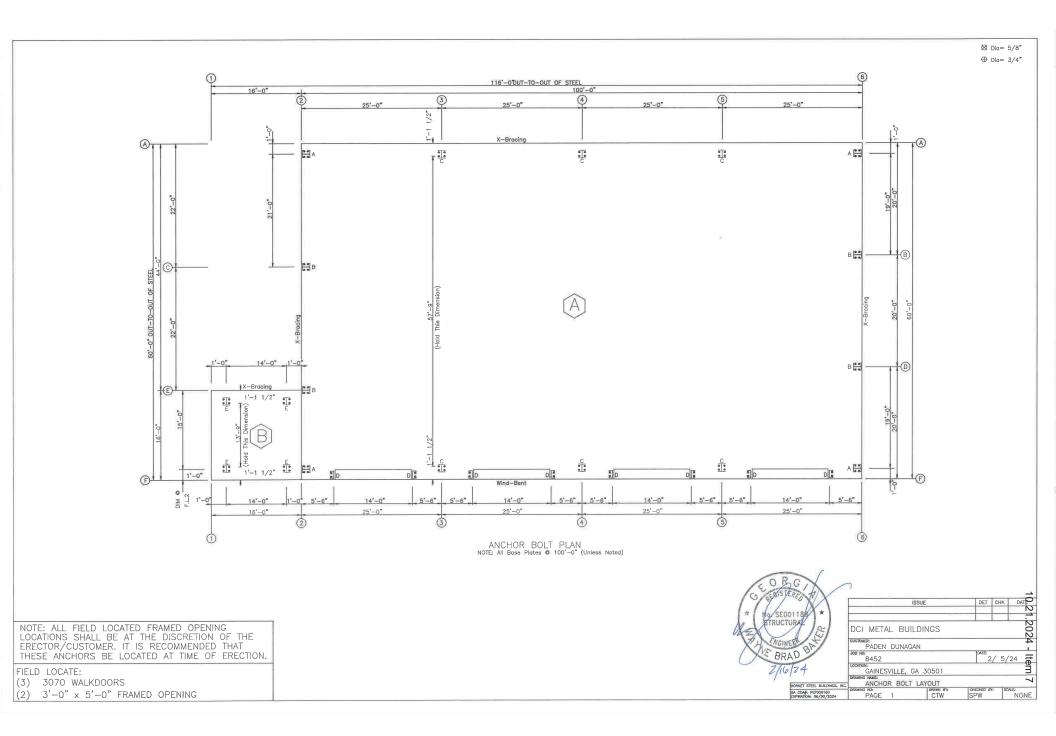
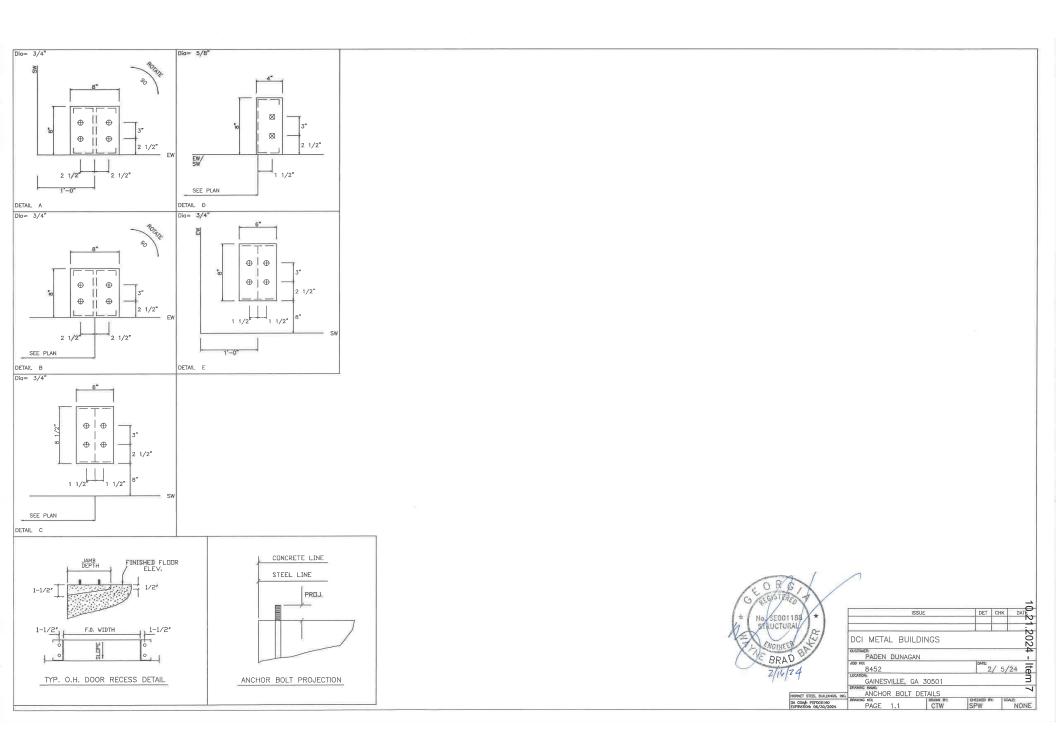
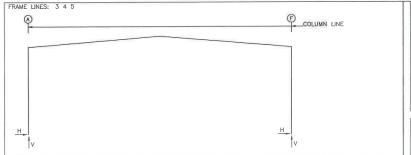
BUILDING PROFILE THE SECONDARY MEMBELS AND PLANCE BRICE CONNECTIONS SHALL ALMAYS BE SAUD TIGHT, UND. 11) ANCING BOILTS A/* IN DIMETER THAN 17.4" IN DIMETER CONFORM TO A.ST.M. F1554 GR. 36. ANCING BOILTS 1/2" IN DIMETER CONFORM TO A.ST.M. A.—307. CHEFOR D. S.T. F1554 GR. 36. DI MILESS NOTED OHIERWISE ON FRANIC COLOR CHATT: ALL STEEL MEMBERS EXCEPT BOILS, FASTENERS, CABLE AND ROOS SHALL RECEIVE ONE COAT OF STANDARD RED OXDE SHOP PRIMER. 5) SHOP AND FIELD INSPECTIONS AND ASSOCIATED EETS ARE THE RESPONSIBILITY OF THE CONTRACTOR, UNLESS STIPULATED OTHERWISE IN THE CONTRACT. APPROVAL NOTES 10) SECONDARY MEMBERS AND FLANGE BRACE CONNECTIONS SHALL ALWAYS BE SNUG TIGHT, UNO. Width (ft) = 60Eave Height (ft) Length (ft) = 100 Roof Slope (Rise/12) = 1.0:12= 21.33Width (ft) = 16Eave Height (ft) Roof Slope (Rise/12) = 1.0:12Length (ft) = 16THE FOLLOWING CONDITIONS APPLY IN THE EVENT THAT THESE DRAWINGS ARE USED AS APPROVAL DRAWINGS: A) IT IS IMPERATIVE THAT ANY CHANGES TO THESE DRAWINGS: BUILDING LOADS Pre-Engineered Metal Buildings 1) BE MADE IN CONTRASTING INK A) THIS IS TO CERTIFY THAT THIS STRUCTURE IS DESIGNED UTILIZING THE LOADS INDICATED AND APPLIED AS REQUIRED BY GSBC 24 / IBC 18 2) HAVE ALL INSTANCES OF CHANGE CLEARLY INDICATED. 3) BE LEGIBLE AND UNAMBIGUOUS. B) THIS CERTIFICATION IS LIMITED TO THE STRUCTURAL DESIGN OF THE FRAMING AND COVERING PARTS MANUFACTURED BY THE BUILDING MANUFACTURER AND AS SPECIFIED IN THE CONTRACT. ACCESSORY ITEMS SUCH AS DOORS, WINDOWS, LOWERS, TRANSLUCENT PANELS, VENTILATORS ARE NOT INCLUDED. ALSO EXCLUDED ARE OTHER PARTS OF THE PROJECT NOT PROVIDED BY THE IT IS THE RESPONSIBILITY OF THE CUSTOMER TO PROVIDE B) DATED SIGNATURE IS REQUIRED ON ALL PAGES. ALL DOCUMENTATION REQUIRED FOR ANY ACCESSORIES NOT PROVIDED BY MBM TO THEIR LOCAL PERMITTING OFFICE. DRAWING INDEX C) MANUFACTURER RESERVES THE RIGHT TO RESUBMIT DRAWINGS WITH EXTENSIVE OR COMPLEX CHANGES REQUIRED TO AVOID MISFABRICATION. THIS MAY IMPACT THE DELIVERY SCHEDULE. NOI INCLUMEN. ALSO EXCLUDED ARE OTHER PARTS OF THE PROJECT NOT PROVIDED BY THE BUILDING MAINFACTURER SUCH AS FOUNDATIONS, MASONRY WALLS, MECHANICAL EQUIPMENT AND THE ERECTION AND INSPECTION OF THE BUILDING, THE BUILDING SHOULD BE ERECTED ON A PROPERTY DESIGNED FOUNDATION IN ACCORDANCE WITH THE BUILDING MAINFACTURER'S DESIGN MAINFALL THE ATTACHED DRAWINGS, AND GOOD ERECTION PROCIDES. THE END USER AND/OR SSORIES MUST COMPLY AND MEET ALL DESIGN REV. D) APPROVAL OF THESE DRAWINGS INDICATES CONCLUSINGLY THAT THE MANUFACTURER HAS CORRECTLY INTERPRETED THE CONTRACT REQUIREMENTS, AND FURTHER CONSTITUTES AGREEMENT THAT THE BUILDING AS DRAWN, OR AS DRAWN WITH INDICATED CHANGES REPRESENTS THE TOTAL OF THE MATERIALS TO BE SUPPLIED BY MANUFACTURER. REQUIREMENTS PER LOCAL CODES. COVER PAGE ALL VEHICULAR FRAMED OPENINGS SUPPLIED ON THIS PROJECT ALL VEHICULAR FRANCE OPENINGS SUPPLIED ON THIS PROJECT HAVE BEEN DESIGNED TO SUPPORT WIND LOADS NORMAL TO A DOOR SYSTEM, BASED ON THE STANDARD BUILDING CODE CRITERIA. THE VEHICULAR FRANCE OPENING HOS NOT BEEN DESIGNED FOR ANY ADDITIONAL MOMENT OR CATEMARY FORCE FROM THE DOOR SYSTEM. ANY CHANGES TO THE INFORMATION SHOWN HERE WOULD REQUIRE AN ENGINEERING SWESTIGATION AND POSSIBLE BUILDING RETINORCHEMBER. ANCHOR BOLT LAYOUT ENGINEER OF RECORD IS TO CONFIRM THAT THESE LOADS COMPLY WITH REQUIREMENTS OF LOCAL BUILDING DEPT. ANCHOR BOLT DETAILS B) ANY CHANGES NOTED ON THE BRAININGS NOT IN CONFERENCE WITH THE TERMS AND REQUIREMENTS OF THE CONTRACT BETWEEN MAULTACHER AND ITS CUSTOMER ARE NOT BRONNE ON MAURTACHERSE HOUSE SUBSEQUENTLY SPECIFICALLY ACKNOWLEDED AND AGREED TO IN WEIGHT ONE OF CHANGE ORDER OR SEPARATE DOCUMENTATION, MANUFACTURER RECOGNIZES THAT RUBBER STAMPS ARE ROUTHENLY USED FOR INDICATING ANCHOR BOLT REACTIONS <u>II - Normal</u> <u>Is 1.0000</u> <u>Je 1.00</u> 1.2-1.3 OCCUPANCY/RISK CATEGORY ULTIMATE 110 MPH NOMINAL 85.21 MPH WIND EXPOSURE B ROOF FRAMING LAYOUT WIND LOAD DOCUMENTATION, MANUFACTURES RECOGNIZES THAT RUBBERS STAMES, AME ROBINITIES DE FORMANISMENTE DE L'ORIGINATION INTERNAL WIND COEF. -0.18 / 0.18 2.1-2.4 RIGID FRAME CROSS SECTION CLOSURE TYPE Enclosed 5.00 PSF ROOF SNOW LOAD 3.5 PSF Ce 1.0000 Ct 1.00 FRAMING COLORS 3-3.2 ENDWALL FRAMING LAYOUT GROUND SNOW LOAD | Rigid Frame: | RO | RO - Red Oxide | RO | Roger | Ro SNOW BANKING LOADS PER CODE 4-4.2 SIDEWALL FRAMING LAYOUT STEEL SYSTEM NOT SPECIFICALLY DETAILED FOR SPISMIC RESISTANCE. COLLATERAL DEAD LOAD 3 PSF 5-5.5 FRAMING DETAILS SAFETY COMMITMENT 20.00 PSF (REDUCIBLE Yes | Arguer | Fall ROOF LIVE LOAD ROOF PANELS & TRIM A) THE BULDING MAURICIDATE HAS A COMMINIST TO MAURICIDATE COLUMN BUILDING COMPONENTS THAT CAN BE SAFETY REPORTED, HOWEVER, THE SERETY COMMINIST AND JOB STIE PRACTICES OF THE ERECTOR ARE BEYOND THE CONTROL OF THE BULDING MANUFACTURER. B) IT IS STROWLY RECOMMENDED THAT SAFE WORKING COMDITIONS AND ACCIDENT PREVENTION PRACTICES BE THE TOP 2.000 PSF (FOR ROOF PANELS AND PURLINS) DEAD LOAD ROOF PANEL DETAILS SEISMIC SIDEWALL PANEL DETAILS SPECTRAL RESPONSE Ss 0.2146 St 0.0896 Sds 0.2283 Sdt 0.1424 IT IS STRONGLY RECOMMENDED THAT SAFE WORKING CONDITIONS AND ACCIDENT PREVENTION PRACTICES BE THE TOP PROPRIYED FAN YOUR SITE. LICAL, STATE AND FEDERAL SAFETY AND HEALTH STANDARDS SHOULD ALWAYS BE FOLLOWED TO HELP INSURE WORKER SAFETY, MAYE CERTAIN ALL EURIPOYEES KNOW THE SAFEST AND MOST PRODUCTIVE WAY OF ERECTING A BUILDING. BUILDINGENEY FROODLINES SHOULD BE WORNIN TO ALL EMPLOYEES. BUILY WEETINGS HOULDHING SAFETY PROCEDURES ARE ALSO RECOMMENDED. THE USES OF HARD HATS, RURBER SOLLS SHOES FOR FROOT WORK, PROPER EQUIPMENT FOR HADDLING MATERIAL, AND SAFETY NETS WHERE APPLICABLE, 8 ENDWALL PANEL DETAILS SITE CLASS d DESIGN RISK CATEGORY C Cs 0.0761 SPECIAL DETAILS RESPONSE MODIFICATION FACTOR R 3.000* FRAMES 3.000* BRACING BASIC SEISMIC FORCE RESISTING SYSTEM (LATERAL DIRECTIONS) = ORDINARY STEEL MOMENT FRAMES (A) BASIC