PRELIMINARY STORMWATER ASSESSMENT

FOR

CAGLE MILL RESIDENTIAL AND COMMERCIAL DEVELOPMENT

4745 CAGLE MILL RD, LULA, GA 30554 4691 NOAH MARTIN DR, LULA, GA 30554



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Issued: March 22, 2024

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PROJECT DESCRIPTION

This Preliminary Hydrology Study has been prepared for Cagle Mill, residential and commercial development in Hall County, Georgia. The property has been surveyed at 72.67 acres and is owned by CBD Investments, LLC. Cornelia Highway (GA State Road #365) abuts the site from the northwest side and Cagle Mill Road from the southeast side. The North Oconee River crosses the site.

Based on FIRM Panel Maps 13139C0225F, dated September 29, 2006, The North Oconee River in our site is within a Special Flood Hazard Area Flood Zone "A" (Appendix A).

This project intends to develop the site into two different parcels, commercial and residential areas. In the residential area, there will be 235 units of single-family, 50 units of townhomes, a clubhouse, and an amenities area. The commercial space will be adjacent to Cornelia Highway and mostly contain industrial-use buildings with a visitor's parking lot.

The development is anticipated to disturb about 75 percent of the site area. More detailed explanations will be provided in the final report. The site will drain to the North Oconee River on the west side of the property, which is also part of the floodplain.

The new construction will include the installation of a new BMP as follows:

- Regenerative Stormwater Conveyance: One (1)
- Stormwater wetlands: Two (2)
- Stormwater Wet Pond: One (1)
- Enhance Swale: One (1)
- Bioretention Basins: Two (2)

The following report describes the methodology and the associated analysis results to meet the proposed developments above performance criteria.

METHODOLOGY

Hydrograph modeling and pond routing are performed using Hydraflow Hydrographs Extension for AutoCAD Civil 3D 2018. Soil Conservation Service (SCS) hydrograph method generates Type II 24-hour hydrographs for all pre-and post-developed basins during storm events. SCS runoff curve numbers are taken from the *Georgia Stormwater Management Manual (GSMM)*. Curve numbers are selected based on existing and revised proposed ground covers located on soil group B. Where ground cover is not uniform over a basin area, a weighted curve number is calculated based on the area of each type of ground cover. Time of concentration calculations for each basin (pre- and post- developed) are performed per the procedure found in USDA's Technical Release 55 *Urban Hydrology for Small Watersheds* (TR-55) and Watershed Lag Method.

HYDROLOGIC ANALYSIS

Predevelopment Conditions

The existing area of the site is primarily wooded. Existing conditions basin maps can be found in Appendix C. Runoff from 3 basins drain to Study point 1 (SP-1), as follows:

• Existing Basin "A" is 53.31 acres located northwest of the site. The maximum allowable curve number for this basin is 55. This basin has a Time Concentration of 23 minutes.

	PRE-DEV BASIN A					
Basin D	Description	Area Description	Area, A (acres)	Curve Number (CN)	CN*A	
		Woods	53.31	55	2932.1	
Α		Total Area =	53.31	Weighted CN =	55	

• Existing Basin "B" is 15.75 acres located southwest of the site. The maximum allowable curve number for this basin is 55. This basin has a Time of Concentration of 21.1 minutes.

PRE-DEV BASIN B						
				Curve Number		
Basin D	escription	Area Description	Area, A (acres)	(CN)	CN*A	
		Woods	15.75	55	866.3	
В						
		Total Area =	15.75	Weighted CN =	55	

• Existing Basin "B2" is 70 acres located southeast of the site. The maximum allowable curve number for this basin is 55. This basin has a time of Concentration of 10 minutes.

PRE-DEV BASIN B-2						
Basin D	escription	Area Description	Area. A (acres)	Curve Number (CN)	CN*A	
		Woods	70.0	55	3850	
В		Total Area =	70.0	Weighted CN =	55	

• Existing Basin "C" is 6.07 acres located southeast of the site. The maximum allowable curve number for this basin is 55. This basin has a Time of Concentration of 22.9 minutes.

PRE-DEV BASIN C						
	Curve Number					
Basin D	Description	Area Description	Area, A (acres)	(CN)	CN*A	
		Woods	6.07	55	333.9	
С						
		Total Area =	6.07	Weighted CN =	55	

 Upstream Basin area is 3176.07 acres. To determine the area of the upstream basin, we subtract the entire area to SP-1 based on StreamStats (See appendix C) to the total area of predeveloped basins A, B, and C. We also use StreamStats to determine the impervious, grass area, and wooded percentages. Based on the StreamStats report, the total area drain to SP-1 is 3251.2 acres. The time of concentration of Upstream Basin is 436.7 min. The maximum allowable curve number for Upstream Basin is 67. Below is the breakdown of each area.

UPSTREAM BASIN					
Basin Description	Area Description	Area, A (acres)	Curve Number (CN)	CN*A	
UPSTREAM	Grass (Fair Condition)	603.50	69	41641.5	
	Woods	1870.67	55	102886.9	
	Impervious	701.90	98	68786.2	
	Total Area =	3176.07	Weighted CN =	67	

Post-Development Conditions

The revised development encompasses a new commercial and residential area. Three basins are draining to SP-1 in the post-developed condition, as follows:

• Post-Basin "A" consists of 44.56 acres. Since most of the area is new residential development, per GSMM table 3.1.5-1, we use a Curve Number of 85 (1/8 acres or less [townhouse]). This basin has a Time of Concentration of 10.3 minutes.

	POST-DEV BASIN A						
Basin I	Description	Area Description	Area, A (acres)	Curve Number (CN)	CN*A		
		1/8 AC or less (Townhouses)	44.56	85	3787.6		
Α		Total Area =	44.56	Weighted CN =	85		

• Post Basin "A" (Bypass) consists of 25.8 acres. The proposed area in this basin is mostly commercial, some residential, and the bridge. We assumed additional impervious on this area is 6.5 acres which makes the curve number 70. The time of concentration of this basin is 18.4 minutes

POST DEV BASIN A (BYPASS)						
				Curve Number		
Basin D	escription	Area Description	Area, A (acres)	(CN)	CN*A	
4		Woods	19.30	61	1177.3	
		Impervious	6.50	98	637.0	
А						
		Total Area =	25.80	Weighted CN =	70	

• Post-Basin "B" consists of 67.44 acres. The majority of the area is phase 2 of the new development approx. 38 acres, the rest will be recreate space or will remain wooded. Hence, we use a Curve Number of 80. This basin has a Time of Concentration of 5 minutes.

POST-DEV BASIN B						
				Curve Number		
Basin D	escription	Area Description	Area, A (acres)	(CN)	CN*A	
		1/8 AC or less (Townhouses)	1.95	85	165.8	
В						
		Total Area =	1.95	Weighted CN =	85	

Water Quality / Runoff Reduction

According to the Georgia Stormwater Management Manual, Section 2.2.2:

"Treat the runoff from 85% of the storms that occur in an average year and reduce average annual postdevelopment total suspended solids loadings by 80%. Average rainfall events across that state of Georgia equates to treating storm events of 1.2 inches or less, as well as the first 1.2 inches of runoff for all more significant storm events.

Communities that adopt runoff reduction may waive the water quality treatment volume from this standard if 100% of the 1.0-inch runoff reduction volume is achieved."

As can be seen by referring to the TSS spreadsheet summary in Appendix F, this site does not achieve 100% Runoff Reduction (RRv), but 80% of the total TSS is removed.

Channel Protection, Overbank Flood Protection, and Extreme Flood Protection

The Channel Protection criteria are typically achieved by providing 24 hours of extended detention for runoff generated by the 1-year, 24-hour rainfall event. The overbank flood protection criteria generally are achieved by attenuating the post-development flows not to exceed the predevelopment flows for the 1, 2, 5, 10, and 25-year storm events. The extreme flood protection criteria are typically achieved by safely controlling and conveying the 100-year storm event.

It is recommended that these criteria not be required for this project due to the nature of the development and the site's proximity to the 100-year floodplain. While this site is anticipated to contribute pollutants via stormwater runoff to be treated by the water quality measures listed above, the expected increase in peak runoff flows will not be significant, and flooding will not be exacerbated. See below for a summary of peak runoff rates for all storm events and Appendix E for Hydraflow Hydrograph model results.

RUNOFF SUMMARY TABLES

The tables below summarize runoff contributions to the Study Point 1 (SP-1)

Return Frequency	PRE DEV BASIN A (Hyd 8) (CFS)	POST DEV BASIN A (Hyd 20) (CFS)	NET INCREASE (CFS)
1	292.32	295.02	2.7
2	394.61	397.57	2.96
5	586.83	590.12	3.29
10	765.47	768.95	3.48
25	1037.95	1041.59	3.64
50	1263.74	1267.44	3.7
100	1504.81	1508.51	3.7

• Post-Basin "C" consists of 2.82 acres. Since most of the area is new residential development, per GSMM table 3.1.5-1, we use a Curve Number of 85 (1/8 acres or less [townhouse]). This basin has a Time of Concentration of 9.3 minutes.

	POST-DEV BASIN C					
Basin D	Description	Area Description	Area, A (acres)	Curve Number (CN)	CN*A	
		1/8 AC or less (Townhouses)	2.82	85	239.7	
С		Total Area =	2.82	Weighted CN =	85	

• Upstream Basin remains the same under Post-Developed conditions.

Study Point(s)

Study Point 1 (SP-1)

SP-1 is located on North Oconee creek on the west side of the site. As mentioned, Basin A, B, C, and Upstream will be combined and drained to SP-1.

Delineation of all pre-developed and post-develop basins was based on StreamStats and Hall County GIS topography of the surrounding. All off-site area surfaces were determined based on aerial imagery from Hall County GIS. Below is a summary of areas contributing to each basin to compare predevelopment and post-development maps.

		Pre-Developed Drainage Areas (acres)			Post-Developed Drainage Areas (acres)			
SP	Description	Off-	On-	Total	Off-	On-	By-	Total
	Basin A		53.31	53.31		44.56	25.80	70.36
CD 1	Basin B		15.75	15.75		1.95		1.95
51-1	Basin C		6.07	6.07		2.82		2.82
	Upstream Basin	3176.07		3176.07			3176.07	3176.07
	Total Drainage Area analyzed in Study Point 1:		Pre-	3251.20			Post-	3251.20

Study Point 2 (SP-2)

SP-2 is located at the confluence of a tributary of North Oconee feeds into the creek. Basin B-2 will drain to SP-2.

Additional Information

For further information on the hydrologic analysis of this project, see the Pre/Post Development Drainage Maps and StreamStats report in Appendix C. For Time of Concentration Calculations, see Appendix D. See Appendix A for summary peak runoff rates for all storm events and Appendix E for Hydraflow Hydrograph model results.

STORMWATER MANAGEMENT DESIGN

This section describes what stormwater management measures are proposed to meet the performance criteria for the post-development condition. A BMP inspection schedule and checklist will be provided separately before approval.

DOWNSTREAM ANALYSIS

As described in the Georgia Stormwater Management Manual, the point at which the site accounts for 10% of the total drainage basin (10% point) typically reveals the most severe issues regarding the timing of hydrographs.

Since the site area (72.67 ac) is less than 10% of the total drainage area (3,251.52 ac), a downstream analysis will be located at SP-1.

CONCLUSION

This report demonstrates that the stormwater management practices designed herein will satisfy stormwater management and storm drainage requirements for the project based upon the following conclusions:

- 1) Water Quality is provided by using BMPs, reducing the Total Suspended Solids.
- 2) Peak flows are anticipated to increase due to this project, but not significantly. In addition, the site is adjacent to the Creek floodplain area. Due to these conditions, it is recommended that the Channel Protection, Overbank Flood Protection, and Extreme Flood Protection criteria be waived for this project.
- 3) No adverse downstream impacts are anticipated due to this development per hydraulic analysis.

