCALE: 1/8" = 1'-0"



FOUNDATION / FIRST FLOOR PLAN

SCALE: 1/8" = 1'-0"

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Drawing Date: 12/12/2022
Project number: 220001
Drawn By: TS
Checked By: EA

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1642 CALVARY INDUSTRIAL DRIVE SW
GAINESVILLE, GA 30507
HALL COUNTY
P.O. BOX DRAWER 1435
GAINESVILLE, GA 30503

Drawing Description:

FOUNDATION/FIRST FLOOR PLAN

S102

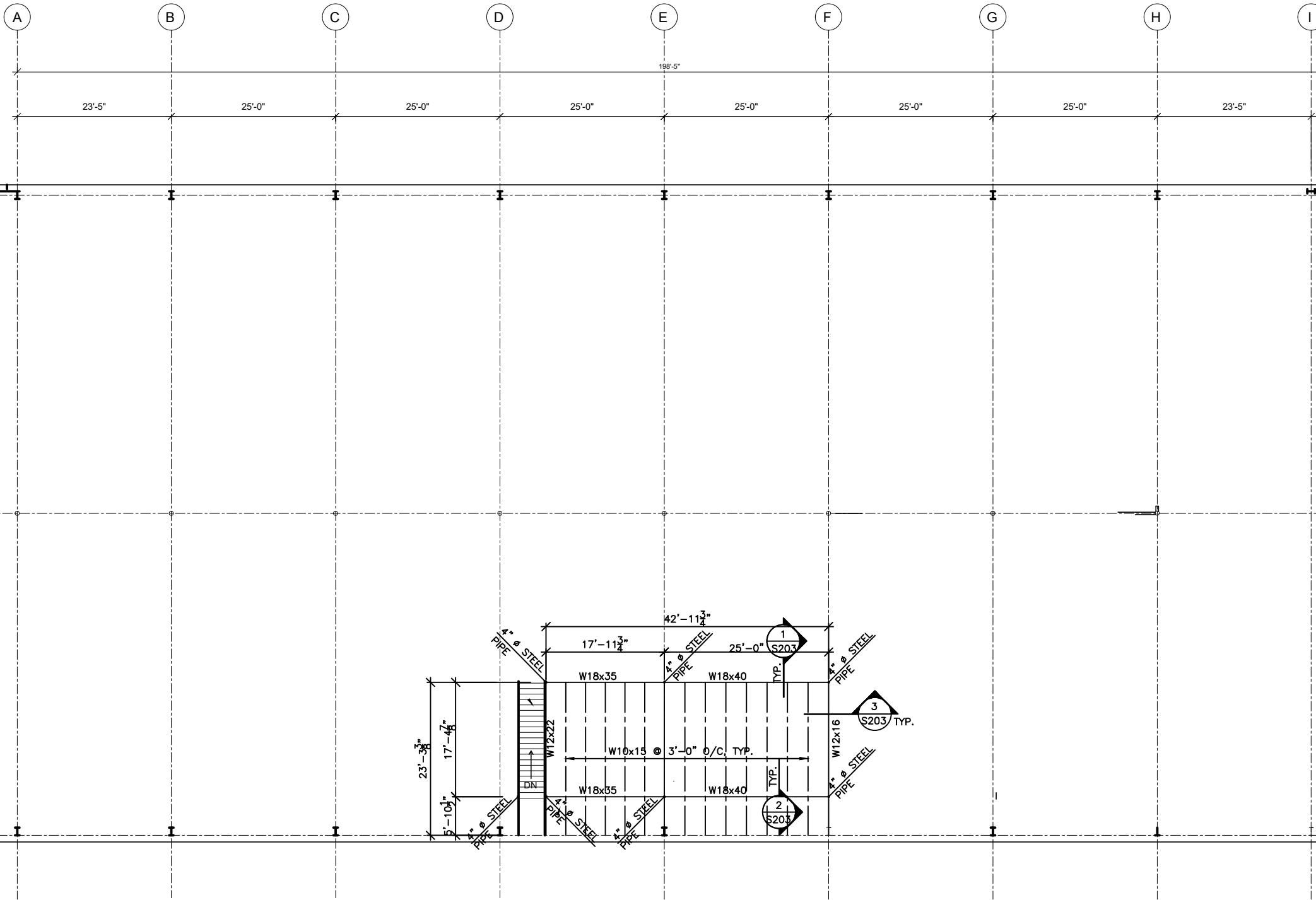
MATCH LINE

1

2

3

MATCH LINE

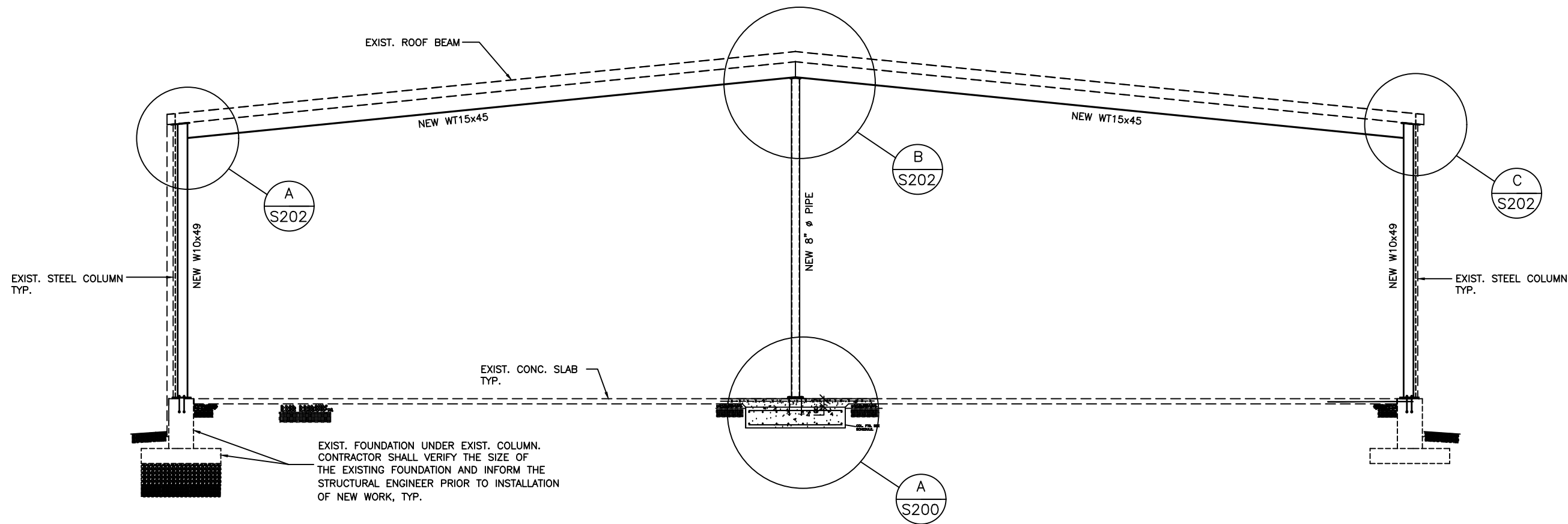


PLAN NOTES :

1. PROPOSED SECOND FLOOR CONSTRUCTION SHALL BE 3" CONCRETE TOPPING ON 1"x 22 GA. FORM DECK SIMILAR TO VULCRAFT 1.0C22 WITH 4x4-W2.9xW2.9 WWM.
2. CONTRACTOR SHALL OBTAIN THE EXACT LOCATION OF FLOOR OPENINGS FROM THE MECH. DWGS.
3. USE THIS DRAWING IN CONJUNCTION WITH:
 - (a) STRUCTURAL NOTES ON DWG. S100
 - (b) PLANS, SECTIONS AND DETAILS ON DWGS. S200 TO S205
 - (c) ENTIRE CONTRACT DOCUMENTS ON THIS PROJECT

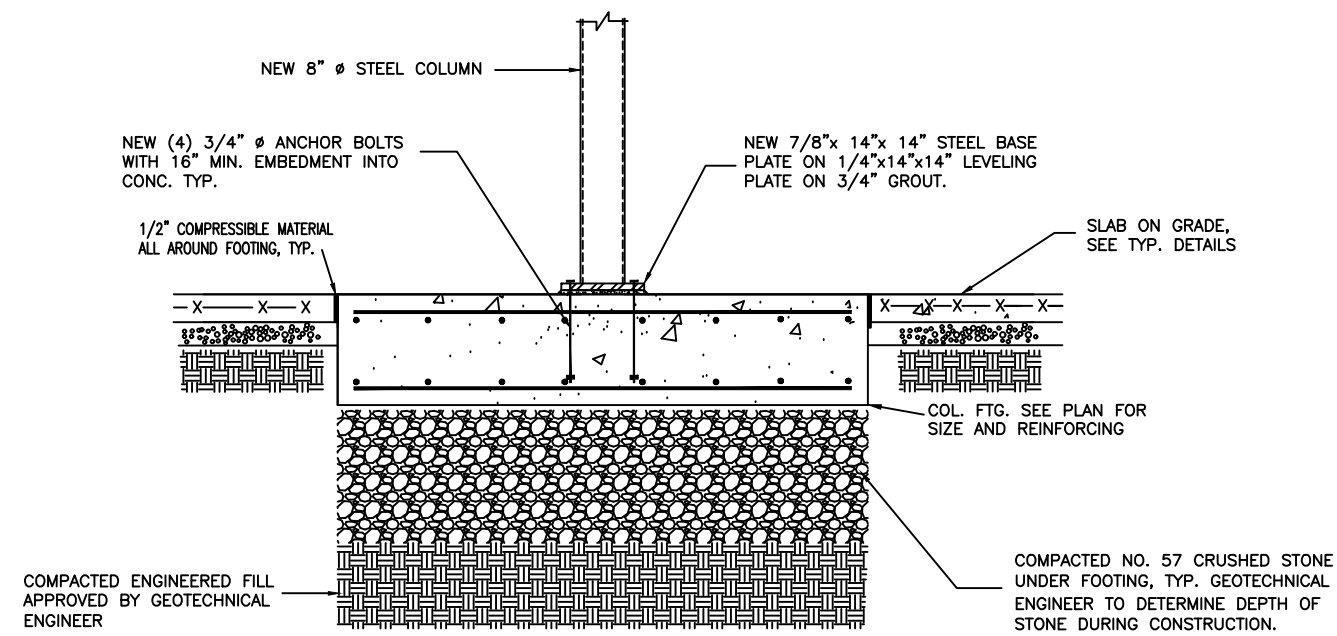
SECOND FLOOR FRAMING PLAN

SCALE : 1/8" = 1'-0"



REINFORCEMENT OF EXISTING STEEL FRAME ALONG GRID LINE A

SCALE: NTS



FOOTING FOR NEW INTERIOR COLUMN ALONG GRID LINE A

SCALE: 3/4 inch = 1 foot - 0 inch

A
S200

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Drawing Date: 09/30/2022
Project number: 220001
Drawn By: TS
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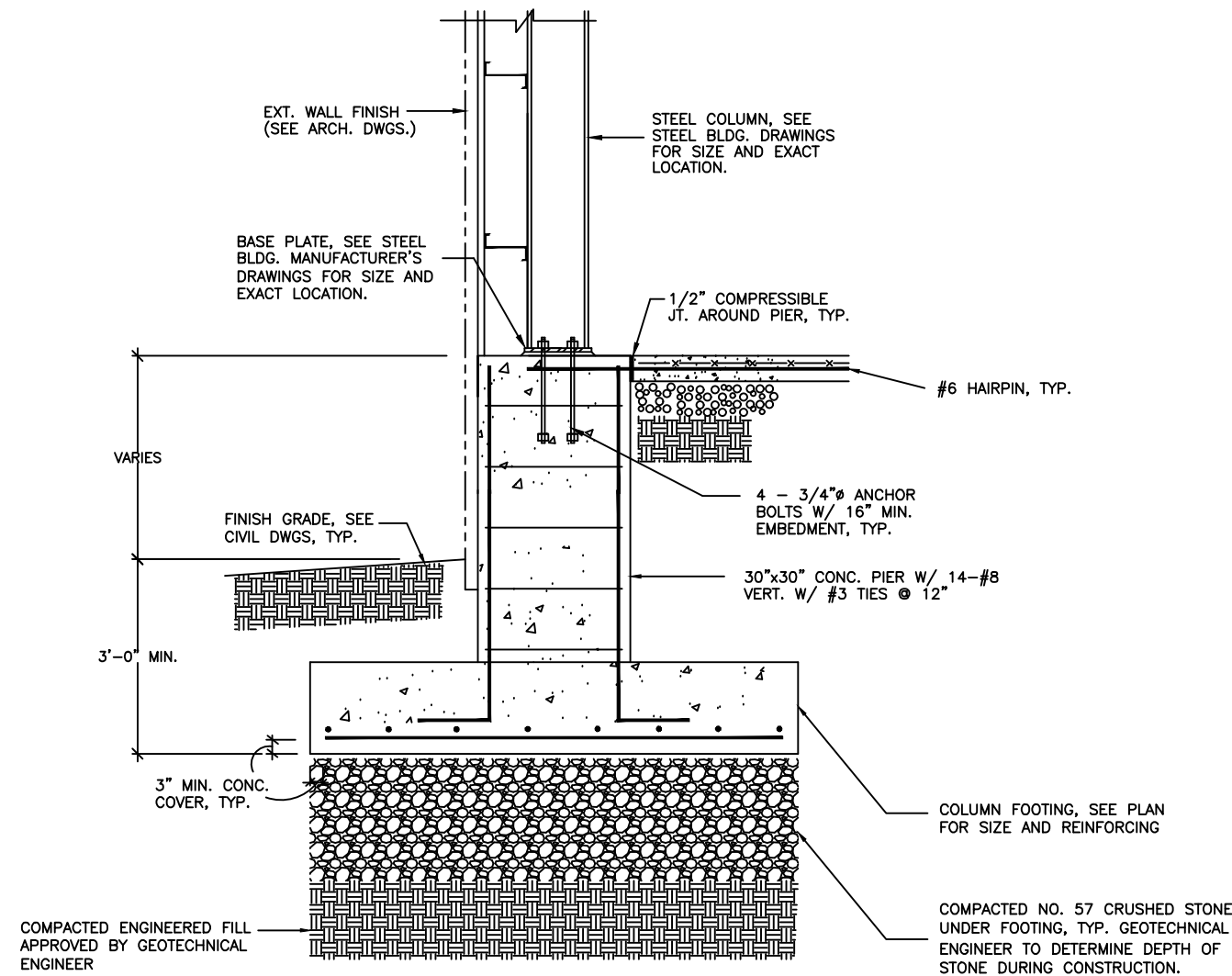
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1042 CALVARY INDUSTRIAL DRIVE SW
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HALL COUNTY
P.O. BOX DRAWER 1435
GAINESVILLE, GA 30503