SEISMIC FORCE RESISTING SYSTEM (ENDWALLS) = ORDINARY STEEL CONCENTRICALLY BRACED FRAMES WHEN GALVANIZED PROVIDED: ALL FINISHED PRIMARY BUILT-UP AND HOT ROLL MEMBE B) BASIC SEISMIC FORCE RESISTING SYSTEM (ENDWALLS) = ORDINARY STEEL MOMENT FRAMES GA ERECTOR / CONTRACTOR RESPONSIBILITIES A BASIC SEISUIC FORCE RESISTING SYSTEM (RSW) = ORDINARY STEEL CONC. BRACED FRAMES 30501 ARE HOT DIPPED GALVANIZED. ALL SECONDARY COLD FORMED MEMBERS ARE PRE-GALVANIZED. BASIC SEISMIC FORCE RESISTING SYSTEM (FSW) = ORDINARY STEEL MOMENT FRAMES BUILDING CODE SPECIFICATIONS REQUIRE CONSIDERATION OF SNOW SURCHARGES FOR ANY LOWER ROOF OF A STRUCTURE LOCATED WITHIN 20th. OF A HIGHER STRUCTURE. INFORMATION SUPPLIED TO THE METAL BUILDING SUPPLIER DOES NOT INDICATE PRESENCE OF A SHADOWING STRUCTURE WITHIN THIS 20th. ROAD GAINESVILLE, (BASIC SEISMIC FORCE RESISTING SYSTEM (LONGITUDINAL DIRECTIONS)= ORDINARY STEEL CONCENTRICALLY BRACED FRAMES A) IT IS THE RESPONSIBILITY OF THE ERECTOR/CONTRINCTOR TO INSURE THAT ALL PROJECT PLANS AND SPECIFICATIONS COMPTY WITH THE APPLICABLE REQUIREMENTS OF ANY CONTRINCE DULINON, AUTHORITIES. THE SUPPLYING OF SEALD ENGINEERING DATA AND DRAWINGS FOR THE METAL BULDING SYSTEM DOES NOT MAYLY OR CONSTITUTE AN ASPECIMENT THAT THE BULDING MANUFACTHER, OR ITS DESCRIBE NORMER IS ACTION AS THE ENGINEER OF RECORD OR DESCRIBE. ANALYSIS PROCEDURE = FOUNALENT LATERAL FORCE PROCEDURE ENVELOPE, AND AS SUCH, SNOW SURCHARGES HAVE NOT BEEN CONSIDERED IN THE DESIGN OF THE BUILDING(S) SHOWN ON THESE PLANS. SERVICEABILITY CRITERIA DUNAGAN GA. CANDLER PROFESSIONAL FOR A CONSTRUCTION PROJECT. B) THE CONTRACTOR MUST SECURE ALL REQUIRED APPROVALS AND PERMITS FROM THE APPROPRIATE AGENCY AS THIS PROJECT IS DESIGNED AS AN ENCLOSED BUILDING, ACCESSORIES (DOORS, WINDOWS, ETC.) BY OTHERS MUST BE DESIGNED AS "COMPONENTS AND CLADDING" IN ACCORDANCE TO SPECIFIC WIND PROVISIONS OF REFERENCED BUILDING CODE. MINIMUM DESIGN DEFLECTIONS **ACCREDITED** Roof Panel (Live) REQUIRED. C) APPROVAL OF THE MANUFACTURER'S DRAWINGS AND CALCULATIONS INDICATE THAT THE BUILDING MANUFACTURER GAINESVILLE, C) APPROVAL OF THE MANUFACTURER'S DRAWINGS AND CALCULATIONS INDICATE THAT THE BULLIONS MANUFACTURER CORRECTLY INTERPRETED AND APPLIED THE REQUIREMENTS OF THE CONTRACT DRAWINGS AND SPECIFICATIONS. (SECT. 4.4.1 ASC CODE OF STANDARD PRACTICES, LAITST ED.) DI WHERE DISCREPANICES DIST EVEREN THE MANUFACTURER'S STRUCTURAL STEEL PLANS AND THE PLANS FOR OTHER TRADES, THE STRUCTURAL STEEL PLANS SHALL DOVERN. (SECT. 3.3 ASC CODE OF STANDARD PRACTICE LAIEST ED.) DI ESSIAC CONSERVATION OF ANY MATERIALS IN THE STRUCTURE WHICH ARE NOT PURNISHED BY THE BULLDING MANUFACT— MANUFACTURER ARE THE SES SPECIFICALLY INDICATED. THE GENETORY CONTRACTOR IS RESPONSIBLE FOR ALL BERGOTON OF STEEL AND ASSOCIATED WORK IN COMPUNACE WITH THE BULLDING MANUFACTURER'S "FOR CONSTRUCTION" DRAWINGS. O) PRODUCTS SHIPPED TO ERECTORY/CONTRACTOR OR HIS CUSTOMER SHALL BE INSPECTED BY ERECTORY/CONTRACTOR MANUFACTURER IN WITHOUT WITH THE MANUFACTURER WITH THE MANUFACTURE WITH THE MANUFACTURER WITH THE MANUFACTURER WITH THE MANUFACTURE W Metal Building Systems Endwall Rafter (Live) = 180 Roof Panel (Wind) = 60 Endwall Rafter (Wind) = 180 Rigid Frame (Horz) = 60 JOBSITE: FOR OCCUPANCY (RISK) CATEGORY I OR II, IBC PROVISIONS INDICATE THAT PADEN Wall Girt = 90 Rigid Frame (Vert) = 180 SINGLE-STORY BUILDINGS SHALL HAVE "NO DRIFT LIMIT" PROVIDED THAT 2010 INTERIOR WALLS, PARTITIONS, CELINGS AND EXTERIOR WALL SYSTEMS HAVE BEEN DESIGNED TO ACCOMMODATE THE SEISMIC STORY ORIFITS. INTERIOR WALLS, PARTITIONS, CELINGS OR EXTERIOR SYSTEMS MOY PROVIDED BY MAIN SHALL BE DESIGNED AND DETAILED BY OTHERS TO ACCOMMODATE THE SEISMIC Roof Purlin (Live) = 150 Rigid Frame (Seismic) = 50 Roof Purlin (Wind) = 150 BUILDING DESIGNED & MANUFACTURED BY AN IAS ACCREDITED FACILITY GENERAL NOTES A) THE STRUCTURE UNDER THE CONTROLT HAS BEEN DESIGNED AND DEFALED FOR THE LOADS AND CONDITIONS STRULATED IN THE CONTRACT AND SKOWN ON THESE DRAWNISS, ANY ALTERATIONS TO THE STRUCTURAL SYSTEM OR REMOVAL OF ANY COMPONENT PARTS, OR THE ADDITION OF OTHER CONSTRUCTION MATERIALS OR LOADS MUST BE DONE WINDER THE ADVICE AND DIRECTION OF A REGISTRED ARGUITCT, CIM. OR STRUCTURAL EMPONE TO DIRECTION OF THE ADVICE AND DIRECTION OF A REGISTRED ARGUITCT, CIM. OR STRUCTURAL EMPONE TO B) THIS METAL BULDING IN DESIGNED WITH THE BULDING MANUFACTURES'S STRAWARD PROCITICS WHICH ARE BASED THIS METAL BULDING IS DESIGNED WITH THE BULDING MANUFACTURES'S STRAWARD PROCITICS WHICH ARE BASED AFTER THE ERECTOR/CONTRACTOR LEARNS OF THE DEFECT. THE MANUFACTURER WILL NOT BE LURIE FOR ANY DEFECT CALM IS MADE WITHIN ONE (1) YEAR AFTER DATE OF THE ORIGINAL SHEWEST BY THE MANUFACTURER TO CONTRACTOR OR HIS CUSTOMER. THE MANUFACTURER WILL BE CIVEN A REASONABLE OPPORTUNITY TO INSPECT DETECTIVE MATERIALS UPON RECEIPT OF CALM BY COMMITCIOR. IF A DEFECT IS OF SUCH MATURE THAT IT CAN BE REMEDIED BY A FIELD OPPATION AT THE JOB SIZE WITHOUT THE MECESSITY OF RETURNING THE MATERIAL TO THE MANUFACTURER, THEN UPON WRITTEN AUTHORIZATION OF THE MANUFACTURER THE CONTRACTOR THAT REPART OF THE MANUFACTURER, THEN UPON WRITTEN AUTHORIZATION OF THE MANUFACTURER THE CONTRACTOR THAT REPART OF THE MANUFACTURER. ON PERTINENT PROCEDURES AND RECOMMENDATIONS OF THE FOLLOWING ORGANIZATIONS AND CODES 1. AMERICAN INSTITUTE OF STEEL CONSTRUCTION: "AISC SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS—ALLOWABLE STRESS DESIGN" AS ADOPTED BY THE BUILDING CODE REFERENCED IN "BUILDING LOADS" SECTION "A" ABOVE. OB 2. AMERICAN, IROM, AND STEEL, INSTITUTE. "SPECIFICATION FOR THE DESIGN OF COLD FORMED STEEL STRUCTURAL MEDIBERS" AS ADOPTED BY THE BUILDING CODE REFERENCE) IN "BUILDING LOADS" SECTION "A" ABOVE. 3. AMERICAN WELDING SOCIETY. "STRUCTURAL WELDING CODE" AWS DI.1. AS ADOPTED BY THE BUILDING CODE 30501 THE CORRECTION OF MINOR MISFITS BY THE USE OF DRIFT PINS TO DRAW THE COMPONENTS IN TO LINE, MODERATE AMOUNTS OF REAMING-DIPPING AND CUTTING, AND THE REPLACEMENT OF MINOR SHORTAGES OF MATERIAL ARE A NORMAL PART OF RECENTION AND ARE NOT SUBJECT TO CLUM. BUILDINGS REFERENCED IN "BUILDING LOADS" SECTION "A" ABOVE. 