The stormwater management facilities will continue to meet the regulatory requirements discussed herein only if the facilities are adequately maintained regularly. Any sediment or trash that accumulates in the facilities should be removed immediately.

This report and associated construction plans only pertain to the topics stated herein. The designer has not evaluated with regards to any other topic including, but not limited to: structural or geotechnical evaluations, environmental evaluations, etc.

Appendix A

Location Map



Local

BRASELTON

Web AppBuilder for ArcGIS

Esri Community Maps Contributors, © OpenStreetMap, Microsoft, Esri, HERE, Garmin, SafeGraph, GeoTechnologies, Inc, METI/NASA, USGS, EPA, NPS, US Census Bureau, USDA | GHCGIS | Hall County GIS |

NOTES TO USERS

This map is for use in administering the National Flood Insurance Program. It does not necessarily identify all areas subject to flooding, particularly from local drainage sources of small size. The **community map repository** should be consulted for possible updated or additional flood hazard information.

To obtain more detailed information in areas where **Base Flood Elevations** (BFEs) and/or **floodways** have been determined, users are encouraged to consult the Flood Profiles and Floodway Data and/or Summary of Stillwater Elevations tables contained within the Flood Insurance Study (FIS) report that accompanies this FIRM. Users should be aware that BFEs shown on the FIRM represent rounded whole-foot elevations. These BFEs are intended for flood insurance rating purposes only and should not be used as the sole source of flood elevation information. Accordingly, flood elevation data presented in the FIS report should be utilized in conjunction with the FIRM for purposes of construction and/or floodplain management.

Boundaries of the **floodways** were computed at cross sections and interpolated between cross sections. The floodways were based on hydraulic considerations with regard to requirements of the National Flood Insurance Program. Floodway widths and other pertinent floodway data are provided in the Flood Insurance Study report for this jurisdiction.

Certain areas not in Special Flood Hazard Areas may be protected by **flood control structures.** Refer to Section 2.4 "Flood Protection Measures" of the Flood Insurance Study report for information on flood control structures in this jurisdiction.

The **projection** used in the preparation of this map was Georgia State Plane West Zone (FIPS zone 1002). The **horizontal datum** was NAD 83, GRS 80 spheroid. Differences in datum, spheroid, projection or State Plane zones used in the production of FIRMs for adjacent jurisdictions may result in slight positional differences in map features across jurisdiction boundaries. These differences do not affect the accuracy of this FIRM.

Flood elevations on this map are referenced to the North American Vertical Datum of 1988. These flood elevations must be compared to structure and ground elevations referenced to the same **vertical datum**. For information regarding conversion between the National Geodetic Vertical Datum of 1929 and the North American Vertical Datum of 1988, visit the National Geodetic Survey website at <u>www.ngs.noaa.gov</u> or contact the National Geodetic Survey at the following address:

Spatial Reference System Division National Geodetic Survey, NOAA Silver Spring Metro Center 1315 East-West Highway Silver Spring, Maryland 20910 (301) 713-3191

To obtain current elevation, description, and/or location information about the **bench marks** shown on this map, please contact the Information Services Branch of the National Geodetic Survey at **(301) 713-3242**, or visit their website at <u>www.ngs.noaa.gov</u>.

Base map information shown on this FIRM was derived from Digital Orthophotos produced by Hall County, Georgia and the Center for GIS, Georgia Institute of Technology at a scale of 1:12,000 and dated 2005 or later.

The **profile baselines** depicted on this map represent the hydraulic modeling baselines that match the flood profiles in the Flood Insurance Study report. As a result of improved topographic data, the profile baseline, in some cases, may deviate significantly from the channel centerline or appear outside the Special Flood Hazard Area.

Corporate limits shown on this map are based on the best data available at the time of publication. Because changes due to annexations or deannexations may have occurred after this map was published, map users should contact appropriate community officials to verify current corporate limit locations.

Please refer to the separately printed **Map Index** for an overview map of the county showing the layout of map panels; community map repository addresses; and aListing of Communities table containing National Flood Insurance Program dates for each community as well as a listing of the panels on which each community is located.

Contact the **FEMA Map Service Center** at 1-800-358-9616 for information on available products associated with this FIRM. Available products may include previously issued Letters of Map Change, a Flood Insurance Study report, and/or digital versions of this map. The FEMA Map Service Center may also be reached by Fax at 1-800-358-9620 and their website at www.fema.gov/msc.

If you have **questions about this map** or questions concerning the National Flood Insurance Program in general, please call **1-877-FEMA MAP** (1-877-336-2627) or visit the FEMA website at <u>www.fema.gov</u>.







NOAA Atlas 14, Volume 9, Version 2 Location name: Lula, Georgia, USA* Latitude: 34.3726°, Longitude: -83.7237° Elevation: 1044 ft** * source: ESRI Maps ** source: USGS



POINT PRECIPITATION FREQUENCY ESTIMATES

Sanja Perica, Deborah Martin, Sandra Pavlovic, Ishani Roy, Michael St. Laurent, Carl Trypaluk, Dale Unruh, Michael Yekta, Geoffery Bonnin

NOAA, National Weather Service, Silver Spring, Maryland

PF_tabular | PF_graphical | Maps_&_aerials

PF tabular

PDS-b	based poir	nt precipit	ation freq	luency es	timates w	ith 90% co	onfidence	intervals	(iı
				Avera	ge recurren	ce interval (years)		
Duration	1	2	5	10	25	50	100	200	
5-min	5.04 (4.13-6.24)	5.76 (4.70-7.13)	6.95 (5.66-8.60)	7.94 (6.47-9.86)	9.34 (7.46-11.7)	10.4 (8.22-13.2)	11.5 (8.90-14.7)	12.7 (9.55-16.3)	(
10-min	3.69 (3.02-4.57)	4.22 (3.45-5.22)	5.08 (4.15-6.30)	5.81 (4.73-7.22)	6.83 (5.46-8.60)	7.63 (6.01-9.65)	8.44 (6.52-10.8)	9.27 (6.99-12.0)	(
15-min	3.00 (2.46-3.71)	3.43 (2.80-4.24)	4.13 (3.38-5.12)	4.73 (3.85-5.87)	5.56 (4.44-7.00)	6.20 (4.89-7.85)	6.86 (5.30-8.76)	7.54 (5.68-9.73)	(
30-min	2.08 (1.70-2.58)	2.39 (1.95-2.95)	2.89 (2.36-3.58)	3.31 (2.69-4.11)	3.90 (3.11-4.90)	4.35 (3.43-5.50)	4.81 (3.72-6.14)	5.28 (3.98-6.81)	(4
60-min	1.33 (1.09-1.65)	1.52 (1.24-1.88)	1.84 (1.50-2.28)	2.11 (1.72-2.62)	2.51 (2.01-3.17)	2.83 (2.24-3.59)	3.16 (2.45-4.05)	3.51 (2.66-4.55)	(2
2-hr	0.813 (0.672-0.993)	0.922 (0.762-1.13)	1.11 (0.919-1.36)	1.28 (1.06-1.58)	1.54 (1.25-1.93)	1.74 (1.39-2.19)	1.96 (1.54-2.49)	2.19 (1.68-2.82)	(
3-hr	0.610 (0.507-0.740)	0.688 (0.572-0.835)	0.828 (0.687-1.01)	0.955 (0.790-1.16)	1.15 (0.941-1.43)	1.31 (1.06-1.64)	1.48 (1.17-1.87)	1.67 (1.29-2.13)	(
6-hr	0.381 (0.320-0.456)	0.427 (0.359-0.512)	0.511 (0.429-0.613)	0.587 (0.491-0.707)	0.704 (0.584-0.870)	0.801 (0.654-0.993)	0.907 (0.726-1.14)	1.02 (0.798-1.29)	(0
12-hr	0.238 (0.203-0.282)	0.268 (0.228-0.317)	0.319 (0.270-0.378)	0.364 (0.308-0.433)	0.431 (0.360-0.523)	0.485 (0.399-0.591)	0.542 (0.438-0.668)	0.603 (0.476-0.752)	(0
24-hr	0.146 (0.126-0.171)	0.165 (0.142-0.194)	0.198 (0.169-0.231)	0.225 (0.192-0.264)	0.263 (0.221-0.314)	0.294 (0.244-0.352)	0.325 (0.264-0.394)	0.357 (0.284-0.438)	(0.
2-day	0.086 (0.075-0.099)	0.098 (0.085-0.113)	0.117 (0.101-0.135)	0.133 (0.115-0.154)	0.156 (0.132-0.183)	0.173 (0.145-0.205)	0.191 (0.157-0.229)	0.209 (0.168-0.254)	(0.
3-day	0.063 (0.055-0.073)	0.071 (0.062-0.082)	0.085 (0.074-0.097)	0.096 (0.084-0.111)	0.112 (0.096-0.131)	0.125 (0.105-0.147)	0.138 (0.114-0.164)	0.151 (0.122-0.182)	(0.
4-day	0.051 (0.045-0.058)	0.057 (0.050-0.065)	0.068 (0.059-0.077)	0.077 (0.067-0.088)	0.089 (0.077-0.104)	0.099 (0.084-0.116)	0.109 (0.091-0.129)	0.119 (0.097-0.143)	(0.
7-day	0.035 (0.031-0.039)	0.038 (0.034-0.043)	0.045 (0.040-0.051)	0.050 (0.044-0.057)	0.058 (0.050-0.067)	0.064 (0.055-0.074)	0.070 (0.059-0.082)	0.077 (0.063-0.091)	(0.
10-day	0.028 (0.025-0.031)	0.030 (0.027-0.034)	0.035 (0.031-0.040)	0.039 (0.035-0.044)	0.045 (0.039-0.051)	0.049 (0.042-0.057)	0.054 (0.046-0.063)	0.058 (0.048-0.069)	(0.
20-day	0.018 (0.017-0.020)	0.020 (0.018-0.022)	0.023 (0.021-0.026)	0.025 (0.023-0.028)	0.029 (0.025-0.032)	0.031 (0.027-0.035)	0.034 (0.029-0.039)	0.036 (0.030-0.042)	(0
30-day	0.015 (0.014-0.016)	0.016 (0.015-0.018)	0.019 (0.017-0.020)	0.020 (0.018-0.022)	0.023 (0.020-0.025)	0.024 (0.021-0.027)	0.026 (0.022-0.030)	0.028 (0.023-0.032)	(0.

45-day	0.012 (0.011-0.014)	0.014 (0.012-0.015)	0.015 (0.014-0.017)	0.017 (0.015-0.018)	0.018 (0.016-0.020)	0.020 (0.017-0.022)	0.021 (0.018-0.023)	0.022 (0.018-0.025)	(0.
60-day	0.011 (0.010-0.012)	0.012 (0.011-0.013)	0.013 (0.012-0.015)	0.015 (0.013-0.016)	0.016 (0.014-0.017)	0.017 (0.015-0.019)	0.018 (0.015-0.020)	0.018 (0.016-0.021)	(0.
¹ Precipitation frequency (PF) estimates in this table are based on frequency analysis of partial duration series (PDS). Numbers in parenthesis are PF estimates at lower and upper bounds of the 90% confidence interval. The probability that precipitation									

a given duration and average recurrence interval) will be greater than the upper bound (or less than the lower bound) is 5%. Estimates checked against probable maximum precipitation (PMP) estimates and may be higher than currently valid PMP values. Please refer to NOAA Atlas 14 document for more information.