Drawing Description:

ELEVATION AND
DETAILS

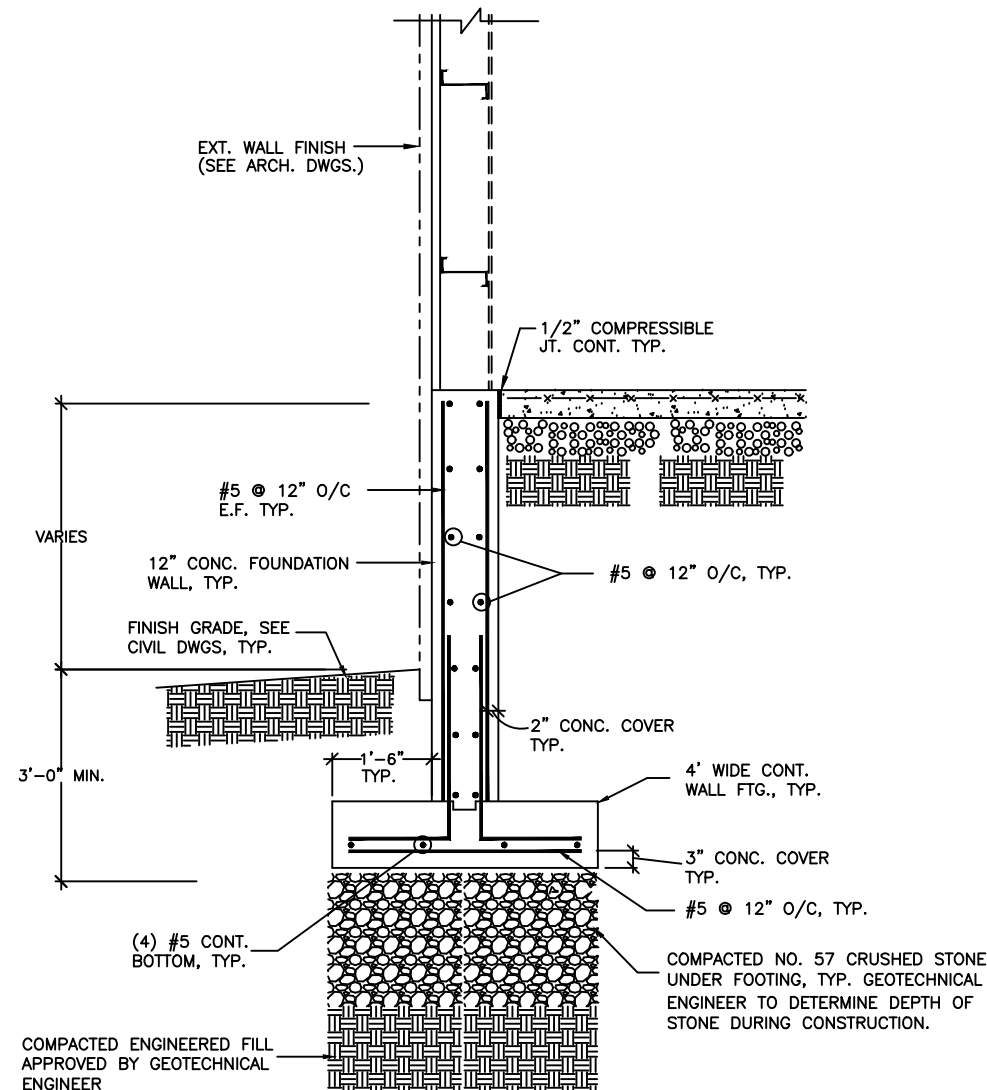
S200



SECTION - EXTERIOR COLUMN ON
CONCRETE PIER

SCALE: 3/4" = 1'-0"

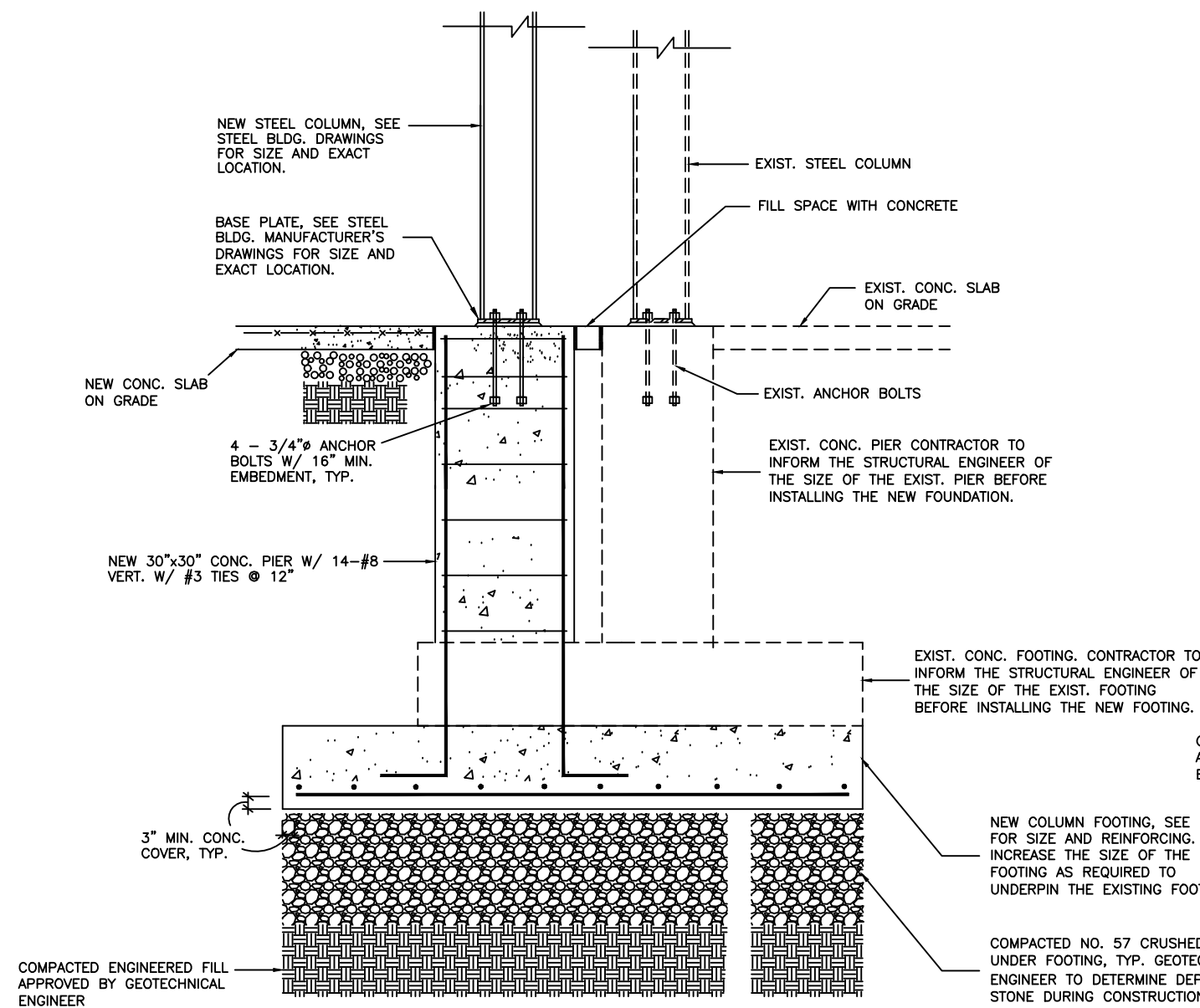
1
S201



SECTION - EXTERIOR FOUNDATION WALL

SCALE: 3/4" = 1'-0"

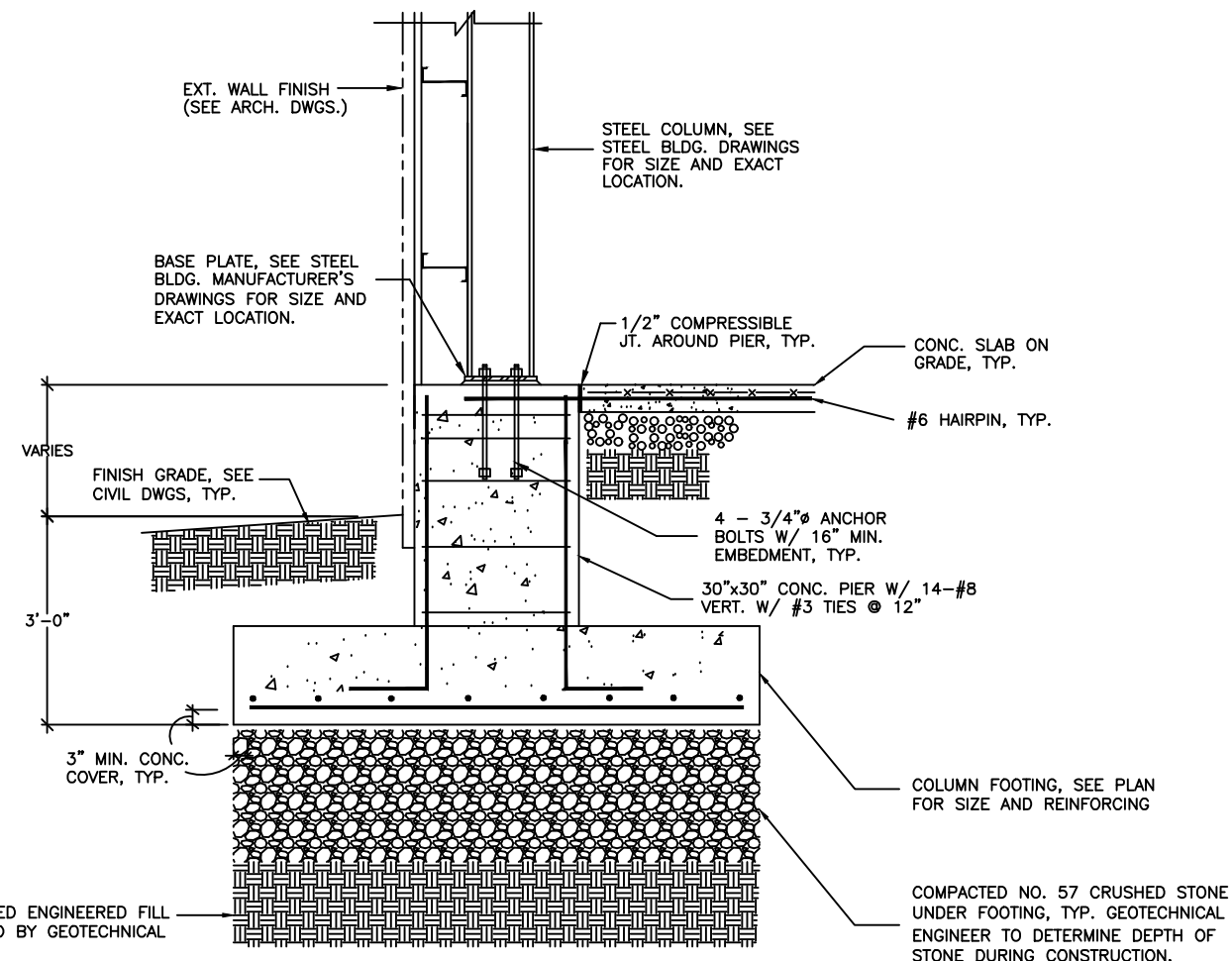
2
S201



SECTION - FOUNDATION OF NEW COLUMN
ADJACENT TO EXISTING COLUMN

SCALE: 3/4" = 1'-0"