4. METAL BUILDING MANUFACTURER'S ASSOCIATION: "LOW RISE BUILDING SYSTEMS MANUAL" AS ADOPTED BY THE BUILDING PART OF ERECTION AND ARE NOT SOMELET TO LOSAN. H. ALL BROKEN AS SHOWN AND PROVIDED BY THE MANUFACTURER FOR THIS BUILDING IS REQUIRED AND SHALL BE INSTALLED BY THE ERECTIOR AS A PERMANENT PART OF THE STRUCTURE. TO THE PROPERTY OF SHALL WE WILL BE DETENDED AND FINANCIAL ORDERING OR OTHER ELEMENTS REQUIRED TO THE REPORT OF SHALL WILL BE DETENDED AND FINANCIAL DISTRICT OF THE STEEL FRAMING, AGAINST TEMPORARY SUPPORTS WILL SEQURE THE STEEL FRAMING, OR ANY PARTLY ASSEMBLED STEEL FRAMING, AGAINST LOADS COMPARABLE IN INTERSTYT TO THOSE FOR WHICH THE STEEL FRAMING FROM THE PERFORMANCE OF WORK BY OR THE ATTO FOR OTHER SHALL BY THE THE OWNER FROM THE PERFORMANCE OF WORK BY OR THE ATTO FOR OTHERS, HOW SUCH UNPREDICTIVED AND SECURITIES FROM THE PERFORMANCE OF WORK BY OR THE ATTO FOR OTHERS, HOW SUCH UNPREDICTIVED LOADS IN THESE DUE TO TORMADO, PERSONANCE OF WORK BY OR THE ATTO FOR OTHERS, HOW SUCH UNPREDICTIVED LOADS IN THESE DUE TO TORMADO, PERSONANCE OF WORK BY OR THE ATTO FOR OTHERS, HOW SUCH UNPREDICTIVED LOADS IN THESE DUE TO TORMADO, PERSONANCE OF WORK BY OR THE ATTO FOR OTHERS, HOW SUCH UNPREDICTIVED LOADS IN THESE DUE TO TORMADO, PERSONANCE OF WORK BY OR THE ATTO FOR OTHERS, HOW SUCH OF MERCHANDES DUE TO THE PERSONANCE OF WORK BY OR THE ATTO FOR OTHERS, HOW SUCH UNPREDICTIVED LOADS IN THE SUCH OR THOUSE DUE TO THE PERSONANCE OF WORK BY OR THE ATTO FOR OTHERS, HOW SUCH OF MERCHANDES DUE TO THE PERSONANCE OF WORK BY OR THE ATTO FOR THE PERSONANCE OF WORK BY OR THE ATTO FOR THE PERSONANCE OF WORK BY OR THE ATTO FOR THE PERSONANCE OF WORK BY OR THE ATTO FOR THE PERSONANCE OF WORK BY OR THE P SE001188 MATERIAL DULLINGS MANUTANI OTHERS ASSOCIATION: OTHER DELICITION STATEMS WITHOUT AS AGOST TO THE DESIGN THREE DULLINGS SECTION (A" ABOVE. (1) MATERIAL PROPERTIES OF SIEEL PLATE USED IN THE FARRICATION OF FRIMARY RIGID FRAMES, AND OTHER PRIMARY STRUCTURAL EXCLUSIVE OF COLD—FORMED SECTIONS, CONFORM TO ASTIM—A529 OR A572. FLANGS WEB MATERIAL CONFORMS TO ASTIM—A529 OR A572 GRADE 55 WITH A MINIMUM YELD POINT OF 55,000 psi. (1) MATERIAL CONFORMS TO ASTIM—A529 OR A572. (2) PARTICIPATION OF THE PRIMARY STRUCTURE OF T GA STRUCTURAL ST. GAINESVILLE, METAL 2) MATERIAL PROPERTIES OF HSS ROUND SECTIONS CONFORM TO ASTM-A500, GRADE B OR C WITH A MINIMUM YIELD PINE POINT OF 42,000 psi. 3) MATERIAL PROPERTIES OF HSS RECT. OR SQUARE SECTIONS CONFORM TO ASTM-A500, GRADE B OR C WITH A MINIMUM BRAD (SECT. 7.10.3 ASC CODE OF STANDARD PRICTICE, LATEST ED.) PLANS PREPARED BY MBM ARE INTERNED TO SHOW ONLY LOCATION, DAMETER AND PROJECTION OF THE ANCHOR RODS REQUIRED LIVERS PREPARED BY MBM ARE INTERNED TO SHOW ONLY LOCATION, DAMETER AND PROJECTION OF THE ANCHOR RODS REQUIRED THELD POINT OF 45000 ps. MATERIAL PROPRIETS OF HOT ROLLED CHANNEL AND ANQLE MEMBERS CONFORM TO THE REQUIREMENTS OF ASTM-4992 WITHINIMUM MIELD POINT OF 50,000 FSI. HOT ROLLED W-SHAPED MEMBERS CONFORM TO THE REQUIREMENTS OF -ROM-2/16/24 908 TO ATTACH THE METAL BUILDING SYSTEM TO FOUNDATION, IT IS RESPONSIBILITY OF THE END CUSTOMER TO ENSURE THAT ADEQUATE 0 PROVISIONS ARE MADE FOR SPECIFYING ROD EMBEDMENT, BEARING VALUES, TIE ROOS AND OTHER ASSOCIATED ITEMS EMBEDDED IN THE ASTM-ASSZWIM MINIMUM YELD POINT OF SQUOD PS. MATERIAL PROPERTIES OF COLD FORMED LEGHT GAGE STEEL MEMBERS CONFORM TO EITHER ASTM A653-06 OR 55 OR MATERIAL PROPERTIES OF ROOM FOR THE MEMBERS OF THE MEMBERS CONFORM TO ASTM-A792 GRADES 80 CLASS 1, 2 OR 3 WITH A MINIMUM YELD STREAMONT OF SQUOD PS. CONTING OF BESE MATERIAL 55 SX ALUMINUM—ZEND CALLOY IN ACCORDANCE WITH A255 SPECIFICATIONS. 7 CABLE JULIZED FOR REACING CONFORMS TO ASTM A475. CASILE BRACING IS TO BE INSTALLED TO A TAUT COLORS: CONCRETE FOUNDATION, AS WELL AS FOUNDATION DESIGN FOR THE LOADS IMPOSED BY MB SYSTEM, OTHER IMPOSED LOAD, AND THE BEARING CAPACITY OF THE SOIL AND OTHER CONDITIONS OF THE BUILDING SITE (MBMA 06 SECTIONS 3.2.2 AND A3) GALVALUME DRAWING STATUS 8452 K) METAL BUILDING MANUFACTURER DOES NOT PROVIDE ANY FIELD SUPERVISION FOR THE ERECTION, NOR DOES ASH GRAY FOR APPROVAL. FINESE DRAWNIOS BEING FOR APPROVAL. ARE BY DEFINITION NOT FINAL, AND ARE FOR CONCEPTUAL REPRESENTATION ONLY PROPERTY AND ARE FOR CONCEPTUAL REPRESENTATION OF THE PROPERTY OF T MBM PERFORM ANY INSPECTIONS DURING OR AFTER ERECTION. 2/ 2/24 GABLE: CHARCOAL GRAY CONSTRUIT. 10 SHOULD USE POR BEACKING MARKERS COKPORN TO ASTMI—ASS WITH MINIMAM PILED POINT OF 36,000 PSI. 9) IT IS THE RESPONSBULLY OF ERECTOR TO ENSURE PROPER BOLT TIORMESS IN ACCORDANCE WITH APPLICABLE RECS SPECIORICATION FIRST REMOUTBLE. JUNIS USING A-325 OR A-490 BOLTS*, ALL A-325 BOLTS IN PRIMARY FRAUNCHMENT BOLTS PROPERTY AS FOLLOWS: 17 LYLLY—PERTABORY A-325 BOLTS IF. COMPONENTS & CLADDING (unfactored) EAVE: CHARCOAL GRAY SCALE CTW NONE Wall Field Values = 16.843 psf / -18.247 psf ASH GRAY Wall Edge Values = 16.843 -22.457 psf FOR PERMIT. FOR PERMIT. THESE DRAWINGS, BEING FOR PERMIT, ARE BY DEFINITION NOT FINAL IN THAT, AS A MINIMUM, PIECE MARKINGS ARE NOT IDENTIFIED, ONLY DRAWINGS ISSUED FOR CONSTRUCTION "CAN BE CONSTRUCTION" CAN BE CONSTRUCTION. CS: CHARCOAL GRAY COVER PAGE DESCRIPTION ACCOUNT ON A HORIZON ESSENCE AREA. FOR IBC—BASED CODE, "HIGH SEISMIC AREA" IS DEFINED AS "SEISMIC DESCRIPTION CATEGORY" OF "D", "E" OR "F". B DILLIONS SUPPORTS A CARME SYSTEM WITH A CAPACITY GREATER THAN 5.00 TONS. COMPONENTS & CLADDING (unfactored) CHITTER-CHARCOAL GRAY IIMBER . Wall Field Values = 16.843 psf / -16.843 psf HORNET STEEL BUILDINGS, INC. DOWNSPOURS: CHARCOAL GRAY PAGE Wall Edge Values = 16.843 psf / -30.879 psf c) BUILDING SUPPORTS MACHINERY THAT CREATES VIBRATION, IMPACT OR STRESS - REVERSALS ON THE CONNECTIONS. GA COA∯: PEF006160 EXPIRATION: 06/30/2024 FOR CONSTRUCTION: THESE DRAWINGS ARE FINAL AND ISSUED FOR FIELD USE FOR BUILDING ERECTION