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PF graphical



PDS-based intensity-duration-frequency (IDF) curves Latitude: 34.3726°, Longitude: -83.7237°

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Created (GMT): Thu Mar 21 22:42:18 2024

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Maps & aerials

Small scale terrain



Large scale terrain Asheville Chattanooga Chattanooga Chattanooga Atlanta* 100km Comi

Large scale map



Large scale aerial



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US Department of Commerce National Oceanic and Atmospheric Administration National Weather Service National Water Center 1325 East West Highway Silver Spring, MD 20910 Questions?: <u>HDSC.Questions@noaa.gov</u>

Disclaimer

Appendix **B**

Appendix C



E PATH: X:\FY22\1220003\05.DISC\03.01.CIVIL\STORMWATER MANAGEMENT\HYDROLOGY_V5_WJ PLOTTED BY: BONNO, ANDRES





FILE PATH: X:/FY22/1220003/05.DISC/03.01.CIVIL/STORMWATER MANAGEMENT/HYDROLOGY_V5_WJ PLOTTED BY: BONNO, ANDRES DATE: 5/1



FILE PATH: X:\FY22\1220003\05.DISC\03.01.CIVIL\STORMWATER MANAGEMENT\HYDROLOGY_V5_WJ PLOTTED BY: BONNO, ANDRES DATE: 5/1

Cagle Mill Phase 2- StreamStats Report

 Region ID:
 GA

 Workspace ID:
 GA20240228150937256000

 Clicked Point (Latitude, Longitude):
 34.36732, -83.72267

 Time:
 2024-02-28 10:09:59 -0500



Collapse All

Basin Characteristics									
Parameter Code	Parameter Description	Value	Unit						
BSLDEM10M	Mean basin slope computed from 10 m DEM	15.896	percent						
CSL10_85	Change in elevation divided by length between points 10 and 85 percent of distance along main channel to basin divide - main channel method not known	315	feet per mi						
DRNAREA	Area that drains to a point on a stream	0.12	square miles						
ELEV	Mean Basin Elevation	1180	feet						

StreamStats

Parameter Code	Parameter Description	Value	Unit
ELEVMAX	Maximum basin elevation	1261	feet
GWHEAD	Mean basin elevation minus minimum basin elevation	106	feet
I24H100Y	Maximum 24-hour precipitation that occurs on average once in 100 years	7.76	inches
I24H10Y	Maximum 24-hour precipitation that occurs on average once in 10 years	5.38	inches
I24H25Y	Maximum 24-hour precipitation that occurs on average once in 25 years	6.3	inches
I24H50Y	Maximum 24-hour precipitation that occurs on average once in 50 years	7.02	inches
LC06AGRI	Percent agriculture computed as total of grass, pasture, and crops, NLCD classes 71, 81 and 82	1.153	percent
LC06DEV	Percentage of land-use from NLCD 2006 classes 21-24	12.536	percent
LC06FOREST	Percentage of forest from NLCD 2006 classes 41-43	85.253	percent
LC06IMP	Percentage of impervious area determined from NLCD 2006 impervious dataset	1.12	percent
LC11DEV	Percentage of developed (urban) land from NLCD 2011 classes 21-24	13.1	percent
LC11IMP	Average percentage of impervious area determined from NLCD 2011 impervious dataset	1.49	percent
MINBELEV	Minimum basin elevation	1074	feet
PCTREG1	Percentage of drainage area located in Region 1 - Piedmont / Ridge and Valley	100	percent
PCTREG2	Percentage of drainage area located in Region 2 - Blue Ridge	0	percent
PCTREG3	Percentage of drainage area located in Region 3 - Sandhills	0	percent
PCTREG4	Percentage of drainage area located in Region 4 - Coastal Plains	0	percent
PCTREG5	Percentage of drainage area located in Region 5 - Lower Tifton Uplands	0	percent

StreamStats

Parameter Code	Parameter Description	Value	Unit
PRECPRIS00	Basin average mean annual precipitation for 1971 to 2000 from PRISM	57.2	inches
RELIEF	Maximum - minimum elevation	187	feet
RRMEAN	Relief ratio defined as (ELEV- MINBELEV)/(ELEVMAX-MINBELEV)	0.569	dimensionless

USGS Data Disclaimer: Unless otherwise stated, all data, metadata and related materials are considered to satisfy the quality standards relative to the purpose for which the data were collected. Although these data and associated metadata have been reviewed for accuracy and completeness and approved for release by the U.S. Geological Survey (USGS), no warranty expressed or implied is made regarding the display or utility of the data for other purposes, nor on all computer systems, nor shall the act of distribution constitute any such warranty.

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Application Version: 4.19.4 StreamStats Services Version: 1.2.22 NSS Services Version: 2.2.1





	MAP L	EGEND		MAP INFORMATION
Area of Int	erest (AOI) Area of Interest (AOI)	8	Spoil Area Stony Spot	The soil surveys that comprise your AOI were mapped at 1:20,000.
Soils	Soil Map Unit Polygons Soil Map Unit Lines	© ∀	Very Stony Spot Wet Spot	Please rely on the bar scale on each map sheet for map measurements.
Special	Soil Map Unit Points Point Features	۵ ••	Other Special Line Features	Source of Map: Natural Resources Conservation Service Web Soil Survey URL: Coordinate System: Web Mercator (EPSG:3857)
9 8	Blowout Borrow Pit	Water Fea	tures Streams and Canals ation	Maps from the Web Soil Survey are based on the Web Mercator projection, which preserves direction and shape but distorts distance and area. A projection that preserves area, such as the
☆ ※	Closed Depression Gravel Pit	~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~	Rails Interstate Highways US Routes	Albers equal-area conic projection, should be used if more accurate calculations of distance or area are required. This product is generated from the USDA-NRCS certified data as
:. ©	Gravelly Spot Landfill	~	Major Roads Local Roads	of the version date(s) listed below. Soil Survey Area: Barrow, Hall, and Jackson Counties, Georgia
∧ ⊸ ⊗	Lava Flow Marsh or swamp Mine or Quarry	Backgrou	nd Aerial Photography	Survey Area Data: Version 18, Aug 30, 2023 Soil map units are labeled (as space allows) for map scales 1:50,000 or larger.
0	Miscellaneous Water Perennial Water			Date(s) aerial images were photographed: Mar 19, 2022—Apr 20, 2022
× + ∷	Rock Outcrop Saline Spot Sandy Spot			The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background imagery displayed on these maps. As a result, some minor shifting of map unit houndaries may be evident
⊕ ◊	Severely Eroded Spot Sinkhole			
¢ Ø	Slide or Slip Sodic Spot			

Map U	Init	Legend
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Map Unit Symbol	Map Unit Name	Acres in AOI	Percent of AOI
Сс	Cartecay and Chewacla soils	18.0	2.5%
MdD	Madison sandy loam, 10 to 15 percent slopes	9.6	1.3%
MdE	Madison sandy loam, 15 to 25 percent slopes	232.6	32.5%
MIC2	Madison sandy clay loam, 6 to 10 percent slopes, moderately eroded	19.6	2.7%
MID2	Madison sandy clay loam, 10 to 15 percent slopes, moderately eroded	225.0	31.5%
MuF	Musella cobbly clay loam, 15 to 35 percent slopes	54.2	7.6%
PaE	Pacolet sandy loam, 15 to 25 percent slopes	25.7	3.6%
PTF	Pacolet-Tallapoosa association, steep	66.0	9.2%
То	Toccoa soils	63.6	8.9%
W	Water	0.8	0.1%
Totals for Area of Interest		715.0	100.0%

Map Unit Descriptions

The map units delineated on the detailed soil maps in a soil survey represent the soils or miscellaneous areas in the survey area. The map unit descriptions, along with the maps, can be used to determine the composition and properties of a unit.

A map unit delineation on a soil map represents an area dominated by one or more major kinds of soil or miscellaneous areas. A map unit is identified and named according to the taxonomic classification of the dominant soils. Within a taxonomic class there are precisely defined limits for the properties of the soils. On the landscape, however, the soils are natural phenomena, and they have the characteristic variability of all natural phenomena. Thus, the range of some observed properties may extend beyond the limits defined for a taxonomic class. Areas of soils of a single taxonomic class rarely, if ever, can be mapped without including areas of other taxonomic classes. Consequently, every map unit is made up of the soils or miscellaneous areas for which it is named and some minor components that belong to taxonomic classes other than those of the major soils.

Most minor soils have properties similar to those of the dominant soil or soils in the map unit, and thus they do not affect use and management. These are called noncontrasting, or similar, components. They may or may not be mentioned in a particular map unit description. Other minor components, however, have properties and behavioral characteristics divergent enough to affect use or to require different

management. These are called contrasting, or dissimilar, components. They generally are in small areas and could not be mapped separately because of the scale used. Some small areas of strongly contrasting soils or miscellaneous areas are identified by a special symbol on the maps. If included in the database for a given area, the contrasting minor components are identified in the map unit descriptions along with some characteristics of each. A few areas of minor components may not have been observed, and consequently they are not mentioned in the descriptions, especially where the pattern was so complex that it was impractical to make enough observations to identify all the soils and miscellaneous areas on the landscape.

The presence of minor components in a map unit in no way diminishes the usefulness or accuracy of the data. The objective of mapping is not to delineate pure taxonomic classes but rather to separate the landscape into landforms or landform segments that have similar use and management requirements. The delineation of such segments on the map provides sufficient information for the development of resource plans. If intensive use of small areas is planned, however, onsite investigation is needed to define and locate the soils and miscellaneous areas.

An identifying symbol precedes the map unit name in the map unit descriptions. Each description includes general facts about the unit and gives important soil properties and qualities.

Soils that have profiles that are almost alike make up a *soil series*. Except for differences in texture of the surface layer, all the soils of a series have major horizons that are similar in composition, thickness, and arrangement.

Soils of one series can differ in texture of the surface layer, slope, stoniness, salinity, degree of erosion, and other characteristics that affect their use. On the basis of such differences, a soil series is divided into *soil phases*. Most of the areas shown on the detailed soil maps are phases of soil series. The name of a soil phase commonly indicates a feature that affects use or management. For example, Alpha silt loam, 0 to 2 percent slopes, is a phase of the Alpha series.

Some map units are made up of two or more major soils or miscellaneous areas. These map units are complexes, associations, or undifferentiated groups.

A *complex* consists of two or more soils or miscellaneous areas in such an intricate pattern or in such small areas that they cannot be shown separately on the maps. The pattern and proportion of the soils or miscellaneous areas are somewhat similar in all areas. Alpha-Beta complex, 0 to 6 percent slopes, is an example.

An *association* is made up of two or more geographically associated soils or miscellaneous areas that are shown as one unit on the maps. Because of present or anticipated uses of the map units in the survey area, it was not considered practical or necessary to map the soils or miscellaneous areas separately. The pattern and relative proportion of the soils or miscellaneous areas are somewhat similar. Alpha-Beta association, 0 to 2 percent slopes, is an example.

An *undifferentiated group* is made up of two or more soils or miscellaneous areas that could be mapped individually but are mapped as one unit because similar interpretations can be made for use and management. The pattern and proportion of the soils or miscellaneous areas in a mapped area are not uniform. An area can be made up of only one of the major soils or miscellaneous areas, or it can be made up of all of them. Alpha and Beta soils, 0 to 2 percent slopes, is an example.

Some surveys include *miscellaneous areas*. Such areas have little or no soil material and support little or no vegetation. Rock outcrop is an example.