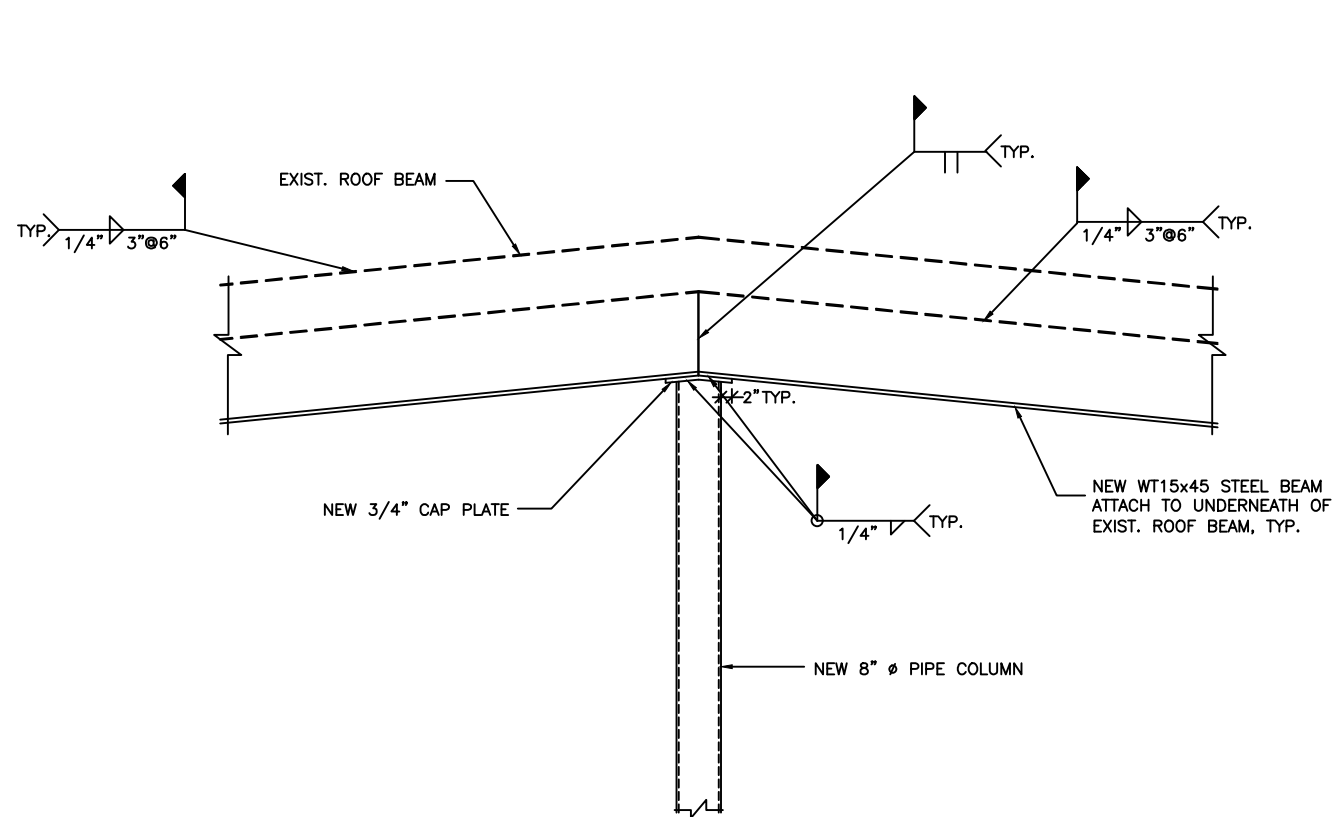
1
S202



SECTION - EXTERIOR COLUMN ON
CONCRETE PIER

SCALE: 3/4" = 1'-0"

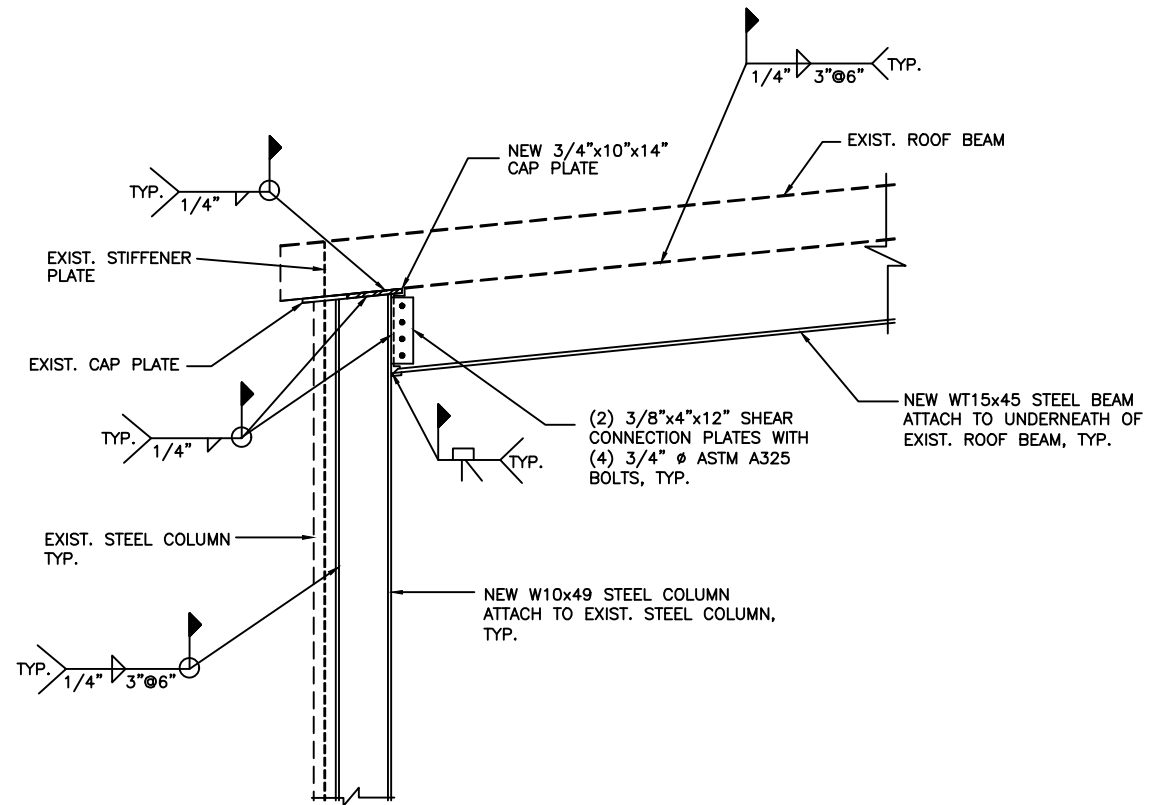
2
S202



DETAIL

SCALE: 3/4"=1'-0"

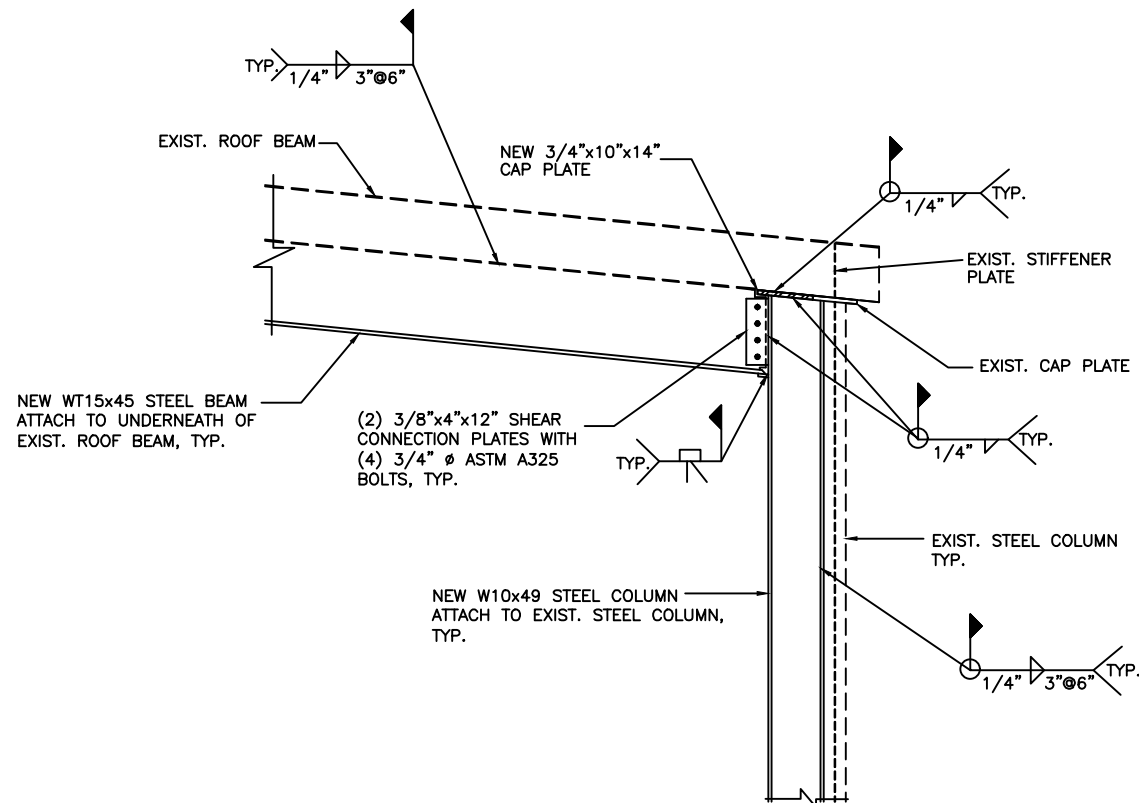
B



DETAIL

SCALE: 3/4"=1'-0"

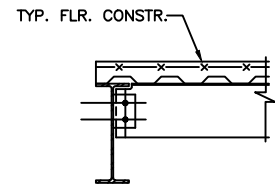
A



DETAIL

SCALE: 3/4"=1'-0"

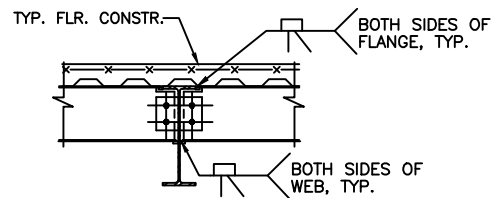
C



SECTION

SCALE: 3/4" = 1'-0"

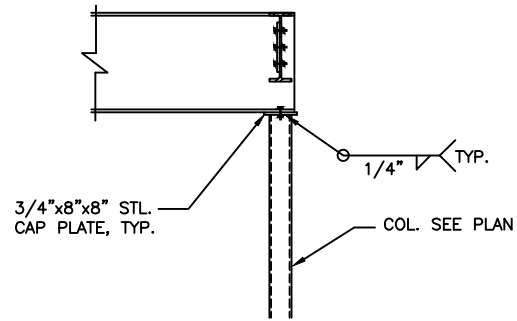
1



SECTION

SCALE: 3/4" = 1'-0"

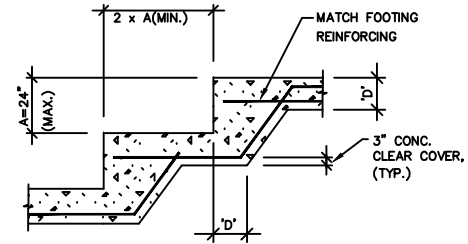
2



TYPICAL DETAIL : STEEL BEAM ON
PIPE COLUMN

SCALE : 3/4" = 1'-0"

A

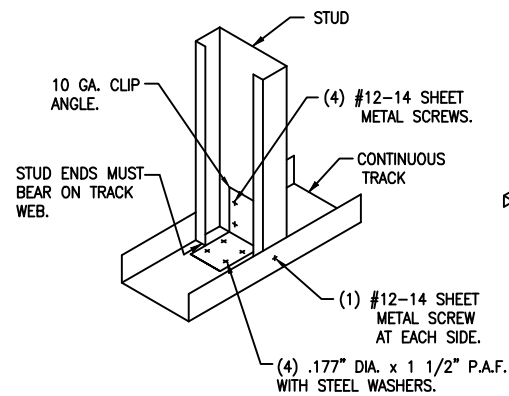


SEE WALL AND FOOTING DETAILS FOR REINFORCING

TYPICAL STEP FOOTING DETAIL

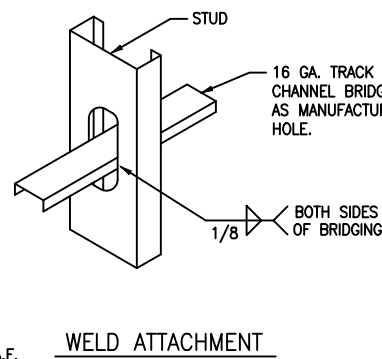
NOT TO SCALE

B

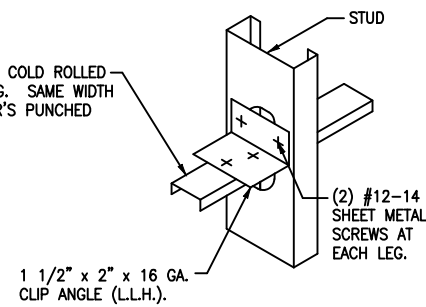


SINGLE STUD ATTACHMENT
SCREW CONNECTION

C



WELD ATTACHMENT

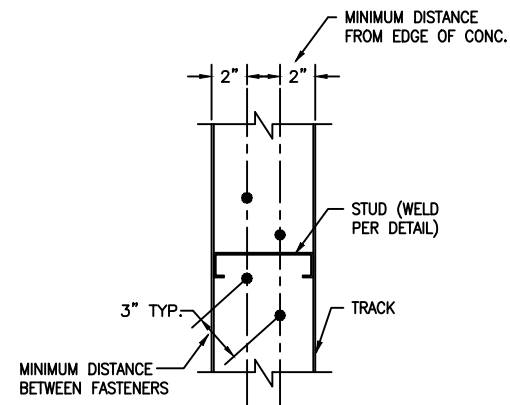


SCREW ATTACHMENT

STUD BRIDGING DETAIL

(SEE ELEVATIONS FOR BRIDGING LOCATIONS)
(SEE DRAWING 242D2 FOR TYPICAL BRIDGING SPLICE)