d) ANY CONNECTION DESIGNATED IN THESE DRAWINGS AS "A-325 - SC".

BURNISHED SLATE







RIGID FRAME: ANCHOR BOLTS & BASE PLATES Frm Col Anc._Bolt Base_Plate (in) Grout Line Line Qty Dia Width Length Thick (in)

4 0.750 6.000 8.500 0.375 0.0 4 0.750 6.000 8.500 0.375 0.0

3* Frame lines: 3 4 5

								_
END	WALL	COL	LUMN:	ANC	HOR BOL	TS & E	ASE PLATES	S
Frm Line	Col Line	Anc. Qty	_Bolt Dia	Base. Width	_Plate (i Length	n) Thick	Grout (in)	
2	Α	4	0.750	8.000	8.000	0.375	0.0	
2	С	4	0.750	8.000	8.000	0.375	0.0	
2	E	4	0.750	8.000	8.000	0.375	0.0	
2	F	4	0.750	8.000	8.000	0.375	0.0	
6	F	4	0.750	8.000	8.000	0.375	0.0	
6	D	4	0.750	8.000	8.000	0.375	0.0	
6	В	4	0.750	8.000	8.000	0.375	0.0	
6	Α	4	0.750	8.000	8.000	0.375	0.0	

NOTES FOR REACTIO	NS
	on = 60.0 = 100.0 = 20.0 / 20.0 = 20.0 / 20.0 = 2.0 = 3.0 = 20.0 = 12.0 = 12.0 = 12.0 = 112.0

BUIL	DINC) B	RAC	ING	REA	ACTIO	SNC			
——Wa	II	Col Line	Н	± Wi lorz	Reactions nd — Vert	ons(k — Sei: Horz) smic- Vert	Panel (lb, Wind	Shear (ft) Seis	Note
L_EW F_SW R_EW B_SW	2 F 6 A	C,E 3,4 D,B 4,3		2.3 2.6 2.3 5.1	2.1 3.7 2.3 3.7	0.5 0.9 0.5 1.8	0.5 1.3 0.5 1.3			(b)
(b)Wind	d ben	t in	bay,	base	above	finis	h floor			

Reactions for seismic represent shear force, Eh

RIG	D FRAM	ΜE: E	ASIC COL	UMN RE	ACTIONS	(k)							
Fram Line 3* 3*	e Column Line A F	Horiz 1.1 -1.1	Dead Vert 2.3 2.3	Colle Horiz 1.3 -1.3	vert 2.3 2.3	Horiz 5.2 -5.2	Vert 9.0 9.0	Horiz 1.5 –1.5	-Snow Vert 2.6 2.6	Wind Horiz -8.8 0.4	Left1- Vert -11.9 -6.9	-Wind_ Horiz -0.4 8.8	Right1- Vert -6.9 -11.9
Fram Line 3* 3*	e Column Line A F	Wind Horiz -8.5 -0.1	_Left2- Vert -7.1 -2.1	-Wind_ Horiz 0.1 8.5	Right2- Vert -2.1 -7.1	Wind Horiz -2.4 2.8	_Long1- Vert -10.3 -8.4	Wind Horiz -2.8 2.4	i_Long2- Vert -8.4 -10.3	-Seism Horiz -0.4 -0.4	ic_Left Vert -0.3 0.3	Seismic Horiz 0.4 0.4	_Right Vert 0.3 -0.3
Fram Line 3* 3*	e Column Lîne A F	-MIN_S Horiz 2.2 -2.2	NOW Vert 3.7 3.7	F1UNB Horiz 1.6 -1.6	SL_L- Vert 2.9 1.7	F1UNB_ Horiz 1.6 -1.6	SL_R- Vert 1.7 2.9						
3*	Frame lin	nes:	3 4 5	i									

END	WALI	COL	LUMN:			REACTIONS Wind		Wind	Wind	Wind	Wind	Wind	Wind
Frm Line 2 2 2 2 2	Col Line A C E F	Dead Vert 0.4 1.2 0.9 0.3	Collat Vert 0.3 1.0 0.8 0.2	Live Vert 2.3 6.3 5.1 1.6	Snow Vert 0.4 1.1 0.9 0.3	Left1 Vert -2.0 -5.4 -2.5 -1.0	Wind Right1 Vert -1.3 -3.5 -4.7 -1.3	Left2 Vert -1.3 -3.7 -1.1 -0.4	Right2 Vert -0.5 -1.9 -3.3 -0.8	Press Horz -1.7 -3.5 -3.0 -1.2	Suct Horz 1.7 3.5 3.0 1.2	Long1 Vert -2.2 -5.1 -2.5 -1.0	Long2 Vert -1.3 -3.5 -4.5 -1.5
Frm Line 2 2 2 2	Col Line A C E F	Seis Left Vert 0.0 0.0 0.0	Seis Right Vert 0.0 0.0 0.0	Seis Long Vert 0.0 0.0 0.0	-MIN Horz 0.0 0.0 0.0	SNOW (Vert 0.6 1.5 1.3 0.4	E1UNB_SL_ Horz Vert 0.0 0.4 0.0 1.6 0.0 0.3 0.0 0.1	0.0 0.0 0.0	8_SL_R- Vert 0.1 0.7 1.4 0.2				
Frm Line 6 6 6	Col Line F D B A	Dead Vert 0.4 0.9 0.9 0.4	Collat Vert 0.3 0.8 0.8 0.3	Live Vert 2.0 5.2 5.2 2.0	Snow Vert 0.4 1.0 1.0	Wind Left1 Vert -1.8 -4.9 -2.8 -1.2	Wind Right1 Vert -1.2 -2.8 -4.9 -1.8	Wind Left2 Vert -1.1 -3.5 -1.4 -0.5	Wind Right2 Vert -0.5 -1.4 -3.5 -1.1	Wind Press Horz -1.5 -3.1 -3.1	Wind Suct Horz 1.6 3.1 3.1	Wind Long1 Vert -2.0 -4.7 -2.8 -1.2	Wind Long2 Vert -1.2 -2.8 -4.7 -2.0
Frm Line 6 6 6	Col Line F D B A	Seis Left Vert 0.0 0.0 0.0	Seis Right Vert 0.0 0.0 0.0	Seis Long Vert 0.0 0.0 0.0	-MIN Horz 0.0 0.0 0.0 0.0	SNOW F Vert 0.5 1.4 1.4 0.5	2UNB_SL_ florz Vert 0.0 0.3 0.0 1.5 0.0 0.5 0.0 0.1	0.0	8_SL_R- Vert 0.1 0.5 1.5 0.3	0.0	_1- E2P fert Hor. 1.9 0. 5.7 0. 2.2 0. -0.2 0.	0 -0.2 0 2.2 0 5.7	
Frm Line 6 6 6	Col Line F D B A	E2PAT_ Horz 0.0 0.0 0.0 0.0	Vert F 2.2 2.6	0.0 -	_4- ort 0.3 2.7 2.7 0.3								

ANCHOR BOLT SUMMARY

Dia (in) Type

5/8" A307
3/4" GR36
3/4" GR36 Qty Locate

\$\text{Q}\$ 16 Jamb
\$\tilde{\to}\$ 32 Endwall
\$\tilde{\to}\$ 24 Frame 1.50 1.50 2.50

GENERAL NOTES

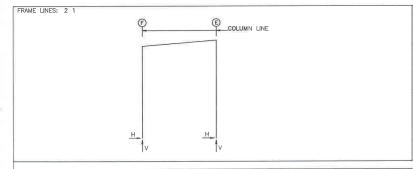
- FOUNDATION DESIGN AND CONSTRUCTION ARE NOT THE RESPONSIBILITY OF METAL BUILDING MANUFACTURER.
- 2. ALL REACTIONS ARE UNFACTORED.
- 3 ULTIMATE WIND LOADS ARE USED TO DERIVE THE WIND REACTION.
- ANCHOR BOLTS SHALL BE ACCURATELY SET TO A TOLLERANCE OF +/- 1/8" IN BOTH ELEVATION AND LOCATION.
- 5. COLUMN BASE PLATES ARE DESIGNED NOT TO EXCEED A BEARING PRESSURE OF 1050 POUNDS PER SQUARE INCH.