Appendix D

Hyd. No. 1

PRE-DEV BASIN A

Description	<u>A</u>		<u>B</u>		<u>C</u>		<u>Totals</u>
Sheet Flow Manning's n-value Flow length (ft) Two-year 24-hr precip. (in) Land slope (%)	= 0.150 = 150.0 = 2.20 = 5.00		0.011 0.0 0.00 0.00		0.011 0.0 0.00 0.00		
Travel Time (min)	= 11.33	+	0.00	+	0.00	=	11.33
Shallow Concentrated Flow Flow length (ft) Watercourse slope (%) Surface description Average velocity (ft/s)	= 1600.00 = 2.00 = Unpaved =2.28	b	0.00 0.00 Paved 0.00		0.00 0.00 Paved 0.00		
Travel Time (min)	= 11.69	+	0.00	+	0.00	=	11.69
Channel Flow X sectional flow area (sqft) Wetted perimeter (ft) Channel slope (%) Manning's n-value Velocity (ft/s)	= 0.00 = 0.00 = 0.00 = 0.015 =0.00		0.00 0.00 0.00 0.015 0.00		0.00 0.00 0.00 0.015 0.00		
Flow length (ft)	({0})0.0		0.0		0.0		
Travel Time (min)	= 0.00	+	0.00	+	0.00	=	0.00
Total Travel Time, Tc							23.00 min

Hyd. No. 2

PRE-DEV-BASIN B

<u>Description</u>	A		<u>B</u>		<u>C</u>		<u>Totals</u>
Sheet Flow Manning's n-value Flow length (ft) Two-year 24-hr precip. (in) Land slope (%)	= 0.240 = 300.0 = 2.20 = 13.00		0.011 0.0 0.00 0.00		0.011 0.0 0.00 0.00		40.00
Travel Time (min)	= 19.60	+	0.00	+	0.00	=	19.60
Shallow Concentrated Flow Flow length (ft) Watercourse slope (%) Surface description Average velocity (ft/s)	= 260.00 = 12.00 = Unpaved =5.59	d	0.00 0.00 Paved 0.00		0.00 0.00 Paved 0.00		
Travel Time (min)	= 0.78	+	0.00	+	0.00	=	0.78
Channel Flow X sectional flow area (sqft) Wetted perimeter (ft) Channel slope (%) Manning's n-value Velocity (ft/s)	= 4.00 = 8.00 = 10.00 = 0.025 =11.85		0.00 0.00 0.00 0.015 0.00		0.00 0.00 0.00 0.015 0.00		
Flow length (ft)	({0})500.0		0.0		0.0		
Travel Time (min)	= 0.70	+	0.00	+	0.00	=	0.70
Total Travel Time, Tc							21.10 min

Hyd. No. 3

PRE-DEV-BASIN C

Description	A		<u>B</u>		<u>C</u>		<u>Totals</u>
Sheet Flow Manning's n-value Flow length (ft) Two-year 24-hr precip. (in) Land slope (%)	= 0.240 = 100.0 = 2.20 = 10.00		0.011 0.0 0.00 0.00		0.011 0.0 0.00 0.00		
Travel Time (min)	= 9.04	+	0.00	+	0.00	=	9.04
Shallow Concentrated Flow Flow length (ft) Watercourse slope (%) Surface description Average velocity (ft/s)	= 600.00 = 13.00 = Unpave =5.82	d	0.00 0.00 Paved 0.00		0.00 0.00 Paved 0.00		
Travel Time (min)	= 1.72	+	0.00	+	0.00	=	1.72
Channel Flow X sectional flow area (sqft) Wetted perimeter (ft) Channel slope (%) Manning's n-value Velocity (ft/s)	= 0.00 = 0.00 = 0.00 = 0.015 =0.00		0.00 0.00 0.00 0.015 0.00		0.00 0.00 0.00 0.015 0.00		
Flow length (ft)	({0})0.0		0.0		0.0		
Travel Time (min)	= 0.00	+	0.00	+	0.00	=	0.00
Total Travel Time, Tc							10.80 min

Hyd. No. 11

POST-DEV BASIN A

Description	<u>A</u>		<u>B</u>		<u>C</u>		<u>Totals</u>
Sheet Flow Manning's n-value Flow length (ft) Two-year 24-hr precip. (in) Land slope (%)	= 0.011 = 150.0 = 3.89 = 5.00		0.011 0.0 0.00 0.00		0.011 0.0 0.00 0.00		
Travel Time (min)	= 1.05	+	0.00	+	0.00	=	1.05
Shallow Concentrated Flow Flow length (ft) Watercourse slope (%) Surface description Average velocity (ft/s)	= 1600.00 = 2.00 = Paved =2.87		0.00 0.00 Paved 0.00		0.00 0.00 Paved 0.00		
Travel Time (min)	= 9.28	+	0.00	+	0.00	=	9.28
Channel Flow X sectional flow area (sqft) Wetted perimeter (ft) Channel slope (%) Manning's n-value Velocity (ft/s)	= 2.00 = 4.00 = 5.00 = 0.015 =13.96		0.00 0.00 0.00 0.015 0.00		0.00 0.00 0.00 0.015 0.00		
Flow length (ft)	({0})0.0		0.0		0.0		
Travel Time (min)	= 0.00	+	0.00	+	0.00	=	0.00
Total Travel Time, Tc							

Hyd. No. 12

POST-DEV BASIN A (BYPASS)

<u>Description</u>	<u>A</u>		<u>B</u>		<u>C</u>		<u>Totals</u>
Sheet Flow Manning's n-value Flow length (ft) Two-year 24-hr precip. (in) Land slope (%)	= 0.150 = 150.0 = 3.89 = 5.00		0.011 0.0 0.00 0.00		0.011 0.0 0.00 0.00		
Travel Time (min)	= 8.52	+	0.00	+	0.00	=	8.52
Shallow Concentrated Flow Flow length (ft) Watercourse slope (%) Surface description Average velocity (ft/s)	= 1350.00 = 2.00 = Unpaved =2.28	ł	0.00 0.00 Paved 0.00		0.00 0.00 Paved 0.00		
Travel Time (min)	= 9.86	+	0.00	+	0.00	=	9.86
Channel Flow X sectional flow area (sqft) Wetted perimeter (ft) Channel slope (%) Manning's n-value Velocity (ft/s)	= 2.00 = 4.00 = 10.00 = 0.150 =1.97		0.00 0.00 0.00 0.015 0.00		0.00 0.00 0.00 0.015 0.00		
Flow length (ft)	({0})0.0		0.0		0.0		
Travel Time (min)	= 0.00	+	0.00	+	0.00	=	0.00
Total Travel Time, Tc							
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

Hyd. No. 14

POST-DEV BASIN C

Description	<u>A</u>		<u>B</u>		<u>C</u>		<u>Totals</u>
Sheet Flow Manning's n-value Flow length (ft) Two-year 24-hr precip. (in) Land slope (%)	= 0.150 = 150.0 = 3.89 = 5.00		0.011 0.0 0.00 0.00		0.011 0.0 0.00 0.00		
Travel Time (min)	= 8.52	+	0.00	+	0.00	=	8.52
Shallow Concentrated Flow Flow length (ft) Watercourse slope (%) Surface description Average velocity (ft/s)	= 220.00 = 5.00 = Paved =4.55		0.00 0.00 Paved 0.00		0.00 0.00 Paved 0.00		
Travel Time (min)	= 0.81	+	0.00	+	0.00	=	0.81
Channel Flow X sectional flow area (sqft) Wetted perimeter (ft) Channel slope (%) Manning's n-value Velocity (ft/s)	= 0.00 = 0.00 = 0.015 =0.00		0.00 0.00 0.00 0.015 0.00		0.00 0.00 0.00 0.015 0.00		
Flow length (ft)	({0})0.0		0.0		0.0		
Travel Time (min)	= 0.00	+	0.00	+	0.00	=	0.00
Total Travel Time, Tc						9.30 min	

Appendix E

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

Hyd. No. 21

POST-DEV BASIN B

Hydrograph type =	SCS Runoff	Peak discharge	= 187.09 cfs
Storm frequency =	1 yrs	Time to peak	= 11.97 hrs
Time interval =	2 min	Hyd. volume	= 377,378 cuft
Drainage area =	67.440 ac	Curve number	= 80*
Basin Slope =	0.0 %	Hydraulic length	= 0 ft
Tc method =	User	Time of conc. (Tc)	= 5.00 min
Total precip. =	3.51 in	Distribution	= Type II
Storm duration =	24 hrs	Shape factor	= 484

* Composite (Area/CN) = [(13.210 x 55) + (16.000 x 65) + (38.230 x 95)] / 67.440



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Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

Hyd. No. 22

POST-DEV BASINB (BYPASS)

Hydrograph type	= SCS Runoff	Peak discharge	= 16.94 cfs
Storm frequency	= 1 yrs	Time to peak	= 11.97 hrs
Time interval	= 2 min	Hyd. volume	= 34,172 cuft
Drainage area	= 9.900 ac	Curve number	= 70*
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= User	Time of conc. (Tc)	= 5.00 min
Total precip.	= 3.51 in	Distribution	= Type II
Storm duration	= 24 hrs	Shape factor	= 484

* Composite (Area/CN) = [(3.750 x 95) + (6.150 x 55)] / 9.900



Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

Hyd. No. 23

POST-BASIN B POND

Hydrograph type	= Reservoir	Peak discharge	= 27.08 cfs
Storm frequency	= 1 yrs	Time to peak	= 12.17 hrs
Time interval	= 2 min	Hyd. volume	= 377,329 cuft
Inflow hyd. No.	= 21 - POST-DEV BASIN B	Max. Elevation	= 1083.43 ft
Reservoir name	= WET POND 2	Max. Storage	= 156,233 cuft

Storage Indication method used.



Pond Report

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

Pond No. 4 - WET POND 2

Pond Data

Contours -User-defined contour areas. Conic method used for volume calculation. Begining Elevation = 1080.00 ft

Stage / Storage Table

Elevation (ft)	Contour area (sqft)	Incr. Storage (cuft)	Total storage (cuft)
1080.00	40,453	0	0
1081.00	43,362	41,895	41,895
1082.00	46,333	44,835	86,730
1083.00	49,368	47,838	134,567
1084.00	52,465	50,904	185,471
1085.00	55,624	54,031	239,502
1086.00	62,132	58,842	298,345
1087.00	68,890	65,475	363,820
1088.00	75,899	72,359	436,179
1089.00	83,159	79,494	515,672
	Elevation (ft) 1080.00 1081.00 1082.00 1083.00 1084.00 1085.00 1086.00 1087.00 1088.00 1089.00	Elevation (ft)Contour area (sqft)1080.0040,4531081.0043,3621082.0046,3331083.0049,3681084.0052,4651085.0055,6241086.0062,1321087.0068,8901088.0075,8991089.0083,159	Elevation (ft)Contour area (sqft)Incr. Storage (cuft)1080.0040,45301081.0043,36241,8951082.0046,33344,8351083.0049,36847,8381084.0052,46550,9041085.0055,62454,0311086.0062,13258,8421087.0068,89065,4751088.0075,89972,3591089.0083,15979,494

Culvert / Orifice Structures

Weir Structures

	[A]	[B]	[C]	[PrfRsr]		[A]	[B]	[C]	[D]
Rise (in)	= 42.00	0.00	0.00	0.00	Crest Len (ft)	Inactive	Inactive	0.00	35.00
Span (in)	= 42.00	0.00	0.00	0.00	Crest El. (ft)	= 1087.00	1083.50	0.00	1088.50
No. Barrels	= 1	0	0	0	Weir Coeff.	= 3.33	3.33	3.33	3.33
Invert El. (ft)	= 1080.00	0.00	0.00	0.00	Weir Type	= 1	Rect		Ciplti
Length (ft)	= 20.00	0.00	0.00	0.00	Multi-Stage	= Yes	Yes	No	No
Slope (%)	= 1.00	0.00	0.00	n/a					
N-Value	= .013	.013	.013	n/a					
Orifice Coeff.	= 0.60	0.60	0.60	0.60	Exfil.(in/hr)	= 0.000 (by	Wet area)		
Multi-Stage	= n/a	No	No	No	TW Elev. (ft)	= 0.00			

Note: Culvert/Orifice outflows are analyzed under inlet (ic) and outlet (oc) control. Weir risers checked for orifice conditions (ic) and submergence (s). Stage / Storage / Discharge Table