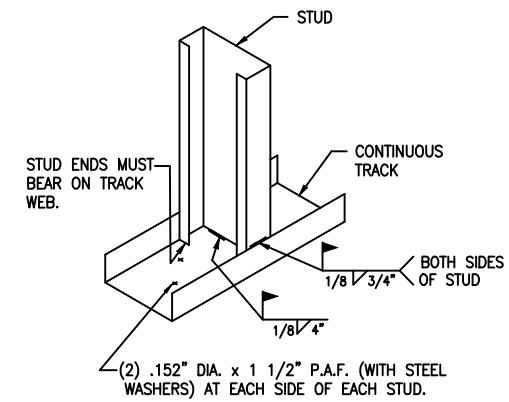
D



FASTENING ARRANGEMENT
TRACK TO CONCRETE (PLAN)

TYPICAL DETAIL

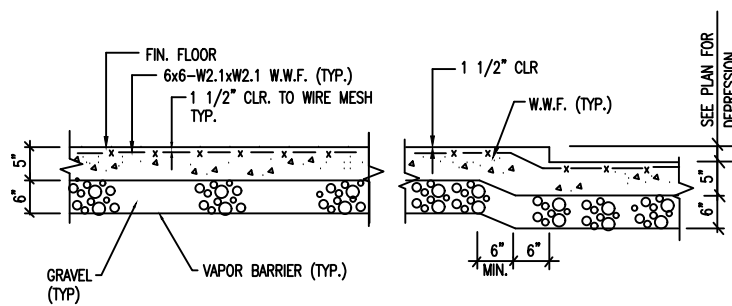
E



SINGLE STUD ATTACHMENT
TRACK TO CONCRETE (VIEW)

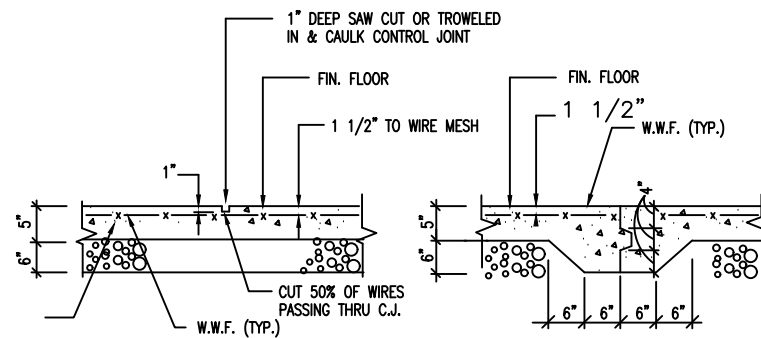
TYPICAL DETAIL

F



TYPICAL

AT DEPRESSION



CONTROL JOINT (C.J.)

TYP. CONSTRUCTION JOINT

TYPICAL SLAB ON GRADE DETAILS

NOT TO SCALE

J

SECTION AT NEW AND EXISTING CONCRETE SLAB LINE

SCALE: 3/4"=1'-0"

3

STRUCTURAL ENGINEER
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Drawing Date: 12/12/2022
Project number: 220001
Drawn By: TS
Checked By: EA

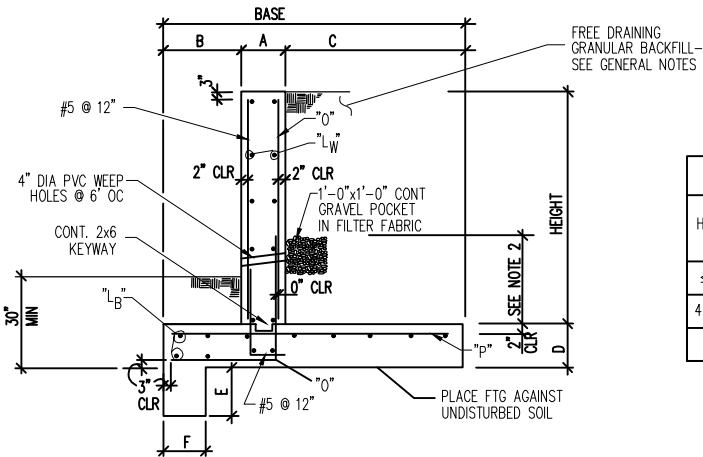
GEORGIA MOUNTAIN FOOD BANK
1642 CALVARY INDUSTRIAL DRIVE SW
GAINESVILLE, GA 30507

HALL COUNTY
P.O. BOX DRAWER 1435
GAINESVILLE, GA 30503

Drawing Description:

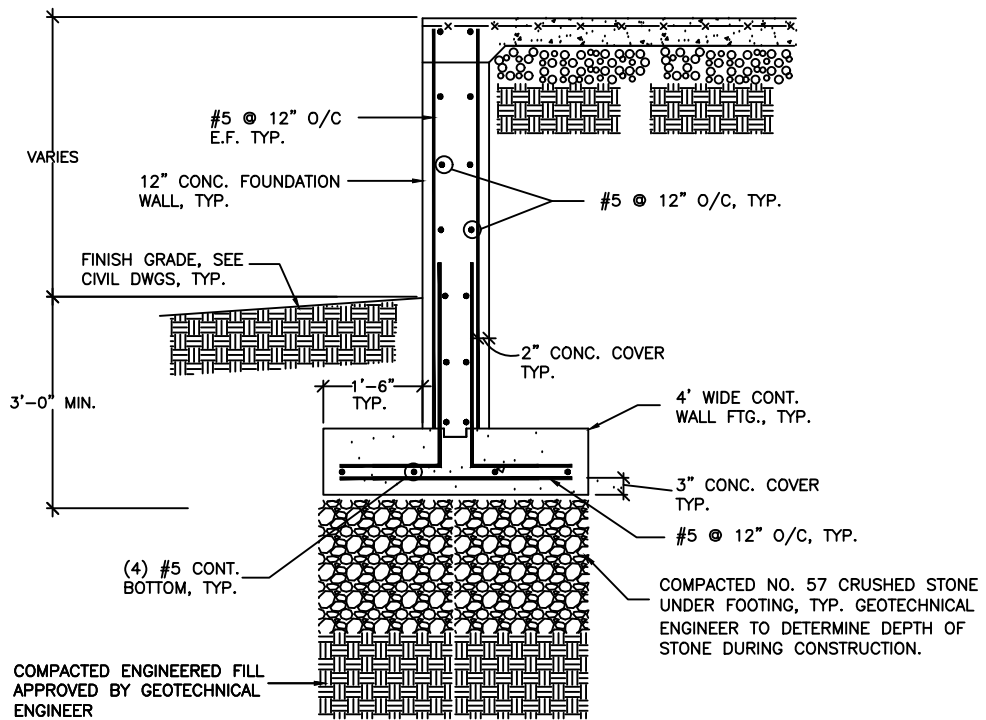
SECTIONS AND
DETAILS

S204



- CONCRETE RETAINING WALL NOTES:
- 1) PROVIDE WEAKENED PLANE CONTROL JOINTS OR CONSTRUCTION JOINTS AT 25'-0"± ON CENTER AS PER DETAIL 3/SA602 AND 4/SA602.
 - 2) PROVIDE KEED EXPANSION JOINTS AT 100'-0"± ON CENTER AS PER DETAIL 6/SA602.
 - 3) ALL LAP SPLICES OF TENSION FACE REINFORCING AND HORIZONTAL REINFORCING SHALL BE TENSION LAP SPLICES. SEE 1/SA601 FOR SCHEDULE.
 - 4) TOP OF FOOTING SURFACE AT WALL SHALL BE INTENTIONALLY ROUGHENED TO A FULL AMPLITUDE OF APPROXIMATELY 1/4".
 - 5) CHAMFER ALL EXPOSED WALL CORNERS.
 - 6) ALL BACKFILL SHALL BE ACCOMPLISHED USING MATERIAL CONSISTING OF BANK RUN GRAVEL, CRUSHED STONE AND/OR MATERIAL APPROVED BY THE GEOTECHNICAL ENGINEER, WITH OPTIMUM MOISTURE CONTENT FOR COMPACTING AND SHALL BE FREE OF ANY DEBRIS.

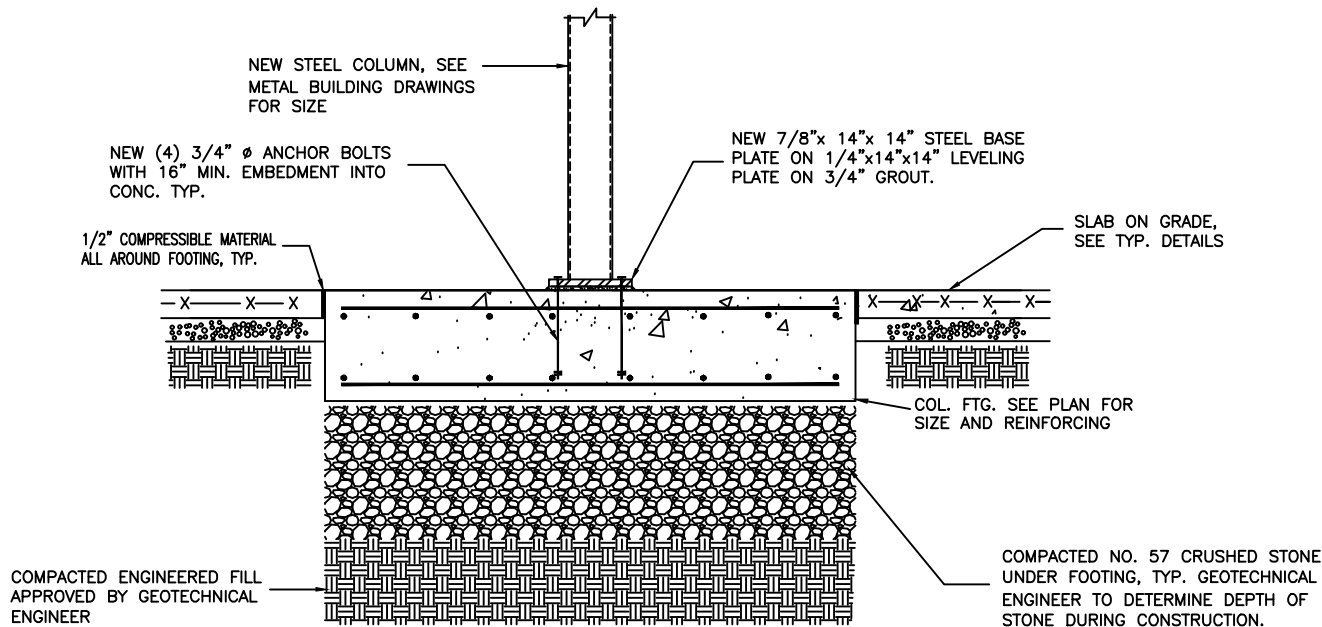
CONCRETE RETAINING WALL SCHEDULE											
HEIGHT (ft.)	BASE	B	A	C	D	E	F	REINFORCEMENT			
								"O" BARS	"L _B " BARS	"P" BARS	"L _W " BARS
≤3	3'-6"	1'-0"	10"	2'-8"	1'-1"			#5 @ 8"	#5 @ 12"	#5 @ 12"	#5 @ 12"
4 – 6	5'-6"	1'-6"	1'-0"	4'-6"	1'-2"			#5 @ 8"	#5 @ 12"	#5 @ 12"	#5 @ 12"



SECTION

SCALE: 3/4" = 1'-0"