	IS	SUE	DET	CHK	DATE
					- 1
	DCI METAL BUIL	DINGS			.2024
	PADEN DUNAGA	N			4
	JOB NO: 8452		DATE:	/ 5/	24 🛱
	GAINESVILLE, G	4 30501			²⁴ E
	ANCHOR BOLT	REACTIONS			7
1	DRAWING NO: PAGE 1.2	DRAWN BY: CTW	SPW BY	30	NONE

NOTE: THE FRAMING AT BOTH ENDWALLS IS NOT DESIGNED TO ACCOMMODATE FUTURE RELIBERATIONS. REACTIONS CORRESPONDING TO THESE FRAME LINES OR ACCOUNT OF THE PROPERTY OF THE PROPERT



RIGID FRAME: ANCHOR BOLTS & BASE PLATES

Frm Col Anc._Bolt Base_Plate (in) Grout Line Line Qty Dia Width Length Thick (in) 4 0.750 6.000 8.000 0.375 0.0 4 0.750 6.000 8.000 0.375 0.0

2* Frame lines: 2 1

ANCHOR BOLT SUMMARY

	Qty	Locate	Dia (în)	Туре	Projection (in)
0	16	Frame	3/4"	GR36	2.50

GENERAL NOTES

- FOUNDATION DESIGN AND CONSTRUCTION ARE NOT THE RESPONSIBILITY OF METAL BUILDING MANUFACTURER.
- 2. ALL REACTIONS ARE UNFACTORED.
- 3 ULTIMATE WIND LOADS ARE USED TO DERIVE THE WIND REACTION.
- ANCHOR BOLTS SHALL BE ACCURATELY SET TO A TOLLERANCE OF +/- 1/8" IN BOTH ELEVATION AND LOCATION.
- 5. COLUMN BASE PLATES ARE DESIGNED NOT TO EXCEED A BEARING PRESSURE OF 1050 POUNDS PER SQUARE INCH.

Τ	RIGIE	FRAN	/E: в	ASIC COL	UMN REA	CTIONS ((k)							
	Frame Line 2* 2*	Column Line F E	Horiz 0.0 0.0	Dead Vert 0.4 0.4	-—-Collo Horiz 0.0 0.0	teral- Vert 0.2 0.2	Horiz 0.1 -0.1	Vert 1.3 1.3	Horiz 0.0 0.0	-Snow Vert 0.2 0.2	Wind Horiz -1.3 -1.4	_Left1- Vert -3.2 0.8	-Wind_ Horiz 1.7 1.4	Right1- Vert 1.4 -3.8
1	Frame Line 2* 2*	Column Line F E	Wind Horiz -1.8 -1.1	Left2- Vert -2.1 2.1	-Wind_ Horiz 1.2 1.7	Right2- Vert 2.3 -2.2	Wind Horiz 0.0 -1.6	Long1- Vert -3.6 0.8	Wind Horiz 1.3 0.2	_Long2- Vert 2.2 -2.1	Seism Horiz 0.1 0.1	ic_Left Vert -0.2 0.2	Seismic Horiz 0.1 0.1	_Right Vert 0.2 -0.2
	Frame Line 2* 2*	Column Line F E	-Seismi Horiz -0.1 -0.1	ic_Long Vert -0.2 0.2	Wind Horiz -1.3 -1.5	_Left3- Vert -2.5 1.7	-Wind_ Horiz 1.6 1.3	Right3- Vert 1.9 -2.6	Wind Horiz -1.7 -1.0	_Left4- Vert -2.8 1.2	-Wind_ Horiz 1.2 1.8	Right4— Vert 1.8 -3.4	-MIN_S Horiz 0.0 0.0	NOW Vert 0.3 0.3
	2*	Frame lin	ies:	2 1										

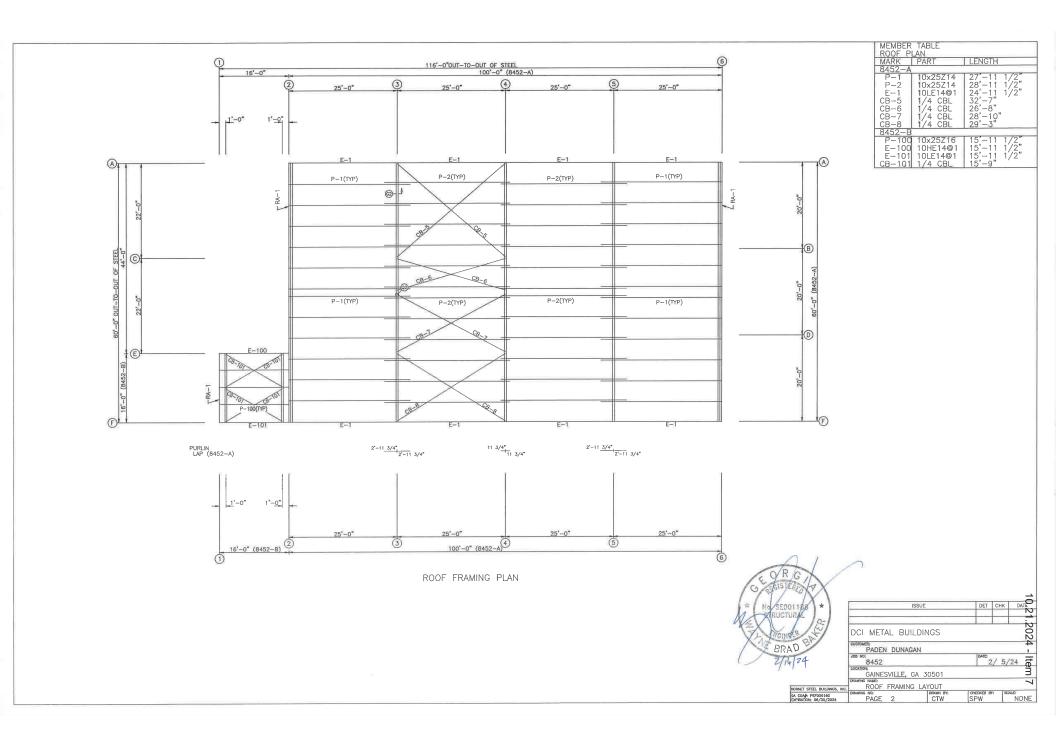
NOTES FOR REACTIONS Building reactions are based on the following building data: Width (ft) = Length (ft) = Length (ft) = Roof Slope (Rise/12) = Roof Slope (Rise/12) = Dead Load (psf) = Length (psf) = Leng = 16.0 = 16.0 = 20.0/ 21.3 = 1.0 = 2.0 = 2.0 = 3.5 = 110.0 = GSBC 24 (IBC 18) = Enclosed = 1.00 = 1.00 = 0.34 Clósure Importance Wind Importance Seismic Seismic Zone Seismic Coeff (Fa*Ss)

BUILDIN						Panel_Shea	r
Loc Line	- Col Line	Horz	ind — Vert	—Sei Horz	Ismic — Vert	Panel_Shea (lb/ft) Wind Seis	Note
L_EW 2 F_SW E	2,1	2.9	4.1	0.2	0.3		(h)
B_SW F	Torsio	nal Bra	cing (lsed			(11)
(h)Rigid fro	ame at	endwall					
Reactions	for seisr	nic rep	resent	shear	force,	Eh	



DET CHK DATE 21.2024 ISSUE DCI METAL BUILDINGS PADEN DUNAGAN
JOB NO:
8452 GAINESVILLE, GA 30501 WE NAME:
ANCHOR BOLT REACTIONS
NG NO:
PAGE 1.3 DRAWN BY:
CTW PAGE 1.3 NONE

NOTE: THE FRAMING AT BOTH ENDWALLS IS NOT DESIGNED TO ACCOMMODATE FUTURE ADDITIONS. REACTIONS CORRESPONDING TO THESE FRAME LINES
REFLECT LOADINGS FOR ACTUAL TRIBUTARY AREA AND ARE NOT INTENDED TO INCLUDE ANY FUTURE MODIFICATIONS UNLESS NOTED OTHERWISE.

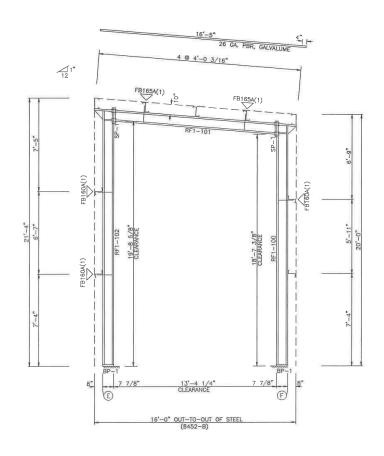


SPLICE BOLT TABLE | Qty | Top Bot Int TYPE DIA Length | 4 4 0 A325 5/8" 2"

BASE PLATE TABLE COL PLATE SIZE
MARK Width THICK Length
BP-1 6° 3/8° 8"

FLANGE BRACES: (1) One Side; (2) Two Sides FBxxA(1): xx=length(in)
A - L2x2x14

MEMBER SIZE TABLE WEIGHT 227 161 239 MARK MEMBER LENGTH RF1-100 W8X10 RF1-101 W8X10 RF1-102 W8X10



RIGID FRAME ELEVATION: FRAME LINE 1

NOTE: THE FRAMING AS DEPICTED ABOVE IS NOT DESIGNED TO ACCOMMODATE ANY FUTURE EXPANSION. 6



DET CHK DATE ISSUE DCI METAL BUILDINGS CUSTOMER:
PADEN DUNAGAN
WOR NO:
8452
LOCATION: 2/ 5/24 **E** DRAWING NO.