-	-	-											
Stage ft	Storage cuft	Elevation ft	Clv A cfs	Clv B cfs	Clv C cfs	PrfRsr cfs	Wr A cfs	Wr B cfs	Wr C cfs	Wr D cfs	Exfil cfs	User cfs	Total cfs
0.00	0	1080.00	0.00				0.00	0.00		0.00			0.000
1.00	41,895	1081.00	6.26 oc				0.00	0.00		0.00			6.256
2.00	86,730	1082.00	16.11 oc				0.00	0.00		0.00			16.11
3.00	134,567	1083.00	24.98 oc				0.00	0.00		0.00			24.98
4.00	185,471	1084.00	50.79 oc				0.00	0.00		0.00			50.79
5.00	239,502	1085.00	79.15 oc				0.00	0.00		0.00			79.15
6.00	298,345	1086.00	95.49 ic				0.00	0.00		0.00			95.49
7.00	363,820	1087.00	106.13 ic				0.00	0.00		0.00			106.13
8.00	436,179	1088.00	115.80 ic				0.00	0.00		0.00			115.80
9.00	515,672	1089.00	124.72 ic				0.00	0.00		41.21			165.93

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Hyd. No. 24

POST-DEV BASIN B TO SP-2

Hydrograph type Storm frequency	= Reach = 1 vrs	Peak discharge Time to peak	= 24.41 cfs = 12.93 hrs
Time interval	= 2 min	Hyd. volume	= 376,886 cuft
Inflow hyd. No.	= 23 - POST-BASIN B POND	Section type	= Trapezoidal
Reach length	= 1295.0 ft	Channel slope	= 4.0 %
Manning's n	= 0.150	Bottom width	= 18.0 ft
Side slope	= 10.0:1	Max. depth	= 2.0 ft
Rating curve x	= 0.289	Rating curve m	= 1.281
Ave. velocity	= 0.78 ft/s	Routing coeff.	= 0.0887

Modified Att-Kin routing method used.



Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

Hyd. No. 26

POST-DEV AT SP-2

Hydrograph type Storm frequency	= Combine = 1 vrs	Peak discharge Time to peak	= 25.56 cfs = 12.90 brs
Time interval	$= 2 \min$	Hyd. volume	= 411,058 cuft
Inflow hyds.	= 22, 24	Contrib. drain. area	= 9.900 ac



Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

Hyd. No. 27

PRE DEV-BASIN B2

Hydrograph type	= SCS Runoff	Peak discharge	= 22.32 cfs
Storm frequency	= 1 yrs	Time to peak	= 12.03 hrs
Time interval	= 1 min	Hyd. volume	= 88,707 cuft
Drainage area	= 70.000 ac	Curve number	= 55
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= User	Time of conc. (Tc)	= 10.00 min
Total precip.	= 3.51 in	Distribution	= Type II
Storm duration	= 24 hrs	Shape factor	= 484



Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

Hyd. No. 21

POST-DEV BASIN B

Hydrograph type =	SCS Runoff	Peak discharge	= 229.10 cfs
Storm frequency =	2 yrs	Time to peak	= 11.93 hrs
Time interval =	2 min	Hyd. volume	= 462,892 cuft
Drainage area =	67.440 ac	Curve number	= 80*
Basin Slope =	0.0 %	Hydraulic length	= 0 ft
Tc method =	User	Time of conc. (Tc)	= 5.00 min
Total precip. =	3.97 in	Distribution	= Type II
Storm duration =	24 hrs	Shape factor	= 484

* Composite (Area/CN) = [(13.210 x 55) + (16.000 x 65) + (38.230 x 95)] / 67.440



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Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

Hyd. No. 22

POST-DEV BASINB (BYPASS)

Hydrograph type =	= SCS Runoff	Peak discharge	= 22.03 cfs
Storm frequency :	= 2 yrs	Time to peak	= 11.97 hrs
Time interval	= 2 min	Hyd. volume	= 44,125 cuft
Drainage area	= 9.900 ac	Curve number	= 70*
Basin Slope :	= 0.0 %	Hydraulic length	= 0 ft
Tc method =	= User	Time of conc. (Tc)	= 5.00 min
Total precip.	= 3.97 in	Distribution	= Type II
Storm duration	= 24 hrs	Shape factor	= 484

* Composite (Area/CN) = [(3.750 x 95) + (6.150 x 55)] / 9.900



Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

Hyd. No. 23

POST-BASIN B POND

Hydrograph type	= Reservoir	Peak discharge	= 52.76 cfs
Storm frequency	= 2 yrs	Time to peak	= 12.07 hrs
Time interval	= 2 min	Hyd. volume	= 462,843 cuft
Inflow hyd. No.	= 21 - POST-DEV BASIN B	Max. Elevation	= 1084.06 ft
Reservoir name	= WET POND 2	Max. Storage	= 188,500 cuft

Storage Indication method used.



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Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

Wednesday, 02 / 28 / 2024

Hyd. No. 24

POST-DEV BASIN B TO SP-2

Hydrograph type Storm frequency	= Reach = 2 yrs	Peak discharge Time to peak	= 36.54 cfs = 12.47 hrs
Time interval	= 2 min	Hyd. volume	= 462,400 cuft
Inflow hyd. No.	= 23 - POST-BASIN B POND	Section type	= Trapezoidal
Reach length	= 1295.0 ft	Channel slope	= 4.0 %
Manning's n	= 0.150	Bottom width	= 18.0 ft
Side slope	= 10.0:1	Max. depth	= 2.0 ft
Rating curve x	= 0.289	Rating curve m	= 1.281
Ave. velocity	= 0.90 ft/s	Routing coeff.	= 0.1019

Modified Att-Kin routing method used.



Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

Hyd. No. 26

POST-DEV AT SP-2

Hydrograph type	= Combine	Peak discharge	= 38.84 cfs
Storm frequency	= 2 yrs	Time to peak	= 12.43 hrs
Time interval	= 2 min	Hyd. volume	= 506,525 cuft
Inflow hyds.	= 22, 24	Contrib. drain. area	= 9.900 ac



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Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

Hyd. No. 27

PRE DEV-BASIN B2

Hydrograph type	= SCS Runoff	Peak discharge	= 41.71 cfs
Storm frequency	= 2 yrs	Time to peak	= 12.02 hrs
Time interval	= 1 min	Hyd. volume	= 131,594 cuft
Drainage area	= 70.000 ac	Curve number	= 55
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= User	Time of conc. (Tc)	= 10.00 min
Total precip.	= 3.97 in	Distribution	= Type II
Storm duration	= 24 hrs	Shape factor	= 484



Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

Hyd. No. 21

POST-DEV BASIN B

Hydrograph type =	SCS Runoff	Peak discharge	= 301.57 cfs
Storm frequency =	5 yrs	Time to peak	= 11.93 hrs
Time interval =	2 min	Hyd. volume	= 612,163 cuft
Drainage area =	÷ 67.440 ac	Curve number	= 80*
Basin Slope =	÷ 0.0 %	Hydraulic length	= 0 ft
Tc method =	: User	Time of conc. (Tc)	= 5.00 min
Total precip. =	÷ 4.74 in	Distribution	= Type II
Storm duration =	24 hrs	Shape factor	= 484

* Composite (Area/CN) = [(13.210 x 55) + (16.000 x 65) + (38.230 x 95)] / 67.440



Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

Hyd. No. 22

POST-DEV BASINB (BYPASS)

Hydrograph type	= SCS Runoff	Peak discharge	= 31.08 cfs
Storm frequency	= 5 yrs	Time to peak	= 11.97 hrs
Time interval	= 2 min	Hyd. volume	= 62,183 cuft
Drainage area	= 9.900 ac	Curve number	= 70*
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= User	Time of conc. (Tc)	= 5.00 min
Total precip.	= 4.74 in	Distribution	= Type II
Storm duration	= 24 hrs	Shape factor	= 484

* Composite (Area/CN) = [(3.750 x 95) + (6.150 x 55)] / 9.900



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Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

Hyd. No. 23

POST-BASIN B POND

Hydrograph type	= Reservoir	Peak discharge	= 81.47 cfs
Storm frequency	= 5 yrs	Time to peak	= 12.07 hrs
Time interval	= 2 min	Hyd. volume	= 612,112 cuft
Inflow hyd. No.	= 21 - POST-DEV BASIN B	Max. Elevation	= 1085.10 ft
Reservoir name	= WET POND 2	Max. Storage	= 245,441 cuft

Storage Indication method used.



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Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

Wednesday, 02 / 28 / 2024

Hyd. No. 24

POST-DEV BASIN B TO SP-2

Hydrograph type Storm frequency	= Reach = 5 yrs	Peak discharge Time to peak	= 60.51 cfs = 12.47 hrs = 611 670 ouff
Inflow hyd. No.	= 23 - POST-BASIN B POND	Section type	= Trapezoidal
Reach length	= 1295.0 ft	Channel slope	= 4.0 %
Manning's n	= 0.150	Bottom width	= 18.0 ft
Side slope	= 10.0:1	Max. depth	= 2.0 ft
Rating curve x	= 0.289	Rating curve m	= 1.281
Ave. velocity	= 1.00 ft/s	Routing coeff.	= 0.1115

Modified Att-Kin routing method used.



Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

Hyd. No. 26

POST-DEV AT SP-2

Hydrograph type Storm frequency	= Combine = 5 vrs	Peak discharge Time to peak	= 63.59 cfs = 12.43 brs
Time interval	$= 2 \min$	Hyd. volume	= 673,852 cuft
Inflow hyds.	= 22, 24	Contrib. drain. area	= 9.900 ac



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Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

Hyd. No. 27

PRE DEV-BASIN B2

Hydrograph type	= SCS Runoff	Peak discharge	= 81.73 cfs
Storm frequency	= 5 yrs	Time to peak	= 12.00 hrs
Time interval	= 1 min	Hyd. volume	= 216,884 cuft
Drainage area	= 70.000 ac	Curve number	= 55
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= User	Time of conc. (Tc)	= 10.00 min
Total precip.	= 4.74 in	Distribution	= Type II
Storm duration	= 24 hrs	Shape factor	= 484



Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

Hyd. No. 21

POST-DEV BASIN B

Hydrograph type =	SCS Runoff	Peak discharge	= 363.85 cfs
Storm frequency =	= 10 yrs	Time to peak	= 11.93 hrs
Time interval =	2 min	Hyd. volume	= 742,624 cuft
Drainage area =	= 67.440 ac	Curve number	= 80*
Basin Slope =	= 0.0 %	Hydraulic length	= 0 ft
Tc method =	= User	Time of conc. (Tc)	= 5.00 min
Total precip. =	= 5.39 in	Distribution	= Type II
Storm duration =	= 24 hrs	Shape factor	= 484

* Composite (Area/CN) = [(13.210 x 55) + (16.000 x 65) + (38.230 x 95)] / 67.440



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Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

Hyd. No. 22

POST-DEV BASINB (BYPASS)

Hydrograph type	= SCS Runoff	Peak discharge	= 39.11 cfs
Storm frequency	= 10 yrs	Time to peak	= 11.97 hrs
Time interval	= 2 min	Hyd. volume	= 78,498 cuft
Drainage area	= 9.900 ac	Curve number	= 70*
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= User	Time of conc. (Tc)	= 5.00 min
Total precip.	= 5.39 in	Distribution	= Type II
Storm duration	= 24 hrs	Shape factor	= 484

* Composite (Area/CN) = [(3.750 x 95) + (6.150 x 55)] / 9.900



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Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

Hyd. No. 23

POST-BASIN B POND

Hydrograph type	= Reservoir	Peak discharge	= 95.20 cfs
Storm frequency	= 10 yrs	Time to peak	= 12.07 hrs
Time interval	= 2 min	Hyd. volume	= 742,573 cuft
Inflow hyd. No.	= 21 - POST-DEV BASIN B	Max. Elevation	= 1085.97 ft
Reservoir name	= WET POND 2	Max. Storage	= 296,812 cuft

Storage Indication method used.



Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

Wednesday, 02 / 28 / 2024

Hyd. No. 24

POST-DEV BASIN B TO SP-2

Hydrograph type Storm frequency Time interval Inflow hyd. No. Reach length	 Reach 10 yrs 2 min 23 - POST-BASIN B POND 1295.0 ft 0.150 	Peak discharge Time to peak Hyd. volume Section type Channel slope	= 77.02 cfs = 12.50 hrs = 742,131 cuft = Trapezoidal = 4.0 %
Inflow hvd. No.	= 23 - POST-BASIN B POND	Section type	= Trapezoidal
Reach length	= 1295.0 ft	Channel slope	= 4.0 %
Manning's n	= 0.150	Bottom width	= 18.0 ft
Side slope	= 10.0:1	Max. depth	= 2.0 ft
Rating curve x	= 0.289	Rating curve m	= 1.281
Ave. velocity	= 1.03 ft/s	Routing coeff.	= 0.1152

Modified Att-Kin routing method used.



Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

Hyd. No. 26

POST-DEV AT SP-2

Hydrograph type	= Combine	Peak discharge	= 80.59 cfs
Storm frequency	= 10 yrs	Time to peak	= 12.47 hrs
Time interval	= 2 min	Hyd. volume	= 820,629 cuft
Inflow hyds.	= 22, 24	Contrib. drain. area	= 9.900 ac



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Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

Hyd. No. 27

PRE DEV-BASIN B2

Hydrograph type	= SCS Runoff	Peak discharge	= 121.27 cfs
Storm frequency	= 10 yrs	Time to peak	= 12.00 hrs
Time interval	= 1 min	Hyd. volume	= 299,959 cuft
Drainage area	= 70.000 ac	Curve number	= 55
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= User	Time of conc. (Tc)	= 10.00 min
Total precip.	= 5.39 in	Distribution	= Type II
Storm duration	= 24 hrs	Shape factor	= 484



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Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

Hyd. No. 21

POST-DEV BASIN B

Hydrograph type =	SCS Runoff	Peak discharge	= 452.89 cfs
Storm frequency =	25 yrs	Time to peak	= 11.93 hrs
Time interval =	2 min	Hyd. volume	= 932,281 cuft
Drainage area =	67.440 ac	Curve number	= 80*
Basin Slope =	0.0 %	Hydraulic length	= 0 ft
Tc method =	User	Time of conc. (Tc)	= 5.00 min
Total precip. =	6.31 in	Distribution	= Type II
Storm duration =	24 hrs	Shape factor	= 484

* Composite (Area/CN) = [(13.210 x 55) + (16.000 x 65) + (38.230 x 95)] / 67.440



Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

Hyd. No. 22

POST-DEV BASINB (BYPASS)

Hydrograph type	= SCS Runoff	Peak discharge	= 50.95 cfs
Storm frequency	= 25 yrs	Time to peak	= 11.93 hrs
Time interval	= 2 min	Hyd. volume	= 102,865 cuft
Drainage area	= 9.900 ac	Curve number	= 70*
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= User	Time of conc. (Tc)	= 5.00 min
Total precip.	= 6.31 in	Distribution	= Type II
Storm duration	= 24 hrs	Shape factor	= 484

* Composite (Area/CN) = [(3.750 x 95) + (6.150 x 55)] / 9.900



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Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

Hyd. No. 23

POST-BASIN B POND

Hydrograph type	= Reservoir	Peak discharge	= 107.63 cfs
Storm frequency	= 25 yrs	Time to peak	= 12.07 hrs
Time interval	= 2 min	Hyd. volume	= 932,231 cuft
Inflow hyd. No.	= 21 - POST-DEV BASIN B	Max. Elevation	= 1087.15 ft
Reservoir name	= WET POND 2	Max. Storage	= 374,639 cuft

Storage Indication method used.



Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

Wednesday, 02 / 28 / 2024

Hyd. No. 24

POST-DEV BASIN B TO SP-2

Hydrograph type	= Reach	Peak discharge	= 93.06 cfs
Storm frequency	= 25 yrs	Time to peak	= 12.57 hrs
Time interval	= 2 min	Hyd. volume	= 931,788 cuft
Inflow hyd. No.	= 23 - POST-BASIN B POND	Section type	 Trapezoidal
Reach length	= 1295.0 ft	Channel slope	= 4.0 %
Manning's n	= 0.150	Bottom width	= 18.0 ft
Side slope	= 10.0:1	Max. depth	= 2.0 ft
Rating curve x	= 0.289	Rating curve m	= 1.281
Ave. velocity	= 1.06 ft/s	Routing coeff.	= 0.1181

Modified Att-Kin routing method used.



Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

Hyd. No. 26

POST-DEV AT SP-2

Hydrograph type Storm frequency	= Combine = 25 yrs	Peak discharge Time to peak	= 96.88 cfs = 12.53 hrs
Time interval	= 2 min	Hyd. volume	= 1,034,651 cuft
Inflow hyds.	= 22, 24	Contrib. drain. area	= 9.900 ac



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Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

Hyd. No. 27

PRE DEV-BASIN B2

Hydrograph type	= SCS Runoff	Peak discharge	= 183.04 cfs
Storm frequency	= 25 yrs	Time to peak	= 12.00 hrs
Time interval	= 1 min	Hyd. volume	= 431,745 cuft
Drainage area	= 70.000 ac	Curve number	= 55
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= User	Time of conc. (Tc)	= 10.00 min
Total precip.	= 6.31 in	Distribution	= Type II
Storm duration	= 24 hrs	Shape factor	= 484



Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

Hyd. No. 21

POST-DEV BASIN B

SCS Runoff	Peak discharge	= 522.92 cfs
50 yrs	Time to peak	= 11.93 hrs
2 min	Hyd. volume	= 1,083,763 cuft
67.440 ac	Curve number	= 80*
0.0 %	Hydraulic length	= 0 ft
User	Time of conc. (Tc)	= 5.00 min
7.03 in	Distribution	= Type II
24 hrs	Shape factor	= 484
	SCS Runoff 50 yrs 2 min 67.440 ac 0.0 % User 7.03 in 24 hrs	SCS RunoffPeak discharge50 yrsTime to peak2 minHyd. volume67.440 acCurve number0.0 %Hydraulic lengthUserTime of conc. (Tc)7.03 inDistribution24 hrsShape factor

* Composite (Area/CN) = [(13.210 x 55) + (16.000 x 65) + (38.230 x 95)] / 67.440



Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

Hyd. No. 22

POST-DEV BASINB (BYPASS)

Hydrograph type	= SCS Runoff	Peak discharge	= 60.72 cfs
Storm frequency	= 50 yrs	Time to peak	= 11.93 hrs
Time interval	= 2 min	Hyd. volume	= 122,748 cuft
Drainage area	= 9.900 ac	Curve number	= 70*
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= User	Time of conc. (Tc)	= 5.00 min
Total precip.	= 7.03 in	Distribution	= Type II
Storm duration	= 24 hrs	Shape factor	= 484

* Composite (Area/CN) = [(3.750 x 95) + (6.150 x 55)] / 9.900



Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

Hyd. No. 23

POST-BASIN B POND

Hydrograph type	= Reservoir	Peak discharge	= 115.94 cfs
Storm frequency	= 50 yrs	Time to peak	= 12.07 hrs
Time interval	= 2 min	Hyd. volume	= 1,083,709 cuft
Inflow hyd. No.	= 21 - POST-DEV BASIN B	Max. Elevation	= 1088.02 ft
Reservoir name	= WET POND 2	Max. Storage	= 437,378 cuft

Storage Indication method used.


Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

Wednesday, 02 / 28 / 2024

Hyd. No. 24

POST-DEV BASIN B TO SP-2

Hydrograph type Storm frequency Time interval Inflow hyd. No. Reach length Manning's n Side slope Rating curve x	 Reach 50 yrs 2 min 23 - POST-BASIN B POND 1295.0 ft 0.150 10.0:1 0.289 1.08 ft/c 	Peak discharge Time to peak Hyd. volume Section type Channel slope Bottom width Max. depth Rating curve m	 = 102.60 cfs = 12.60 hrs = 1,083,271 cuft = Trapezoidal = 4.0 % = 18.0 ft = 2.0 ft = 1.281 = 0.1100
Ave. velocity	= 1.08 ft/s	Routing coeff.	= 0.1199

Modified Att-Kin routing method used.



Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

Hyd. No. 26

POST-DEV AT SP-2

Hydrograph type Storm frequency	= Combine = 50 vrs	Peak discharge Time to peak	= 106.94 cfs = 12.53 hrs
Time interval	= 2 min	Hyd. volume	= 1,206,018 cuft
Inflow hyds.	= 22, 24	Contrib. drain. area	= 9.900 ac



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Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

Hyd. No. 27

PRE DEV-BASIN B2

Hydrograph type	= SCS Runoff	Peak discharge	= 235.24 cfs
Storm frequency	= 50 yrs	Time to peak	= 12.00 hrs
Time interval	= 1 min	Hyd. volume	= 544,520 cuft
Drainage area	= 70.000 ac	Curve number	= 55
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= User	Time of conc. (Tc)	= 10.00 min
Total precip.	= 7.03 in	Distribution	= Type II
Storm duration	= 24 hrs	Shape factor	= 484



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Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

Hyd. No. 21

POST-DEV BASIN B

Hydrograph type =	SCS Runoff	Peak discharge	= 595.01 cfs
Storm frequency =	100 yrs	Time to peak	= 11.93 hrs
Time interval =	2 min	Hyd. volume	= 1,241,564 cuft
Drainage area =	67.440 ac	Curve number	= 80*
Basin Slope =	0.0 %	Hydraulic length	= 0 ft
Tc method =	User	Time of conc. (Tc)	= 5.00 min
Total precip. =	7.77 in	Distribution	= Type II
Storm duration =	24 hrs	Shape factor	= 484

* Composite (Area/CN) = [(13.210 x 55) + (16.000 x 65) + (38.230 x 95)] / 67.440



Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

Hyd. No. 22

POST-DEV BASINB (BYPASS)

Hydrograph type	= SCS Runoff	Peak discharge	= 70.93 cfs
Storm frequency	= 100 yrs	Time to peak	= 11.93 hrs
Time interval	= 2 min	Hyd. volume	= 143,769 cuft
Drainage area	= 9.900 ac	Curve number	= 70*
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= User	Time of conc. (Tc)	= 5.00 min
Total precip.	= 7.77 in	Distribution	= Type II
Storm duration	= 24 hrs	Shape factor	= 484

* Composite (Area/CN) = [(3.750 x 95) + (6.150 x 55)] / 9.900



Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

Hyd. No. 23

POST-BASIN B POND

Hydrograph type	= Reservoir	Peak discharge	= 143.86 cfs
Storm frequency	= 100 yrs	Time to peak	= 12.07 hrs
Time interval	= 2 min	Hyd. volume	= 1,241,510 cuft
Inflow hyd. No.	= 21 - POST-DEV BASIN B	Max. Elevation	= 1088.82 ft
Reservoir name	= WET POND 2	Max. Storage	= 500,989 cuft

Storage Indication method used.



Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

Wednesday, 02 / 28 / 2024

Hyd. No. 24

POST-DEV BASIN B TO SP-2

Hydrograph type Storm frequency	= Reach = 100 yrs	Peak discharge Time to peak	= 112.45 cfs = 12.53 hrs
Time interval	= 2 min	Hyd. volume	= 1,241,072 cuft
Inflow hyd. No.	= 23 - POST-BASIN B POND	Section type	 Trapezoidal
Reach length	= 1295.0 ft	Channel slope	= 4.0 %
Manning's n	= 0.150	Bottom width	= 18.0 ft
Side slope	= 10.0:1	Max. depth	= 2.0 ft
Rating curve x	= 0.289	Rating curve m	= 1.281
Ave. velocity	= 1.13 ft/s	Routing coeff.	= 0.1254

Modified Att-Kin routing method used.



Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

Hyd. No. 26

POST-DEV AT SP-2

Hydrograph type	= Combine	Peak discharge	= 122.02 cfs
Storm frequency	= 100 yrs	Time to peak	= 11.97 hrs
Time interval	= 2 min	Hyd. volume	= 1,384,841 cuft
Inflow hyds.	= 22, 24	Contrib. drain. area	= 9.900 ac
Time interval	= 2 min	Hyd. volume	= 1,384,841 (
Inflow hyds.	= 22, 24	Contrib. drain. area	= 9.900 ac



Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

Hyd. No. 27

PRE DEV-BASIN B2

Hydrograph type	= SCS Runoff	Peak discharge	= 291.72 cfs
Storm frequency	= 100 yrs	Time to peak	= 12.00 hrs
Time interval	= 1 min	Hyd. volume	= 667,784 cuft
Drainage area	= 70.000 ac	Curve number	= 55
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= User	Time of conc. (Tc)	= 10.00 min
Total precip.	= 7.77 in	Distribution	= Type II
Storm duration	= 24 hrs	Shape factor	= 484



Appendix F



Development Name: Cagle Mill

Drainage Basin Name: A

data in
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constar

Site Data

Indicate Pre-Development Land Cover and Runoff Curve Numbers in the Si	te's Disturbe	d Area								
Cover Type	HSG* A (acres)	CN	HSG B (acres)	CN	HSG C (acres)	CN	HSG D (acres)	CN	Total	% Cover
Woods - Good Condition	, ,	30	53.31	55		70		77	53.31	100%
Impervious		98	0.00	98		98		98	0.00	0%
Open space - Good condition (grass cover > 75%)		39	0.00	61		74		80	0.00	0%
Select a land cover type		0		0		0		0	0.00	0%
Select a land cover type		0		0		0		0	0.00	0%
Local Jurisdiction Input									0.00	0%
Other									0.00	0%
Total	0.00		53.31		0.00		0.00		53.31	100%
*HSG = hydrologic soil group							Impervious (ac)	0.00		
							Weighted CN	55	1	
					P	otential Max Soi	Retention, S _{pre} (in)	8.18]	
Indicate Post-Development Land Cover and Runoff Curve Numbers in the S	Site's Disturb	ed Area								
	HSG A	CN		CN	HSG C	CN	HSG D	CN	Total	% Cover
Cover Type	(acres)	CN	HSG B (acres)	CN	(acres)		(acres)	CN	Total	% Cover
Impervious		98	24.56	98		98		98	24.56	35%
Open space - Good condition (grass cover > 75%)		39	20.00	61		74		80	20.00	28%
Woods - Good Condition		30	19.30	55		70		77	19.30	27%
Impervious		98	6.50	98		98		98	6.50	9%
Open space - Good condition (grass cover > 75%)		39	0.00	61		74		80	0.00	0%
Local Jurisdiction Input									0.00	0%
Other									0.00	0%
Total	0.00		70.36		0.00		0.00		70.36	100%
					Ро	tential Max Soil	Weighted CN Retention, S _{post} (in)	76 3.21		
			Conser	vation Are	ea Credits					
Secondria 1: Natural Concentration Area *Sec the CSMM Volume 2: Section 2:2:2:2	r mara informa	tion			Seconaria 21	Soil Postaration	n *Saa tha CSMM	Volumo 2 Soo	tion 1 22 for mo	ro information
Scenario T. Natural Conservation Area See the GSMM Volume 2, Section 2.3.3.3 ic					Scenario S. S	Soll Residiation		volume 2, Sec	1011 4.23 101 1110	re mornation
Check the box if a portion of the post-developed area is protected by a conse protection.	rvation easem	ent or equivale	ent form of			Check the box conservation ea	if a portion of the po asement or equival	ost-developed ent form of prot	area employs <u>s</u> tection.	oil restoration
Area (ac) of development protected by a conservation easement or equivalent form of protection.	Note: The green above is checke	cell will unlock d	if the Scenario 1 box			Area (ac) of de easement or ec	velopment with res quivalent form of pr	tored soils and otection.	protected by a	conservation
Scenario 2: Site Reforestation/Revegetation *See the GSMM Volume 2, Section 4.2	2 for more info	rmation.			Scenario 4: S	Site Reforestati	ion/Revegetation o	& Soil Restora	tion	*See the GS more informa
Check the box if a portion of the post-developed area employs site reforestation conservation easement or equivalent form of protection.	on/revegetatio	<u>n</u> and is prote	cted by a			Check the box soil restoration,	if the same portion and is protected b	of the post-dev y a conservatio	veloped area er on easement or	nploys <u>site re</u> equivalent fo
Area (ac) of development reforested/revegetated and protected by a conservation easement or equivalent form of protection.	Note: The green above is checke	cell will unlock d	if the Scenario 2 box			Area (ac) with r by a conservati	restored soils in a re on easement or eq	eforested & rev uivalent form o	regetated area a f protection.	and protected

Total Conservation Area Credit (acres) 0.00

nput cells ation cells ant values

ind is protected by a

Note: The green cell will unlock if the Scenario 3 box above is checked IM Volume 2, Section 4.22 and 4.23 for ion. restation/revegetation and of protection. Note: The green cell will unlock if the Scenario 4 box above is checked ----

	Development Name: Drainage Basin Name:	Cagle Mill A									data input calculatior constant v
					Wat	er Quality	Goals				
Target Runoff Reduction Storm (in) 1.00 Total Site Area for Water Quality Volume (acres) 70.36 Target Runoff Reduction Volume (cf) 114,243 Target Water Quality Volume (cf) 137,092											
				Select BN	IPs for Rur	off Reduct	tion and V	Vater Qual	lity		
		Area D	Draining to Each	n BMP	Storage Volume	RR Conveyance		Runoff Reduction Calcula			
		On-site Pervious Area (acres)	On-site Impervious Area (acres)	Offsite Area (acres)	Provided by BMP (cf)	Provided by BMP (cf)	Down-stream BMP	RR Volume from Direct Drainage (cf)	RR Volume from Upstream Practices (cf)	Total RR Volume Received by BMP (cf)	Runo Reductio
BMP 1	Stormwater Pond	20.00	24.56		70,000		BMP 2	88,325	0	88,325	0%
BMP 2	Stormwater Wetlands – Level 1		0.00			88,325		0	88,325	88,325	0%
BMP 3	Bioretention Basin (w/o underdrain)		2.50					8,621	0	8,621	100%
BMP 4	Bioretention Basin (w/o underdrain)		2.50					8,621	0	8,621	100%
BMP 5	Regenerative Stormwater Conveyance	19.30	1.50					8,676	0	8,676	0%
BMP 6	Select a BMP		0.00					0	0	0	N/A
BMP 7	Select a BMP							0	0	0	N/A
BMP 8	Select a BMP							0	0	0	N/A
BMP 9	Select a BMP							0	0	0	N/A
BMP 10	Select a BMP							0	0	0	N/A
	TOTAL UNTREATED AREA (acres)	39.30 0.00	31.06 0.00	0.00				114,243			

Target Runoff Reduction Volume (cf)	114,243
Target Achieved?	No
Remaining Runoff Reduction Volume (cf)	114,243

Target Water Quality Volume (cf)	137,092
% TSS Removal Achieved	95%
Target Achieved?	Yes!
Remaining TSS Removal %	0%

it cells on cells values

ions			WQ Calc	ulations
off on %	RR Achieved (cf)	Remaining RR Volume (cf)	WQ _v from Direct Drainage (cf)	Effective TSS Removal %
,	0	88,325	105,990	80%
)	0	88,325	0	80%
%	0	8,621	10,346	100%
%	0	8,621	10,346	100%
,	0	8,676	10,411	80%
۸	0	0	0	N/A
۸	0	0	0	N/A
λ	0	0	0	N/A
\	0	0	0	N/A
۸	0	0	0	N/A
	0		137,092	

Development Name: Cagle Mill

Drainage Basin Name: A

data input cells calculation cells constant values

	1-yr, 24-hr storm	2-yr, 24-hr storm	25-yr, 24-hr storm	100-yr, 24-hr storm
Target Rainfall Event (in)				
Г	1-yr, 24-hr	2-yr, 24-hr	25-yr, 24-hr	100-yr, 24-hr
	storm	storm	storm	storm
Pre-Development Runoff Volume (in)	0.00	0.00	0.00	0.00
Post Development Runoff Volume (in) with no BMPs	0.00	0.00	0.00	0.00
	0.00	0.00	0.00	0.00
Post-Development Runoff Volume (in) with BMPs	0.00	0.00		

*See Stormwater Management Standards to Determine Detention Requirements.

Comments

Development Name: Cagle Mill

Drainage Basin Name: B

data input cells calculation cells constant values

Site Data

Cover Type	HSG* A (acres)	CN	HSG B (acres)	CN	HSG C (acres)	CN	HSG D (acres)	CN	Total	% Cover
Woods - Good Condition		30	70.00	55		70		77	70.00	100%
Select a land cover type		0	0.00	0		0		0	0.00	0%
Select a land cover type		0		0		0		0	0.00	0%
Select a land cover type		0		0		0		0	0.00	0%
Select a land cover type		0		0		0		0	0.00	0%
Local Jurisdiction Input									0.00	0%
Other	0.00		70.00		0.00		0.00		0.00	0%
10ldi	0.00		70.00		0.00			0.00	70.00	100%
Indicate Post-Development Land Cover and Runoff Curve Numbers in the	Site's Disturb	oed Area			P	otential Max Soil	Weighted CN Retention, S _{pre} (in)	55 8.18		
Cover Type	HSG A (acres)	CN	HSG B (acres)	CN	HSG C (acres)	CN	HSG D (acres)	CN	Total	% Cover
Woods - Good Condition		30	13.21	55		70		77	13.21	20%
Open space - Good condition (grass cover > 75%)		39	16.00	61		74		80	16.00	24%
Impervious		98	38.23	98		98		98	38.23	57%
Select a land cover type		0		0		0		0	0.00	0%
Select a land cover type		0		0		0		0	0.00	0%
Local Jurisdiction Input									0.00	0%
Other	0.00		07.44		0.00		0.00		0.00	0%
					Ро	tential Max Soil I	Weighted CN Retention, S _{post} (in)	81 2.38		
			Conser	vation Ar	ea Credits					
Scenario 1: Natural Conservation Area *See the GSMM Volume 2, Section 2.3.3.3 for	or more informa	ation.			Scenario 3: S	Soil Restoratior	*See the GSMM	Volume 2, Sect	tion 4.23 for mo	ore information.
Check the box if a portion of the post-developed area is protected by a conse protection.	ervation easem	ent or equivale	ent form of			Check the box i conservation ea	f a portion of the p asement or equival	ost-developed a ent form of prot	area employs <u>s</u> ection.	<u>oil restoration</u> a
Area (ac) of development protected by a conservation easement or equivalent form of protection.	Note: The green above is checke	cell will unlock d	f the Scenario 1 box			Area (ac) of development or equipment or equ	velopment with res uivalent form of pr	tored soils and otection.	protected by a	conservation
Scenario 2: Site Reforestation/Revegetation *See the GSMM Volume 2, Section 4.2	22 for more info	rmation.			Scenario 4: S	Site Reforestati	on/Revegetation	& Soil Restora	tion	*See the GSM more informati
Check the box if a portion of the post-developed area employs <u>site reforestat</u> conservation easement or equivalent form of protection.	tion/revegetatio	<u>n</u> and is proted	cted by a			Check the box i soil restoration,	f the same portion and is protected b	of the post-dev y a conservatio	eloped area er n easement or	nploys <u>site refo</u> equivalent form
Area (ac) of development reforested/revegetated and protected by a conservation easement or equivalent form of protection.	Note: The green above is checke	cell will unlock d	f the Scenario 2 box			Area (ac) with r by a conservati	estored soils in a r on easement or eq	eforested & rev uivalent form o	egetated area a f protection.	and protected

ind is protected by a

Note: The green cell will unlock if the Scenario 3 box above is checked IM Volume 2, Section 4.22 and 4.23 for ion. restation/revegetation and of protection. Note: The green cell will unlock if the Scenario 4 box above is checked ----