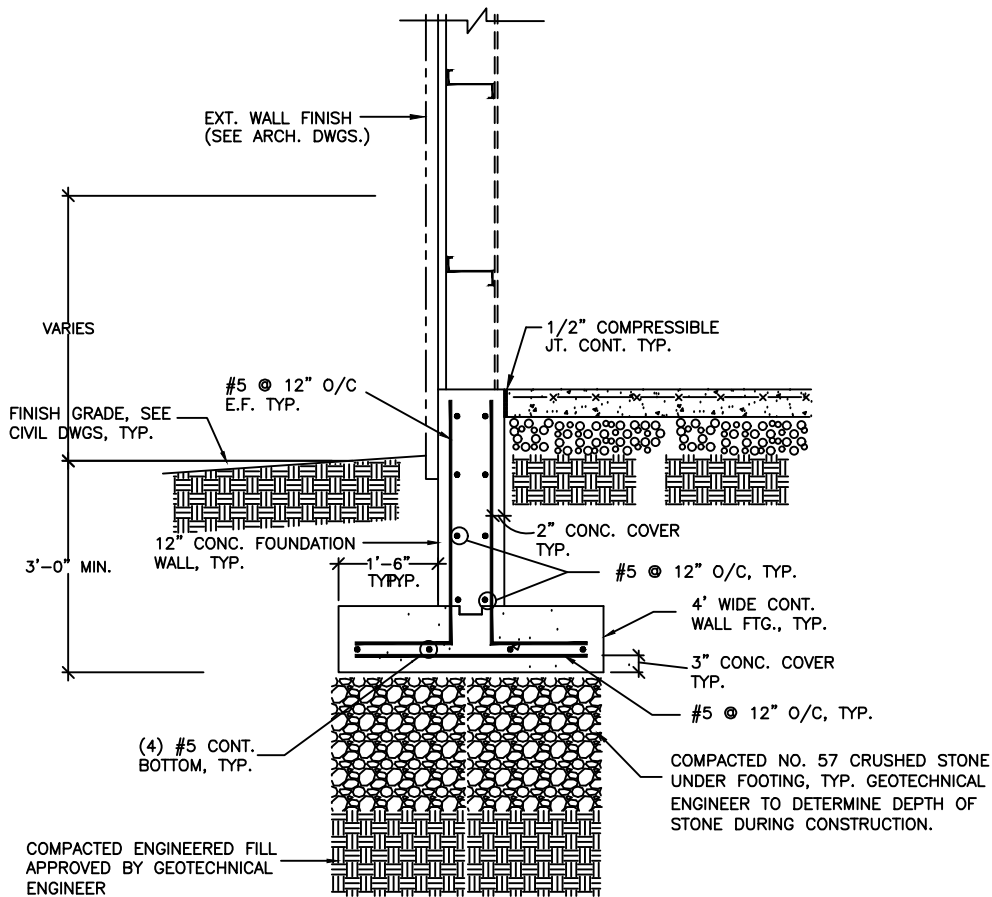
1
S205



DETAIL — NEW INTERIOR COLUMN FOOTING

SCALE: 3/4"=1'-0"

A



SECTION

SCALE: 3/4" = 1'-0"

2
S205

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Drawing Revisions:

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Drawn By: TS
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GEORGIA MOUNTAIN FOOD BANK
1642 CALVARY INDUSTRIAL DRIVE SW
GAINESVILLE, GA 30507

HALL COUNTY
P.O. BOX DRAWER 1435
GAINESVILLE, GA 30503

Drawing Description:

SECTIONS AND DETAILS

S205

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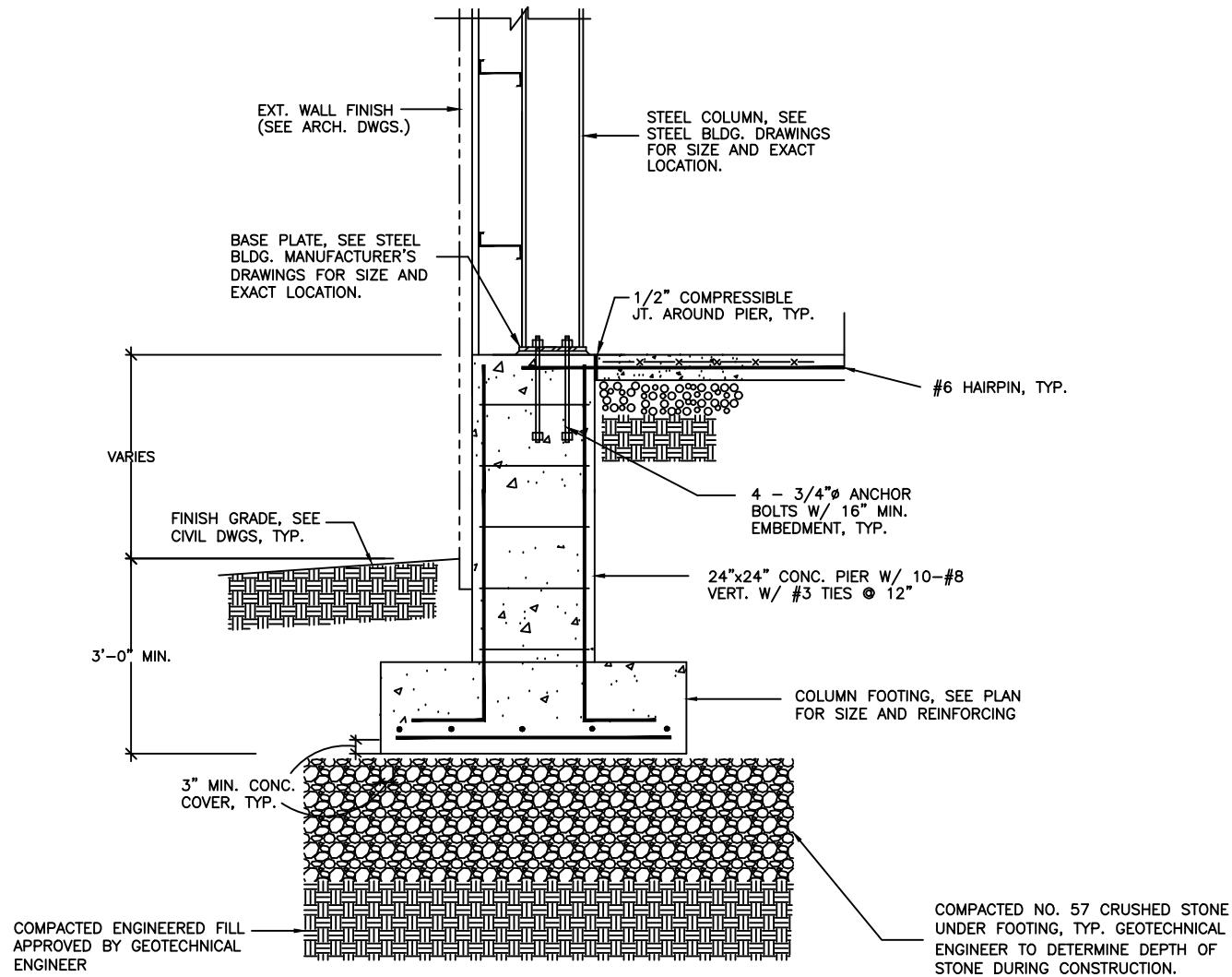
Drawing Date: 12/12/2022
Project number: 220001
Drawn By: TS
Checked By: EA

GEORGIA MOUNTAIN FOOD BANK
1642 CALVARY INDUSTRIAL DRIVE SW
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HALL COUNTY
P.O. BOX DRAWER 1435
GAINESVILLE, GA 30503

Drawing Description:

SECTIONS AND
DETAILS

S206



SECTION – EXTERIOR COLUMN ON
CONCRETE PIER

SCALE: 3/4" = 1'-0"

1
S206



EXHIBIT #1

Pre-Engineered Metal Building Systems

Metal Buildings

Steel Building Products

Our Customers

Portfolio

Resources

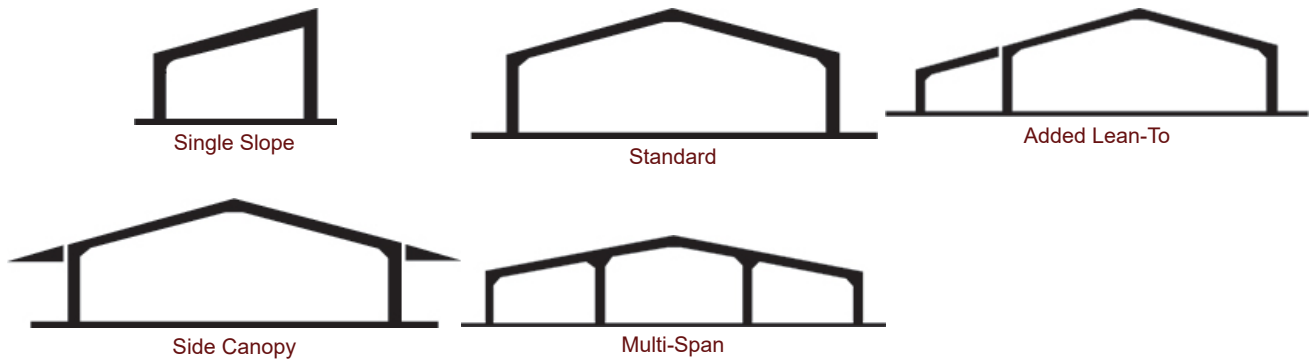
Contact

Steel Building Products

Shenango Steel Buildings are available in a wide variety of shapes, colors and sizes custom made to your specifications.

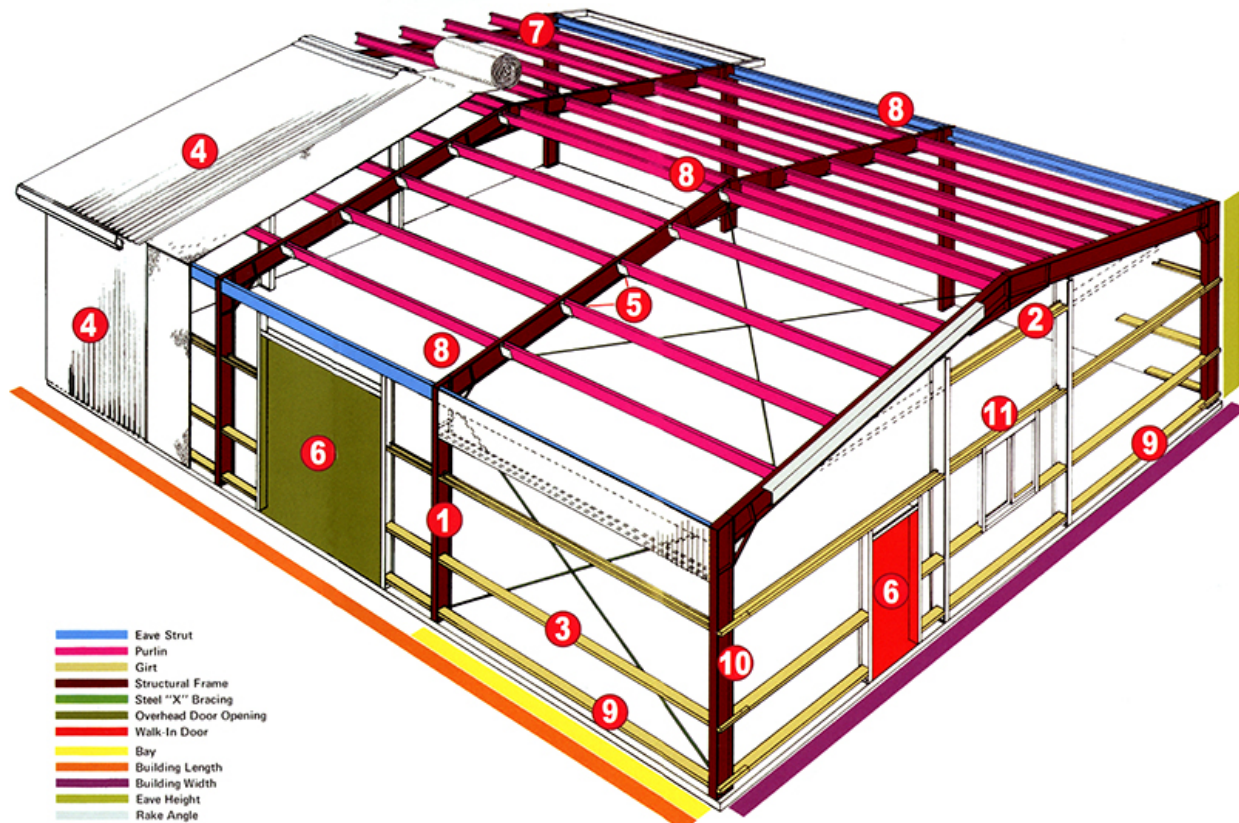
■ Color Selector ([PDF](#))

Standard Building Shapes - Customized Design & Shapes Available



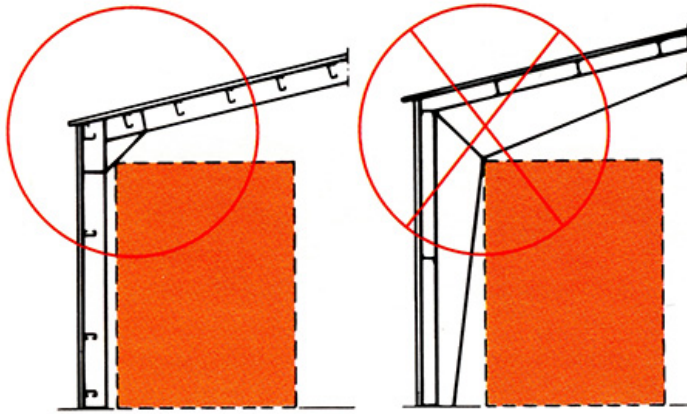
Steel Specifications:

- Structural steel sections designed in accordance with American Iron and Steel Institute (AISI) specifications
- Cold-formed structural members designed in accordance with AISI specifications
- Rigid frame designed in accordance with American Institute of Steel Construction (AISC) manual entitled Plastic Design in Steel
- All connecting bolts are high tensile steel (A325 Class)