DRAWING PAGE 2.1 CTW

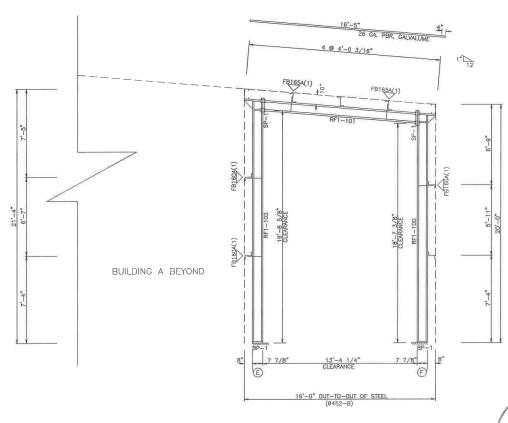
HORNET STEEL BUILDINGS, INC. GA COAF: PEFD06160 EUPIRATION: 06/30/2024

NONE

SPLICE BOLT TABLE

FLANGE BRACES: (1) One Side; (2) Two Sides FBxxA(1): xx=length(in)
A - L2x2x14

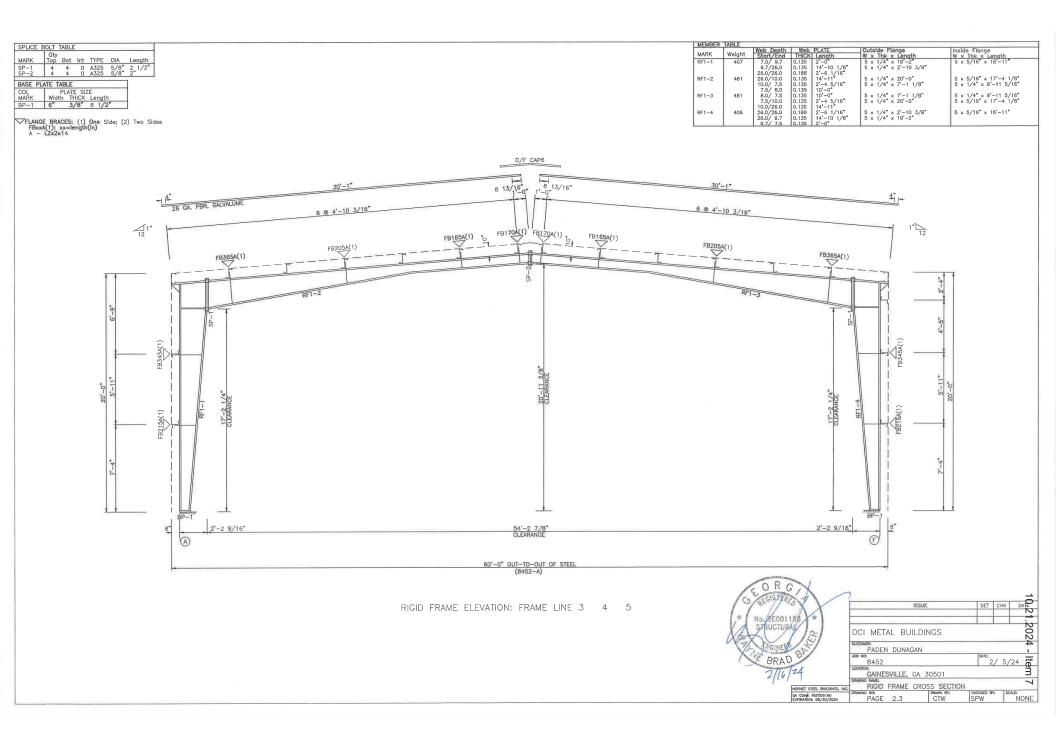
WEWBEK S	IZE TABLE	,	
MARK	MEMBER	LENGTH	WEIGHT
RF1-100	W8X10	19'-2 5/8"	227
RF1-101	W8X10	13'-4 7/16"	161
DE1_102	Weyto	20'-5 5/16"	230

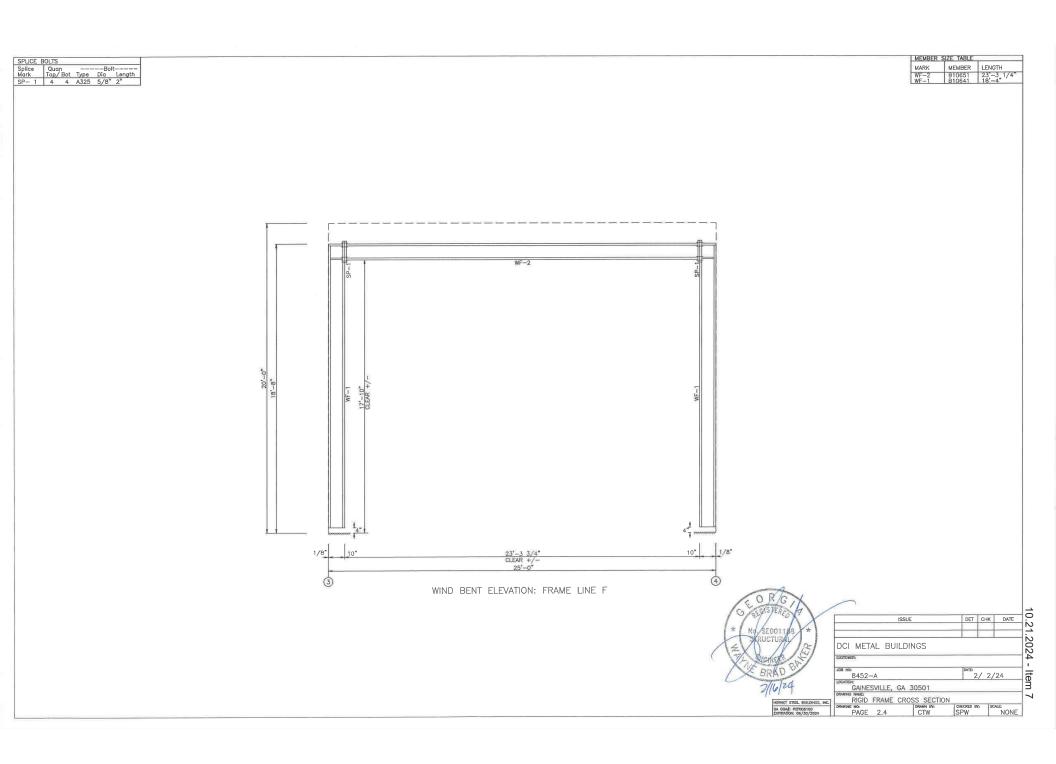


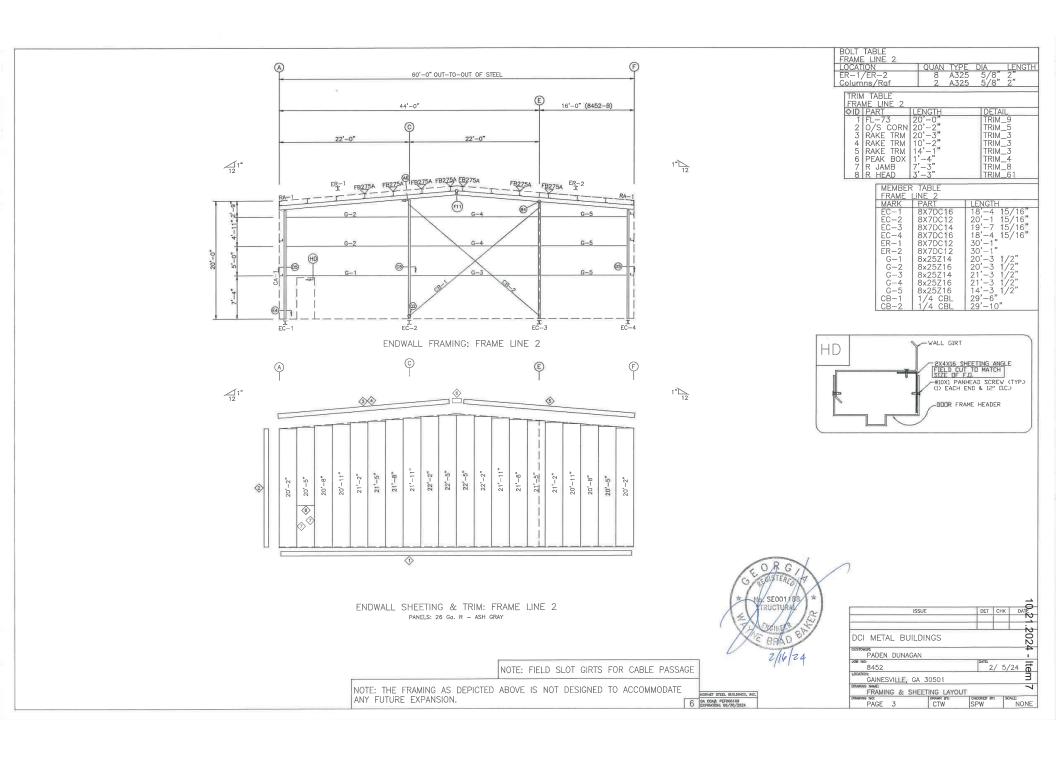
RIGID FRAME ELEVATION: FRAME LINE 2

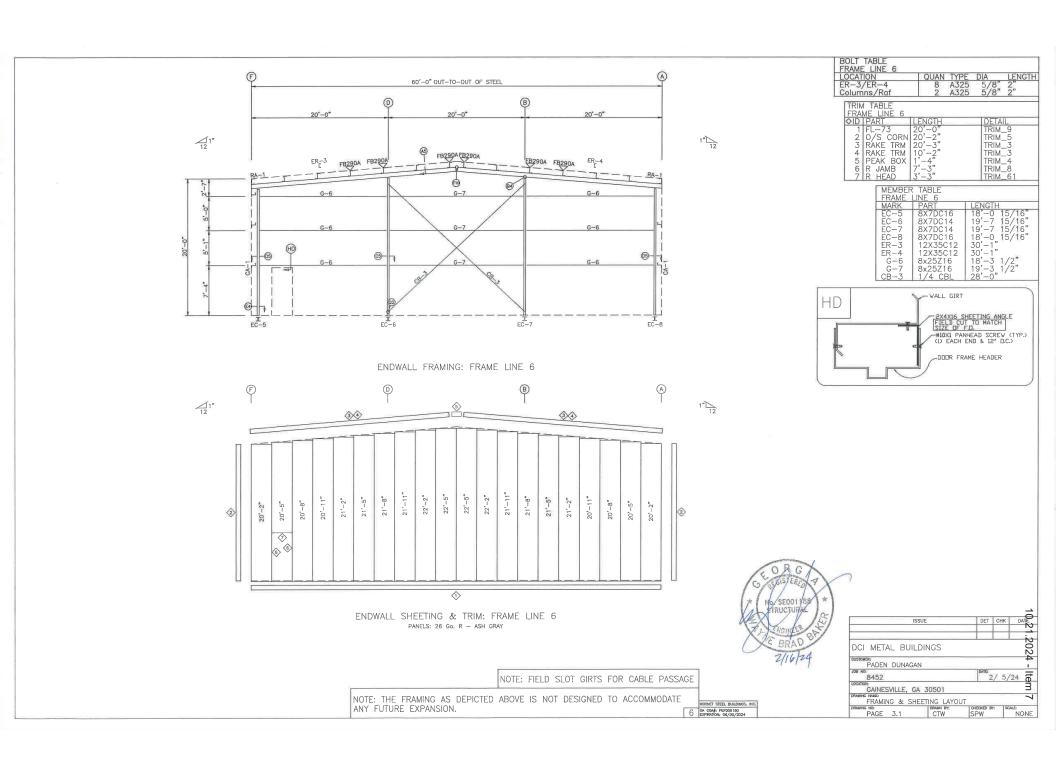
ORG DET CHK DATE 21. ISSUE DCI METAL BUILDINGS PADEN DUNAGAN
JOB NO:
8452
LOCATION 2/ 5/24 🛱 GAINESVILLE, GA 30501 DRAWING NAME
RIGID FRAME CROSS SECTION
DRAWING NO:
PAGE 2.2
DRAWING BT:
CTW