	Development Name: Drainage Basin Name:	Cagle Mill B									data input calculation constant v
					Wat	er Quality	Goals				
	Target Runoff Reduction Storm (in)	1.00	l		Total Site Area for Targe Ta	r Water Quality V t Runoff Reductio arget Water Quali	olume (acres) on Volume (cf) ity Volume (cf)	67.44 137,138 164,565			
	Select BMPs for Runoff Reduction and Water Quality										
		Area [Draining to Each	ו BMP	Otomore Maluma	RR Conveyance			Rı	noff Reduction	Calculati
		On-site Pervious Area (acres)	On-site Impervious Area (acres)	Offsite Area (acres)	Provided by BMP (cf)	Volume Provided by BMP (cf)	Down-stream BMP	RR Volume from Direct Drainage (cf)	RR Volume from Upstream Practices (cf)	Total RR Volume Received by BMP (cf)	Runo Reductio
BMP 1	Stormwater Pond	16.00	38.23	13.21				134,740	0	134,740	0%
BMP 2	Regenerative Stormwater Conveyance	6.15	3.75					14,048	0	14,048	0%
BMP 3	Select a BMP							0	0	0	N/A
BMP 4	Select a BMP							0	0	0	N/A
BMP 5	Select a BMP							0	0	0	N/A
BMP 6	Select a BMP							0	0	0	N/A
BMP 7	Select a BMP							0	0	0	N/A
BMP 8	Select a BMP							0	0	0	N/A
BMP 9	Select a BMP							0	0	0	N/A
BMP 10	Select a BMP							0	0	0	N/A
		22.15	41.98	13.21				148,788			

Target Runoff Reduction Volume (cf)	137,138
Target Achieved?	No
Remaining Runoff Reduction Volume (cf)	137,138

Target Water Quality Volume (cf)	164,565
% TSS Removal Achieved	87%
Target Achieved?	Yes!
Remaining TSS Removal %	0%

it cells on cells values

ions			WQ Calc	ulations
off on %	RR Achieved (cf)	Remaining RR Volume (cf)	WQ _v from Direct Drainage (cf)	Effective TSS Removal %
,	0	134,740	161,688	80%
,	0	14,048	16,858	80%
٨	0	0	0	N/A
λ	0	0	0	N/A
λ	0	0	0	N/A
۸	0	0	0	N/A
۸	0	0	0	N/A
λ	0	0	0	N/A
λ	0	0	0	N/A
λ	0	0	0	N/A
	0		178,546	

Development Name: Cagle Mill Drainage Basin Name: B

calculation cells

	1-yr, 24-hr storm	2-yr, 24-hr storm	25-yr, 24-hr storm	100-yr, 24-hr storm
Target Rainfall Event (in)				
	1-yr, 24-hr	2-yr, 24-hr	25-yr, 24-hr	100-yr, 24-hr
	storm	storm	storm	storm
Pre-Development Runoff Volume (in)	0.00	0.00	0.00	0.00
Post Development Runoff Volume (in) with no BMPs	0.00	0.00	0.00	0.00
Post-Development Runoff Volume (in) with BMPs	0.00	0.00	0.00	0.00
Adjusted CN	0	0	0	0
	•	•	•	•
ee Stormwater Management Standards to Determine Detention Requirements.				
ee Stormwater Management Standards to Determine Detention Requirements.				
ee Stormwater Management Standards to Determine Detention Requirements.				

data input cells constant values

0

0

0.00

0

0

Development Name: Cagle Mill

Select a land cover type ..

Select a land cover type ...

Local Jurisdiction Input

Other Total

0.00

Drainage Basin Name: C

									constant values	
				Site Data						
Indicate Pre-Development Land Cover and Runoff Curve Numbers in the Sit	te's Disturbe	d Area								
Cover Type	HSG* A (acres)	CN	HSG B (acres)	CN	HSG C (acres)	CN	HSG D (acres)	CN	Total	% Cover
Woods - Good Condition		30	6.07	55		70		77	6.07	100%
Select a land cover type		0		0		0		0	0.00	0%
Select a land cover type		0		0		0		0	0.00	0%
Select a land cover type		0		0		0		0	0.00	0%
Select a land cover type		0		0		0		0	0.00	0%
Local Jurisdiction Input									0.00	0%
Other									0.00	0%
Total	0.00		6.07		0.00		0.00		6.07	100%
*HSG = hydrologic soil group			-		-		Impervious (ac)	0.00		
					P	otential Max Soil	Retention, Spra (in)	55 8.18		
Indicate Post-Development Land Cover and Runoff Curve Numbers in the Site's Disturbed Area										
Cover Type	HSG A (acres)	CN	HSG B (acres)	CN	HSG C (acres)	CN	HSG D (acres)	CN	Total	% Cover
Impervious		98	1.52	98		98		98	1.52	54%
Open space - Fair condition (grass cover 50% to 75%)		49	1.30	69		79		84	1.30	46%
Select a land cover type		0		0		0		0	0.00	0%

2.82

				Potential Max Soil Retention, S _{post} (in) 1.82
		Conser	vation Are	a Credits
Scenar	io 1: Natural Conservation Area *See the GSMM Volume 2, Section 2.3.3.	3 for more information.		Scenario 3: Soil Restoration *See the GSMM Volume 2, Section 4.23
	Check the box if a portion of the post-developed area is protected by a co protection.	nservation easement or equivalent form of		Check the box if a portion of the post-developed area emp conservation easement or equivalent form of protection.
	Area (ac) of development protected by a conservation easement or equivalent form of protection.	Note: The green cell will unlock if the Scenario 1 box above is checked		Area (ac) of development with restored soils and protected easement or equivalent form of protection.
Scenar	io 2: Site Reforestation/Revegetation *See the GSMM Volume 2, Section	4.22 for more information.		Scenario 4: Site Reforestation/Revegetation & Soil Restoration
	Check the box if a portion of the post-developed area employs <u>site refores</u> conservation easement or equivalent form of protection.	station/revegetation and is protected by a		Check the box if the same portion of the post-developed at soil restoration, and is protected by a conservation easement
	Area (ac) of development reforested/revegetated and protected by a conservation easement or equivalent form of protection.	Note: The green cell will unlock if the Scenario 2 box above is checked		Area (ac) with restored soils in a reforested & revegetated by a conservation easement or equivalent form of protection
		Total Conservation Area Credit (acres)	0.00	

0

0

data input cells calculation cells constant values

Total	% Cover
1.52	54%
1.30	46%
0.00	0%
0.00	0%
0.00	0%
0.00	0%
0.00	0%
2.82	100%

0

0

1.52

0.54

85

0.00 Impervious (ac)

Weighted CN

Rv

for more information.

loys soil restoration and is protected by a

by a conservation Note: The green cell will unlock if the Scenario 3 box above is checked *See the GSMM Volume 2, Section 4.22 and 4.23 for more information. rea employs site reforestation/revegetation and ent or equivalent form of protection. area and protected Note: The green cell will unlock if the Scenario 4 box above is checked on.

	Development Name: Drainage Basin Name:	Cagle Mill C									data input calculation constant v
					Wat	er Quality	Goals				
	Target Runoff Reduction Storm (in)	1.00	I		Total Site Area fo Targe Ta	r Water Quality V t Runoff Reductio arget Water Qual	olume (acres) on Volume (cf) ity Volume (cf)	2.82 5,478 6,573			
				Select BN	IPs for Rur	off Reduc	tion and V	Nater Qual	lity		
	r	Arro 6	Ducining to Frick	DMD						un off Doduction	Calaulati
		Area I	Draining to Each	n BMP	Storage Volume	RR Conveyance	R Conveyance	Runoff Reduction Calcula			Calculati
		On-site Pervious Area (acres)	On-site Impervious Area (acres)	Offsite Area (acres)	Provided by BMP (cf)	Provided by BMP (cf)	Down-stream BMP	RR Volume from Direct Drainage (cf)	RR Volume from Upstream Practices (cf)	Total RR Volume Received by BMP (cf)	Runo Reductio
BMP 1	Stormwater Wetlands – Level 1	1.30	1.52			5,478		5,478	0	5,478	0%
BMP 2	Select a BMP							0	0	0	N/A
BMP 3	Select a BMP							0	0	0	N/A
BMP 4	Select a BMP							0	0	0	N/A
BMP 5	Select a BMP							0	0	0	N/A
BMP 6	Select a BMP							0	0	0	N/A
BMP 7	Select a BMP							0	0	0	N/A
BMP 8	Select a BMP							0	0	0	N/A
BMP 9	Select a BMP							0	0	0	N/A
BMP 10	Select a BMP							0	0	0	N/A
	TOTAL UNTREATED AREA (acres)	1.30	1.52	0.00				5,478			

Target Runoff Reduction Volume (cf)	5,478
Target Achieved?	No
Remaining Runoff Reduction Volume (cf)	5,478

Target Water Quality Volume (cf)	6,573
% TSS Removal Achieved	80%
Target Achieved?	Yes!
Remaining TSS Removal %	0%

it cells on cells values

ions			WQ Calculations			
off on %	RR Achieved (cf)	Remaining RR Volume (cf)	WQ _v from Direct Drainage (cf)	Effective TSS Removal %		
,	0	5,478	6,573	80%		
۸	0	0	0	N/A		
۸	0	0	0	N/A		
λ	0	0	0	N/A		
λ	0	0	0	N/A		
٨	0	0	0	N/A		
۸.	0	0	0	N/A		
λ	0	0	0	N/A		
<u>۸</u>	0	0	0	N/A		
۸	0	0	0	N/A		
	0		6,573			

Development Name: Cagle Mill

Drainage Basin Name: C

data input cells calculation cells constant values

	1-yr, 24-hr storm	2-yr, 24-hr storm	25-yr, 24-hr storm	100-yr, 24-hr storm
Target Rainfall Event (in)				
1	1-yr, 24-hr	2-yr, 24-hr	25-yr, 24-hr	100-yr, 24-hr
[1-yr, 24-hr storm	2-yr, 24-hr storm	25-yr, 24-hr storm	100-yr, 24-hr storm
Pre-Development Runoff Volume (in)	1-yr, 24-hr storm 0.00	2-yr, 24-hr storm 0.00	25-yr, 24-hr storm 0.00	100-yr, 24-hr storm 0.00
Pre-Development Runoff Volume (in) Post Development Runoff Volume (in) with no BMPs	1-yr, 24-hr storm 0.00 0.00	2-yr, 24-hr storm 0.00 0.00	25-yr, 24-hr storm 0.00 0.00	100-yr, 24-hr storm 0.00 0.00
Pre-Development Runoff Volume (in) Post Development Runoff Volume (in) with no BMPs Post-Development Runoff Volume (in) with BMPs	1-yr, 24-hr storm 0.00 0.00 0.00	2-yr, 24-hr storm 0.00 0.00 0.00	25-yr, 24-hr storm 0.00 0.00 0.00	100-yr, 24-hr storm 0.00 0.00 0.00

*See Stormwater Management Standards to Determine Detention Requirements.

Comments