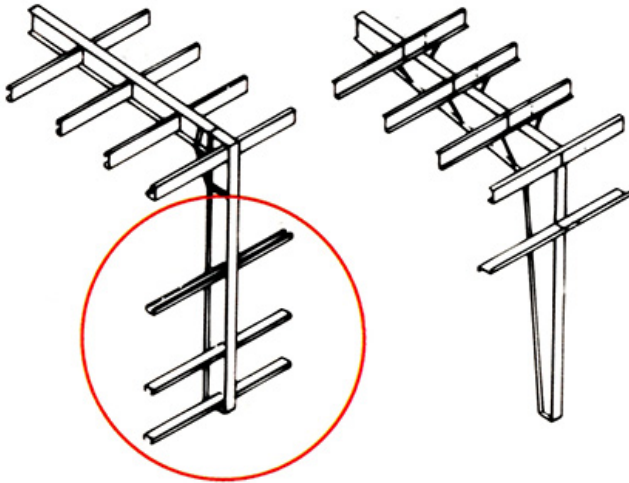


1. Rafters and columns are not tapered

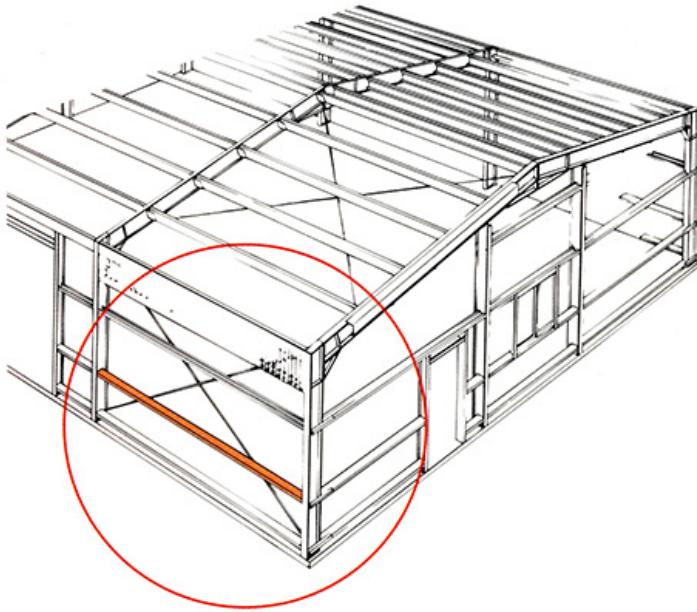
- Maximizes interior space usage compared to tapered rafters & columns

**2. Girts fitted inside the column instead of outside the column**

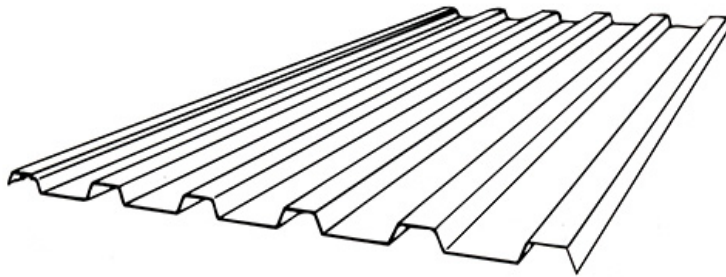
- Saves interior space & simplifies construction
- Requires less steel, bolts & concrete
- Requires less construction time

**3. Sidewall girt at waist level is standard**

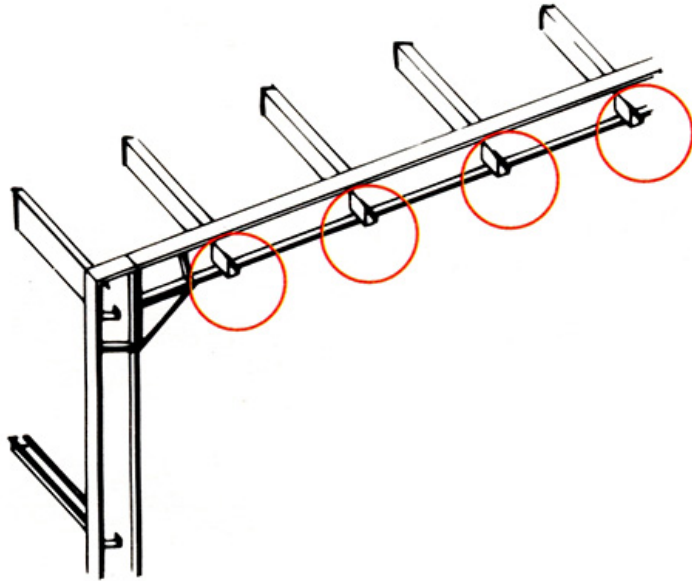
- Allows for easier installation of windows, ventilators, fans, air conditioners, louvers & other accessories

**4. Wall and roof steel panels are roll formed and cut to exact specifications**

- Available in a variety of panel profiles & color finishes
- Enhances building appearance
- Complements the project's facing material (concrete, brick, stone, stucco, etc.)

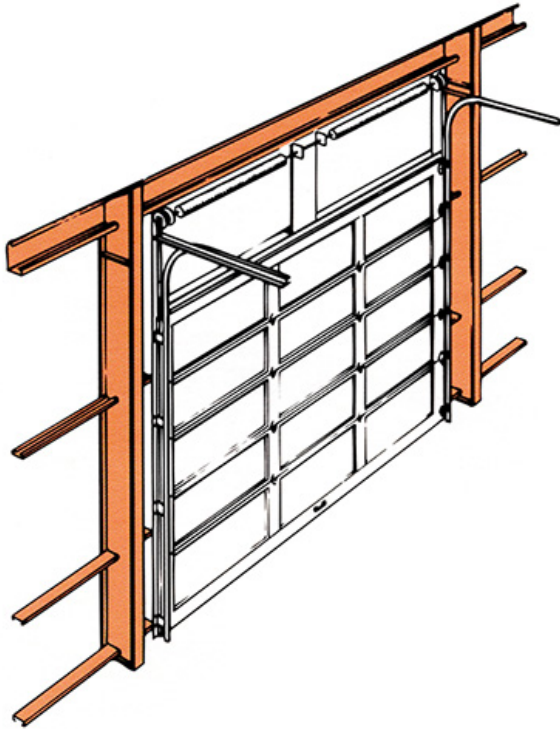
**5. Roof purlin and sidewall girt clips are welded into position at our factory**

- Accelerates construction time
- No loose brackets and fewer bolts to inventory, attach or misplace
- No on-site welding needed



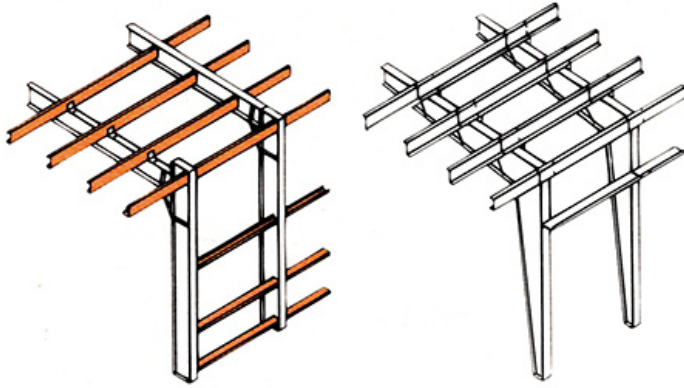
6. Doors and overhead doors can be located anywhere the customer desires them

- Parallel mounting surface permits easy attachment of support & torsion spring assemblies
- Eaves struts are designed to provide maximum space for overhead clearances



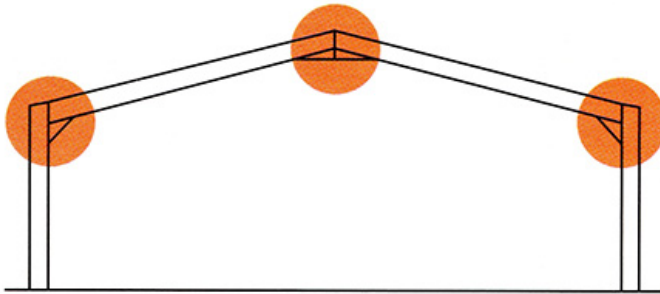
7. Overhangs are built in, not hung on

- Strengthens the steel roof as a whole
- Eliminates roof sagging & problem leaks



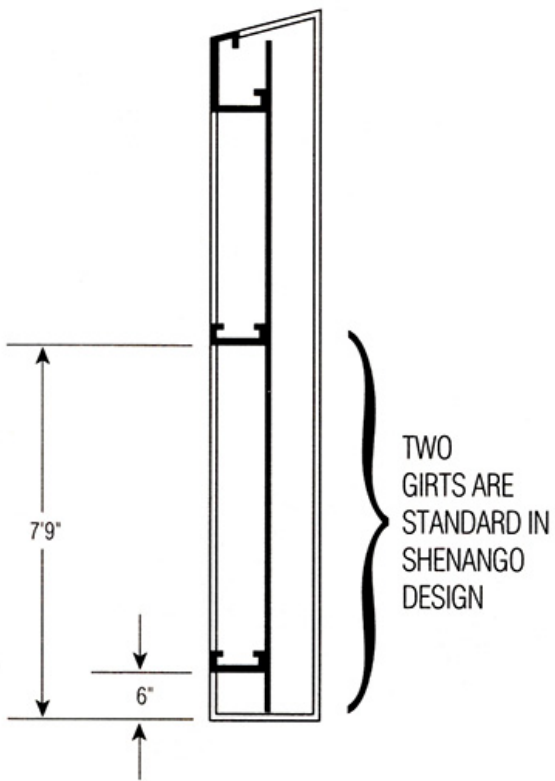
8. Only three connecting points per clear span frame (Industry standard is five)

- Increases frame strength & rigidity
- Increases assembly speed



9. First girt installed 6 inches above the floor (Industry standard is 7 to 8 feet)

- Adds strength & rigidity to metal walls
- Protects individuals & equipment inside the metal building from a lift-truck or vehicle accidentally hitting the wall

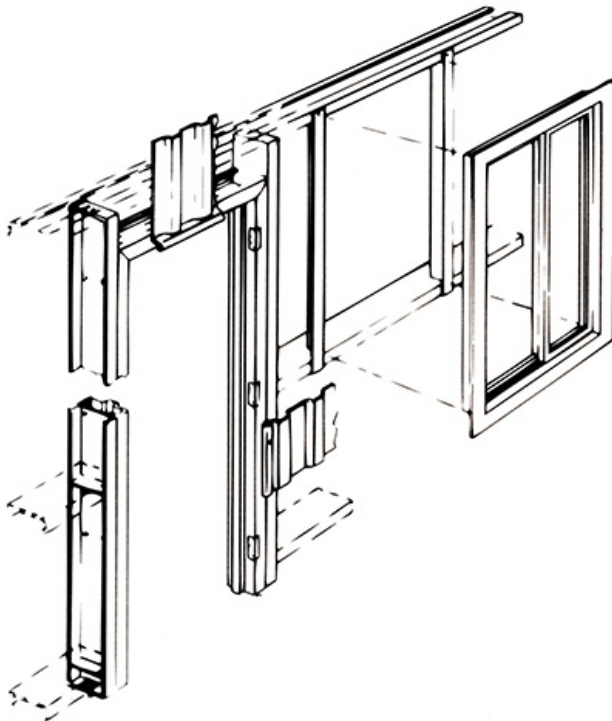


10. Endwall frames identical to intermediate frames

- Facilitates steel building expansion
- Makes door relocation possible

11. Door and window openings are framed and reinforced

- Steel subframing is used around all doors & windows
- The added support provides maximum rigidity, safety & security



Features of Shenango Steel Buildings:

- Anchor bolts included
- Comes standard with galvanized steel purlins, girts, subframing for doors, windows & overhead doors
- Self-drilling & self-tapping sheet metals screws included
- Flange of purlins, girts & frames conceal unsightly wiring & plumbing
- Accessory items such as doors, windows, light transmission panels, ventilators & more are available

Other Products**Roofing**

- Provide a flat or pitched metal roof for your new project

Siding

- Provide metal siding for your new project

Retro-Roofing

- Convert a flat steel roof into a pitched steel roof
- Cover your problem roof

Retro-Siding

- Restore unsightly exteriors
- Create vertical or mansard metal facades

Universal Building Frames

- Widths from 24 to 100 feet
- Eave heights from 8 to 24 feet
- Save the cost of using galvanized purlins & girts
- Integrate traditional building materials & styles

Shen-O-Deck [Brochure ([PDF](#))]

- Custom-made mezzanines
- Maximize the use of your overhead space with a retro-fit mezzanine
- We can design & fabricate steel mezzanines into your new Shenango Steel Building

Metal Buildings**Steel Building Products****Our Customers****Portfolio****Resources**

- Frequently Asked Questions
- Dealer Information
- Literature

Contact**Shenango Steel Buildings, Inc.**

33 Carbaugh Street
West Middlesex, PA
16159-4103 USA

P: (724) 528-9925

Toll Free: (800) 548-0625

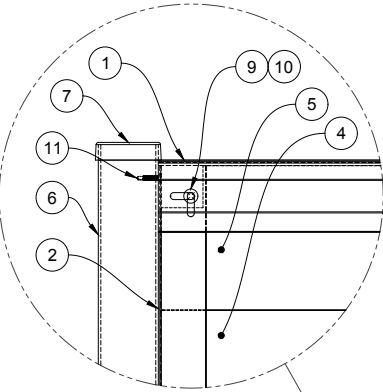
F: (724) 528-2452

E: info@shenangosteelbuildings.com

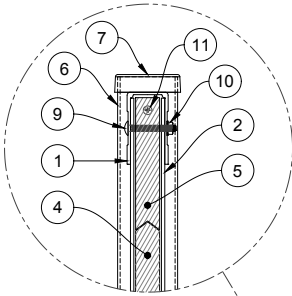


Developed by [Cassidy Advertising & Consulting LLC](#)