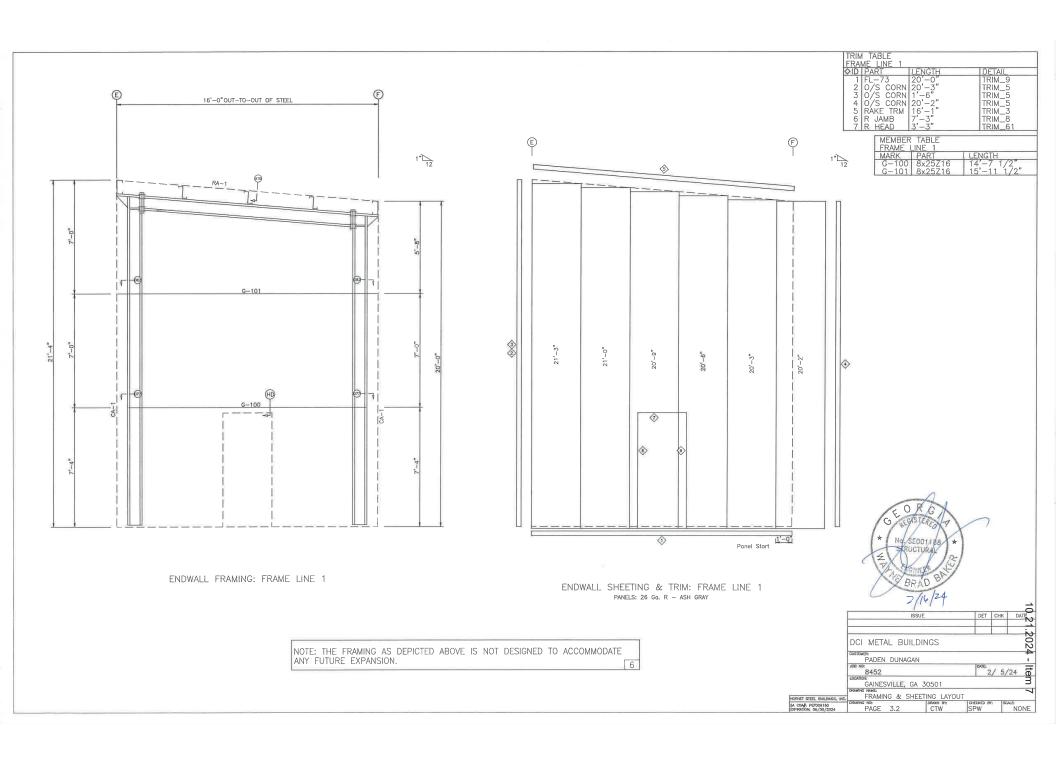
SPW SPW NONE

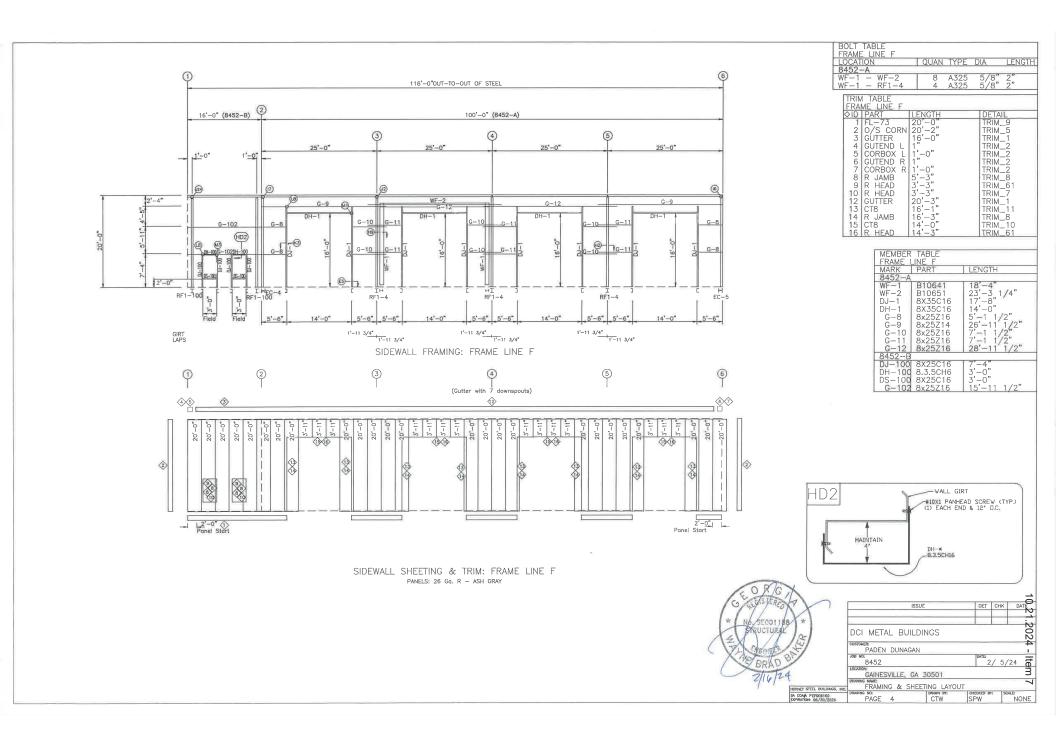


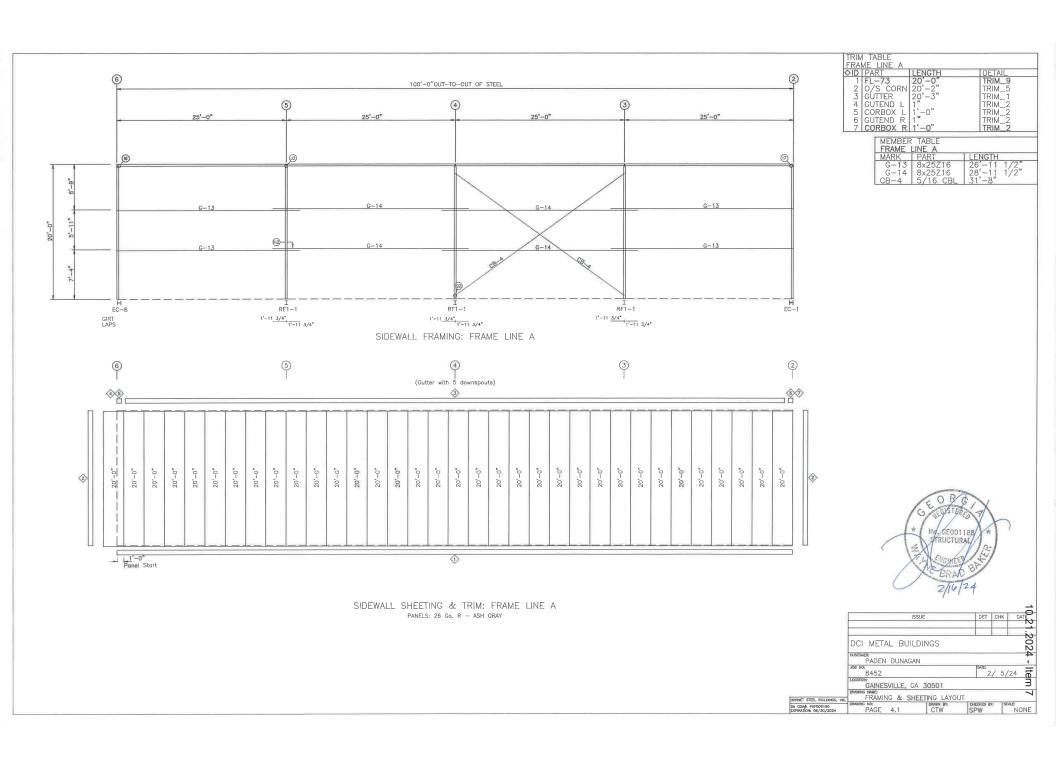


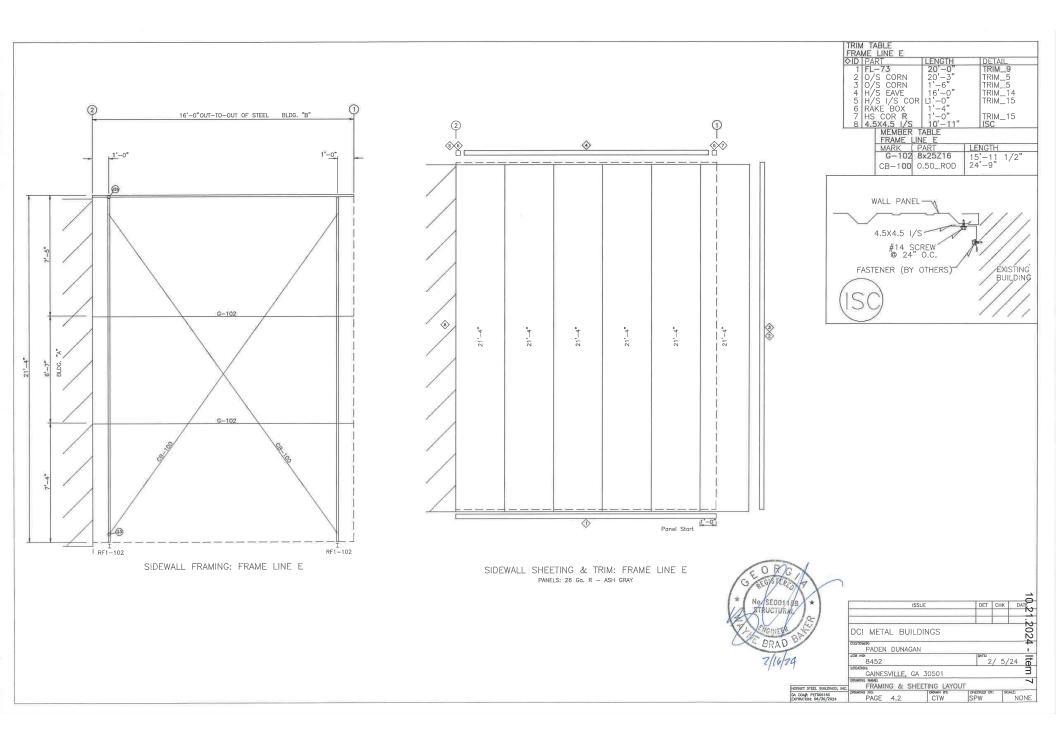