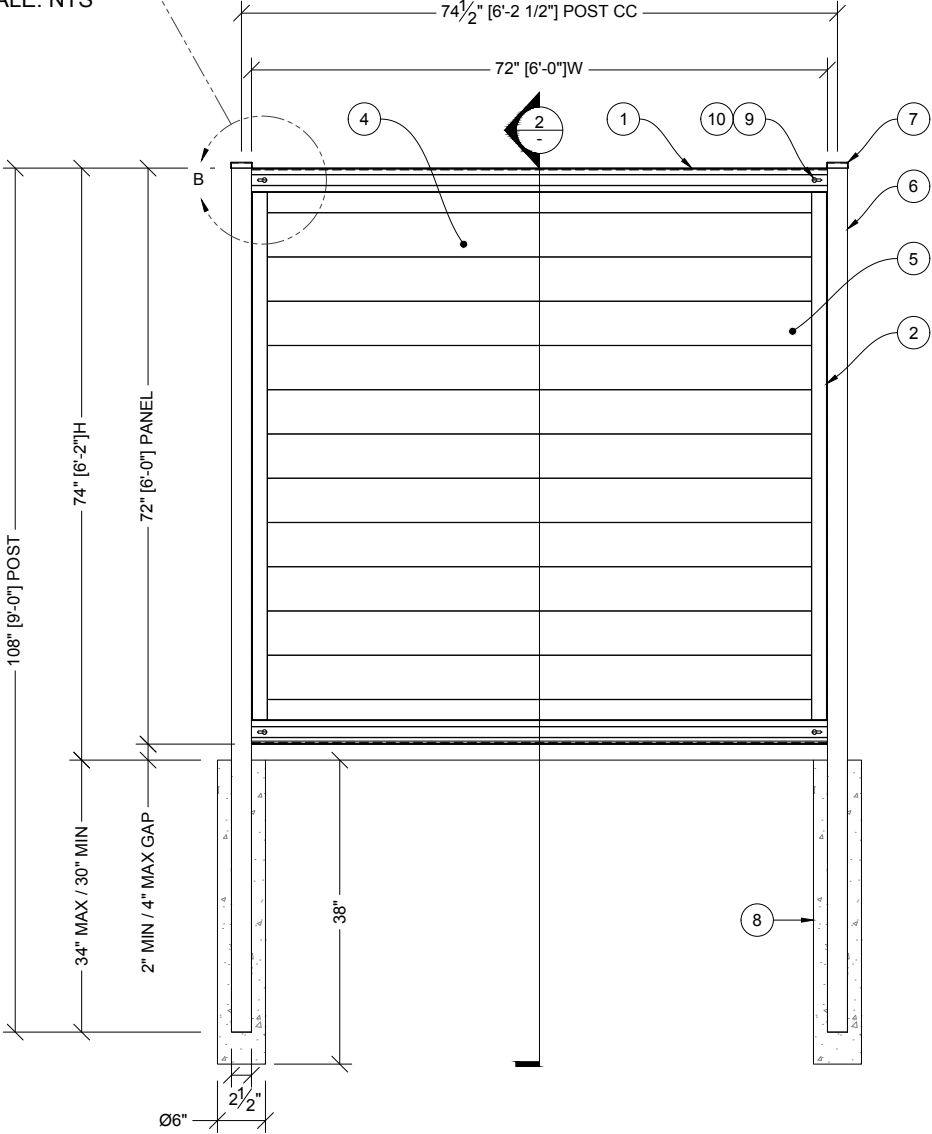
EXHIBIT #2



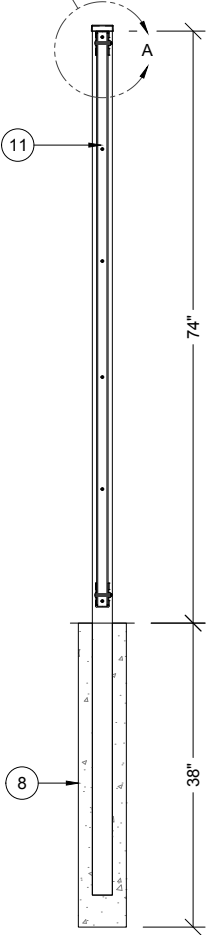
DETAIL-B
SCALE: NTS



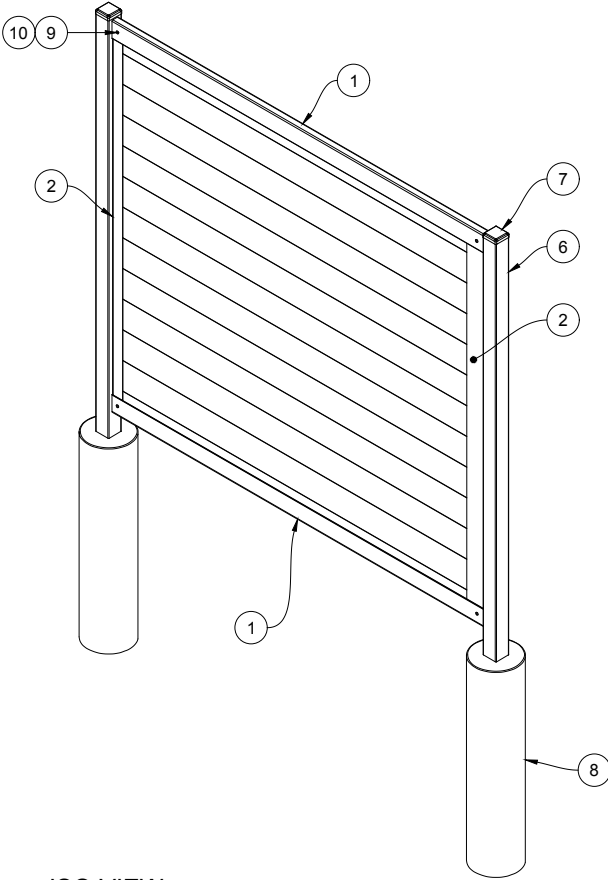
DETAIL-A
SCALE: NTS



1 FRONT VIEW
SCALE: 1/2" = 1'-0"



2 SECTION
SCALE: 1/2" = 1'-0"



3 ISO VIEW
SCALE: 1/2" = 1'-0"

BILL OF MATERIALS			
ITEM	QTY	PART NUMBER	DESCRIPTION
1	2	3" X 6'-0" CHANNEL	HORIZONTAL (TOP/BTM) GALVANIZED STEEL CHANNEL RAIL
2	2	2" X 6'-0" CHANNEL	VERTICAL GALVANIZED STEEL CHANNEL RAIL
3	N/A	CHANNELS ALLOW FOR UP TO 1" THK INFILL MATERIAL	INFILL QUANTITY MAY VARY DEPENDING ON TYPE SELECTED
4	11	1" X 6" X 6'-0" BOARDS	INFILL EXAMPLE ILLUSTRATED: HORIZONTAL BOARDS
5	2	1" X 6" X 6'-0" NOTCHED INFILL/BOARDS	TWO BOARDS TO BE NOTCHED FOR BOLT CLEARANCE AT TOP AND BTM CHANNEL
6	2	2-1/2" X 2-1/2" X 16GA, 9'-0" POST	STANDARD FENCE POST
7	2	2-1/2" X 2-1/2" CAP	2-1/2" SQ POST CAP STANDARD
8	2	6" DIAMETER CONCRETE PIER	POST PIER SIZE AND DEPTH TO BE SPECIFIED BY THE PROJECT'S EOR
9	4	ANSI B18.5 - 1/4-20 X 1-1/2" UNC CARRIAGE BOLT	ROUND HEAD SQUARE NECK CARRIAGE BOLT
10	4	IFI 100/107 1/4-20 HEX-HEAD FLANGE NUT	PREVAILING TORQUE TYPE HEX-HEAD FLANGE NUT
11	12	ANSI B18.6.4 - NO.10 X 5/8" SELF-TAPPING SCREW	VERTICAL CHANNEL, RECESSED HEAD SELF-TAPPING SCREW_TYPE-BT_TYPE-I

ABBREVIATIONS	
ANSI	AMERICAN NATIONAL STANDARDS INSTITUTE
CC	CENTER TO CENTER
EOR	ENGINEER OF RECORD
GA	GAUGE
H	HEIGHT
IFI	INDUSTRIAL FASTENER INSTITUTE
L	LENGTH
MAX	MAXIMUM
MIN	MINIMUM
NTS	NOT TO SCALE / DO NOT SCALE
SQ	SQUARE
THK	THICK / THICKNESS
TYP	TYPICAL
UNC	UNIFIED COURSE THREAD
W	WIDTH

DIMENSIONS ARE IN INCHES

UNLESS OTHERWISE SPECIFIED:
TOLERANCES:
FRACTIONAL ±1/32"
ANGULAR: MACH±1/2° BEND ±1/2°
TWO PLACE DECIMAL ±.03
THREE PLACE DECIMAL ±.005

FENCE TRAC

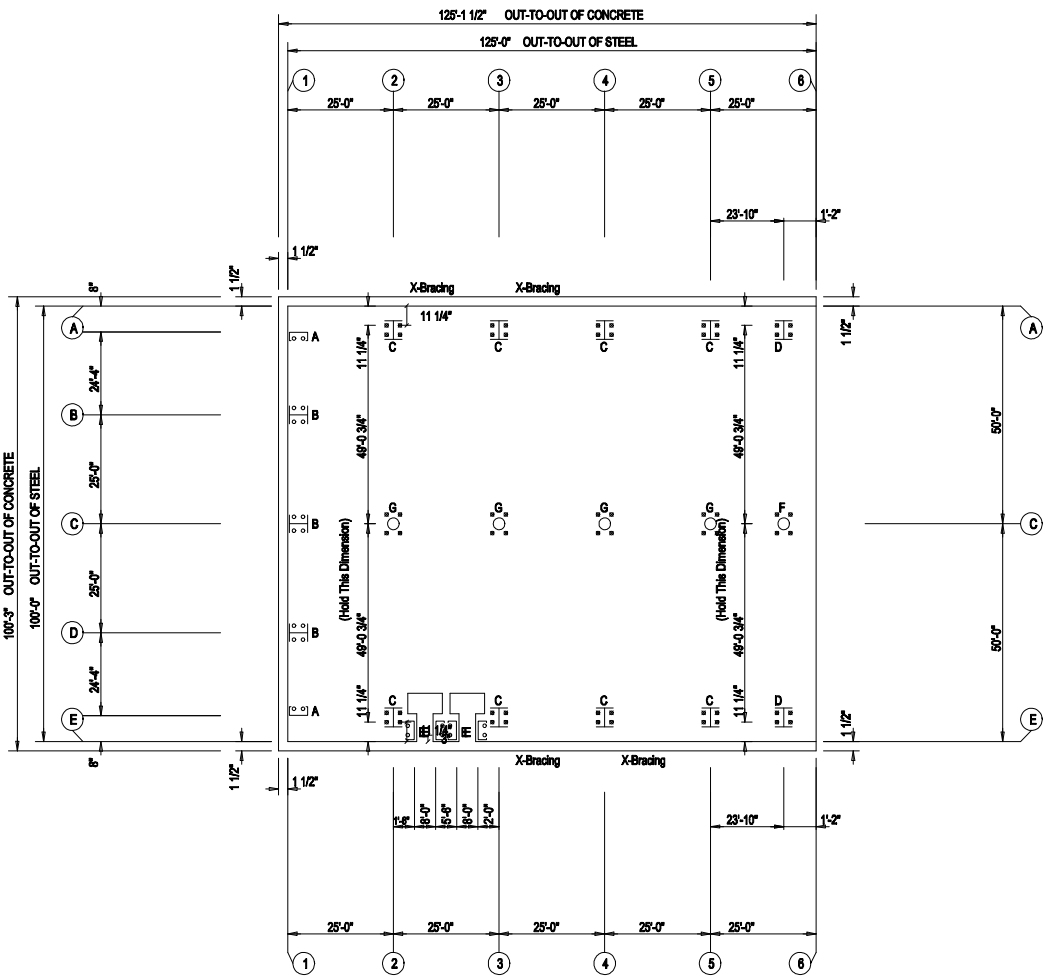
TITLE:
HORIZONTAL 6'X6' PANEL

PROPRIETARY AND CONFIDENTIAL
THE INFORMATION CONTAINED IN THIS
DRAWING IS THE SOLE PROPERTY OF FENCE
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OR AS A WHOLE WITHOUT THE WRITTEN
PERMISSION OFFENCE TRAC IS PROHIBITED.

NAME	DATE
NLI	05/30/2022
REVIS	
MFG APPR	
MATERIAL: GALVANIZED STEEL	

SIZE	DWG. NO.	REV
B	22-0530-002	A
SCALE: SEE DRAWING		SHEET 1 OF 1

EXHIBIT #3




ANCHOR BOLT PLAN
NOTE: All Base Plates @ 100'-0" (U.N.)

○ Dia= 5/8"
⊗ Dia= 3/4"

IMPORTANT!
Do Not Set Anchor Rods with these Preliminary Drawings.

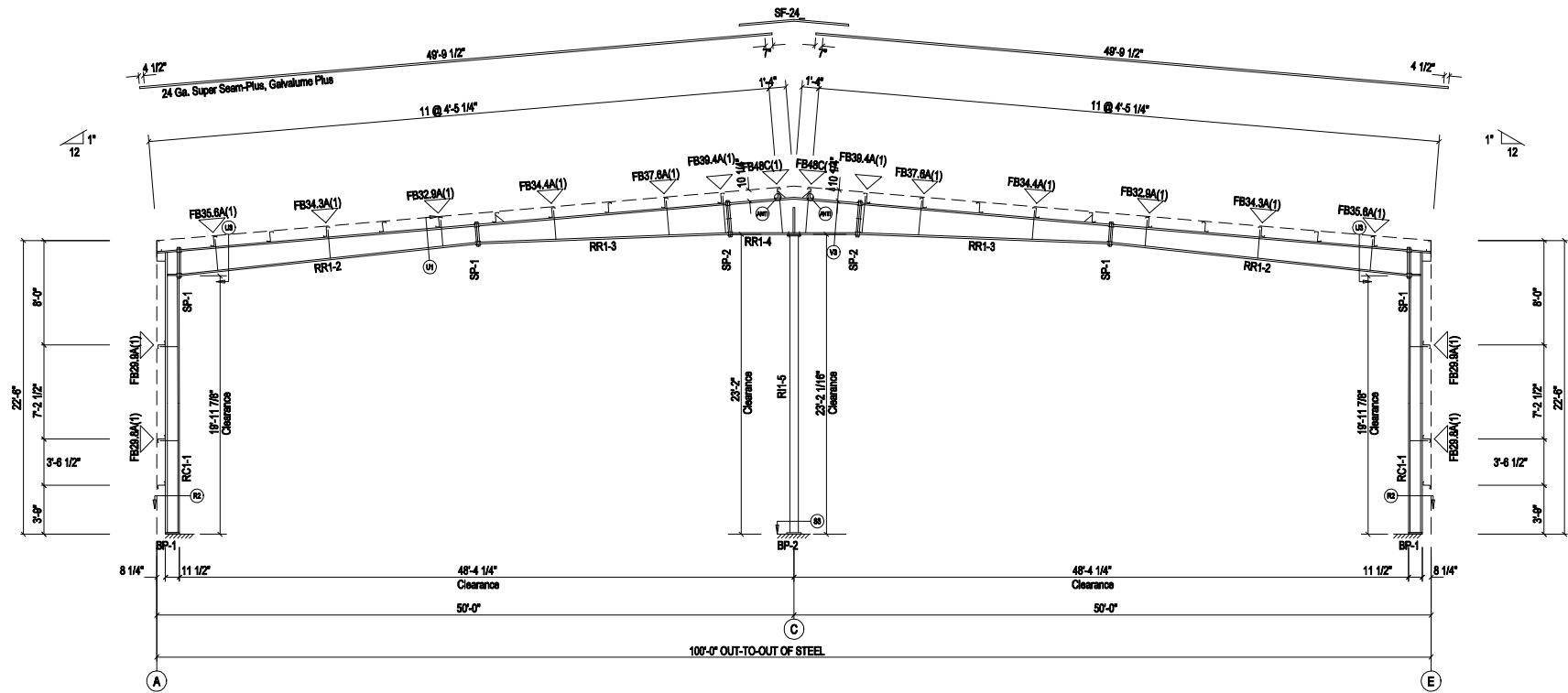
Whirlwind Steel Buildings will not be responsible for the location of the anchor rods on the Preliminary Drawings.

<p>PRELIMINARY DRAWINGS:</p> <p>These drawings are the initial plans prepared by the sales team to provide a means of communication between the customer and Whirlwind Steel Buildings, Inc. These plans are NOT intended to be used for construction.</p> <p>P.O. BOX 75280 HOUSTON, TX 77234</p> <p>PH: 800-324-9992 FAX: 832-553-4600</p> <p>© 2005 Whirlwind Steel Buildings Inc. All rights reserved</p>	 STEEL BUILDINGS	DESC: ANCHOR BOLT PLAN		BLDG. SIZE: 100'-0" x 125'-0" x 22'-6"		
		CUSTOMER: Strytorny Design		LOCATION: City, State Zip		
		REFERENCE: Georgia Mountain Food Bank				
		JOB SITE: Gainesville, GA 30507		COUNTY: Hall		
		DATE: 9/20/22	QUOTE NO: GA1078	DWG NO:	ISSUE: PRELIM	

BOLT TIGHTENING (Snug-Tight)

All bolted joints with ASTM F3125 Grade A325 bolts are specified as Snug-Tightened Joints in accordance with the Specification of Structural Joints Using High-Strength Bolts, June 11, 2020, installation as given in Section 7.1 Washers are not required for Snug-Tightened Joints using standard standard size holes per Section 6.1 of the Specification

Pretensioning methods, including Turn-of-Nut, calibrated wrench, twist-off tension control bolts or direct tension indicator are not required. Installation inspection requirements for Snug-Tight Bolt is found in Section 9.1 of the Specification.



RIGID FRAME ELEVATION: FRAME LINE 6

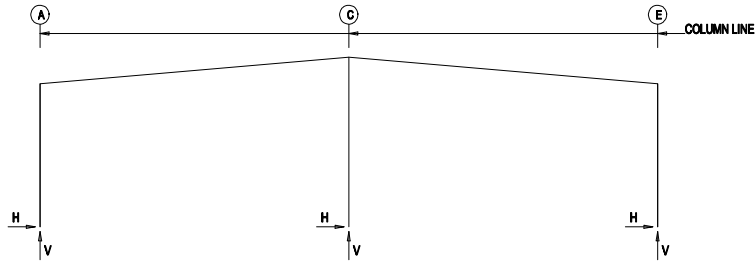
PRELIMINARY DRAWINGS:
These drawings are the initial plans prepared by the sales team to provide a means of communication between the customer and Whirlwind Steel Buildings, Inc. These plans are NOT intended to be used for construction.



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HOUSTON, TX 77234
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PH: 800-324-9992
FAX: 832-553-4600

DESC:	RIGID FRAME ELEVATION	BLDG. SIZE:	100'-0" x 125'-0" x 22'-6"
CUSTOMER:	Stintony Design	LOCATION:	City, State Zip
REFERENCE:	Georgia Mountain Food Bank		
JOB SITE:	Gainesville, GA 30507		
DATE:	9/20/22	QUOTE NO.:	GA1078
DWG NO.:		ISSUE:	PRELIM

FRAME LINES: 2 3 4 5 6



RIGID FRAME: MAXIMUM REACTIONS, ANCHOR BOLTS, & BASE PLATES

Frm Line	Col Line	Column Reactions(k)				Load Id	Hmin H	V Vmin	Bolt(in) Qty	Dia	Base Plate(in)		Thick	Elev. (in)
		Load Id	Hmax H	V Vmax	Load Id						Width	Length		
2"	A	3	5.0	15.1	8	-3.9	-2.2		4	0.750	8.000	10.50	0.375	0.0
		1	3.7	20.9	8	1.0	-6.4							
2"	E	7	3.9	-2.2	2	-5.0	15.1		4	0.750	8.000	10.50	0.375	0.0
		1	-3.7	20.9	9	-1.0	-6.4							
2"	C	5	0.0	-6.8	5	0.0	-6.8		4	0.750	12.00	12.00	0.500	0.0
		1	0.0	54.5										
2"	Frame lines:	2	3	4	5									

RIGID FRAME: MAXIMUM REACTIONS, ANCHOR BOLTS, & BASE PLATES

Frm Line	Col Line	Column Reactions(k)				Load Id	Hmin H	V Vmin	Bolt(in) Qty	Dia	Base Plate(in)		Thick	Elev. (in)
		Load Id	Hmax H	V Vmax	Load Id						Width	Length		
6	A	3	2.2	7.2	6	-2.0	-1.9		4	0.750	8.000	10.50	0.375	0.0
		1	1.5	10.0	4	-1.7	-3.0							
6	E	7	2.0	-1.9	2	-2.2	7.2		4	0.750	8.000	10.50	0.375	0.0
		1	-1.5	10.0	5	1.7	-3.0							
6	C	5	0.0	-5.7	5	0.0	-5.7		4	0.750	10.00	10.00	0.500	0.0
		1	0.0	31.1										

GENERAL NOTES

1. All anchor bolts (by others) to have nuts and flat washers.
2. All anchor bolts are designed to full S.A.E. diameters with cut threads. No substitutions are allowed.
3. The Metal Building Provider is not responsible for the design, materials and workmanship of the foundation. Anchor bolt plans prepared by the Metal Building Provider are intended to show only location, diameter, and projection of anchor bolts required to attach the Metal Building System to the foundation. The Metal Building Provider is responsible for providing to the Builder the loads imposed by the Metal Building System on the foundation. It is the responsibility of the End Customer to ensure that adequate provisions are made for specifying bolt embedment, bearing angles, tie rods, and/or other associated items embedded in the concrete foundation, as well as foundation design for the loads imposed by the Metal Building System, other imposed loads, and the bearing capacity of the soil and other conditions of the building site. This is typically the responsibility of the Design Professional or Engineer of Record, which is another reason that their involvement in the Construction Project from the outset is highly recommended. (2012 MBMA Metal Building Systems Manual, Section 3.2.2)

RIGID FRAME: BASIC COLUMN REACTIONS (k)

Frame Line	Column	---Dead---		---Collateral---		---Live---		---Snow---		---Wind_Left1---		---Wind_Right1---	
Line	Line	Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert
6	A	0.1	1.1	0.2	1.3	1.2	7.6	0.7	4.4	-3.0	-6.1	2.2	-2.2
6	E	-0.1	1.1	-0.2	1.3	-1.2	7.6	-0.7	4.4	-2.2	-6.1	3.0	-6.1
6	C	0.0	3.1	0.0	4.0	0.0	24.0	0.0	14.0	0.0	-12.6	0.0	-12.6
Frame Line	Column	---Wind_Left2---		---Wind_Right2---		---Wind_Long1---		---Wind_Long2---		---Seismic_Left---		---Seismic_Right---	
Line	Line	Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert
6	A	-3.5	-4.4	1.8	-0.4	0.8	-5.8	0.4	-3.1	-0.6	-0.3	0.8	0.3
6	E	-1.6	-0.4	3.5	-4.4	-0.4	-3.1	-0.8	-5.8	-0.6	0.3	0.6	-0.3
6	C	0.0	-8.3	0.0	-8.3	0.0	-12.0	0.0	-12.0	0.0	0.0	0.0	0.0
Frame Line	Column	---MIN_SNOW---		F1UNB_SL_L---		F1UNB_SL_R---							
Line	Line	Horiz	Vert	Horiz	Vert	Horiz	Vert						
6	A	0.8	5.1	0.5	5.4	0.5	0.6						
6	E	-0.8	5.1	-0.5	0.6	-0.5	5.4						
6	C	0.0	16.0	0.0	12.2	0.0	12.2						
Frame Line	Column	---Dead---		---Collateral---		---Live---		---Snow---		---Wind_Left1---		---Wind_Right1---	
Line	Line	Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert
2"	A	0.3	2.0	0.5	2.7	2.9	18.2	1.7	9.5	-5.4	-6.4	4.4	-3.9
2"	E	-0.3	2.0	-0.5	2.7	-2.9	18.2	-1.7	9.5	-4.4	-3.9	5.4	-6.4
2"	C	0.0	4.9	0.0	7.1	0.0	42.5	0.0	24.8	0.0	-16.3	0.0	-16.3
Frame Line	Column	---Wind_Left2---		---Wind_Right2---		---Wind_Long1---		---Wind_Long2---		---Seismic_Left---		---Seismic_Right---	
Line	Line	Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert
2"	A	-6.8	-5.8	3.1	0.0	1.3	-12.7	0.7	-9.5	-0.9	-0.4	0.9	0.4
2"	E	-3.1	0.0	6.8	-5.8	-0.7	-9.5	-1.3	-12.7	-0.9	0.4	0.9	-0.4
2"	C	0.0	-9.0	0.0	-9.0	0.0	-15.4	0.0	-15.4	0.0	0.0	0.0	0.0
Frame Line	Column	---Seismic_Long---		---MIN_SNOW---		F2UNB_SL_L---		F2UNB_SL_R---					
Line	Line	Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert				
2"	A	0.0	-2.0	1.9	10.8	1.2	11.1	1.2	1.9				
2"	E	0.0	-2.0	-1.9	10.8	-1.2	1.9	-1.2	11.1				
2"	C	0.0	0.0	0.0	28.4	0.0	21.8	0.0	21.8				
2"	Frame lines: 2 3 4 5												

PRELIMINARY DRAWINGS:
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FAX: 832-553-4600

DESC: ANCHOR BOLT REACTIONS		BLDG. SIZE: 100'-0" x 125'-0" x 22'-6"	
CUSTOMER: Stintony Design		LOCATION: City, State Zip	
REFERENCE: Georgia Mountain Food Bank			
JOB SITE: Gainesville, GA 30507		COUNTY: Hall	
DATE: 9/20/22	QUOTE NO:	DWG NO:	ISSUE: PRELIM
		GA1078	

Building addition. 100'x125' (line AA thru A)
24 ga Standing Seam Roof Galvalume
26 ga Smp walls
Design per S100
Partition Wall line AA by WW
\$165,000.00 DELIVERED EXCLUDING TAX