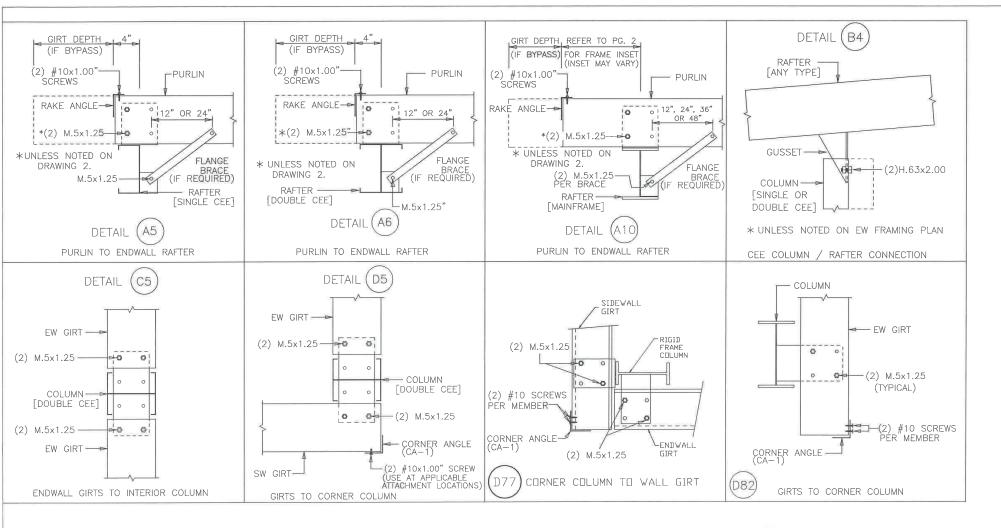




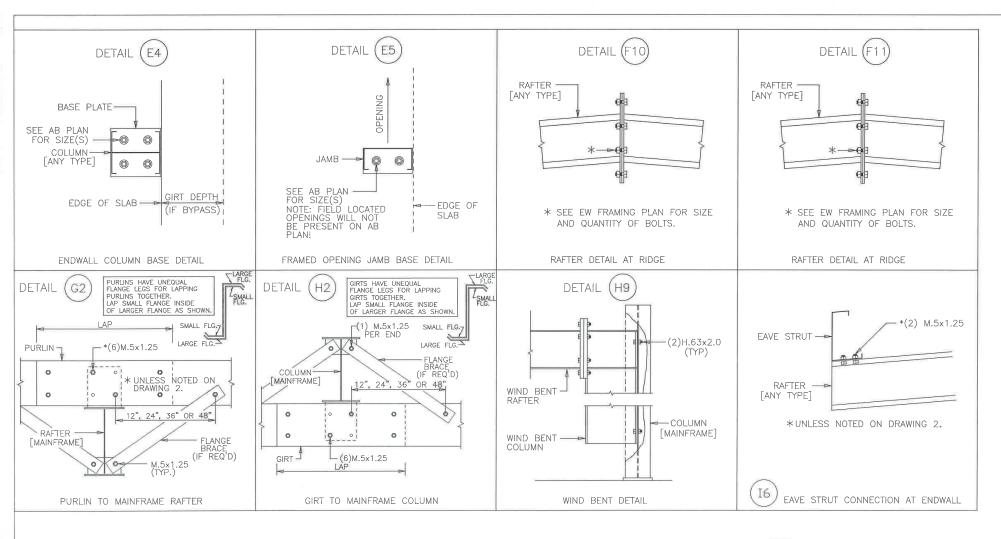




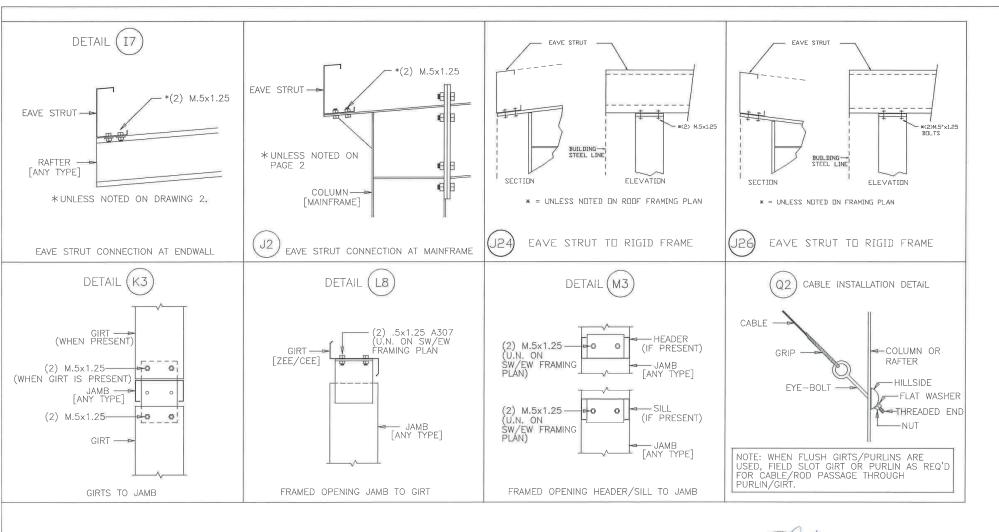




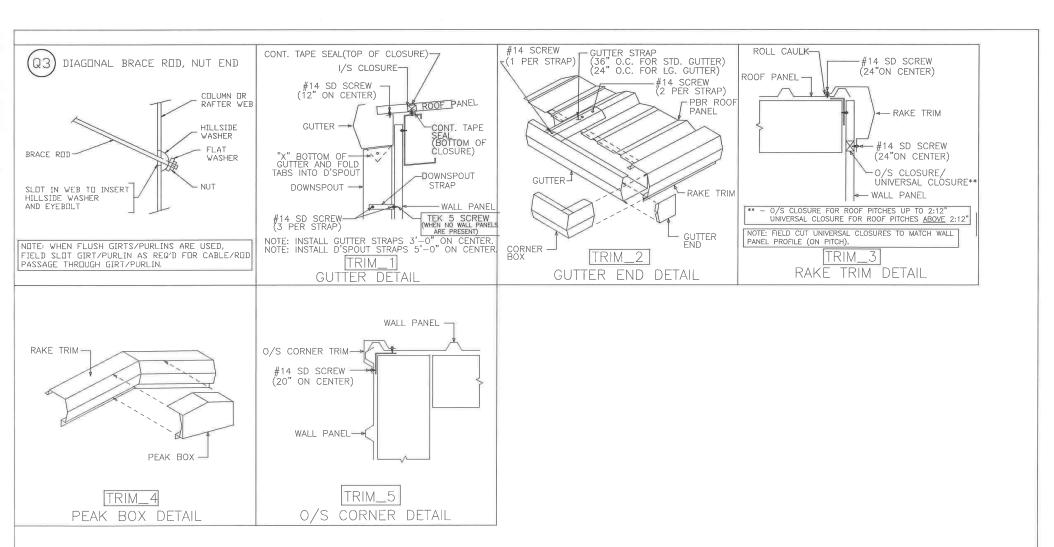




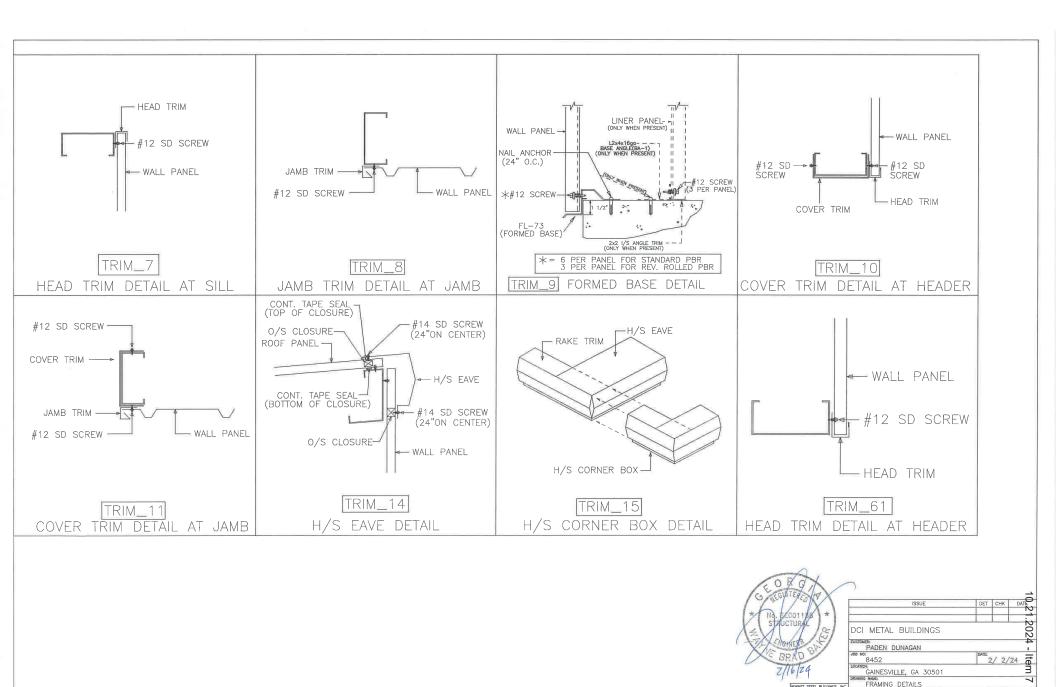




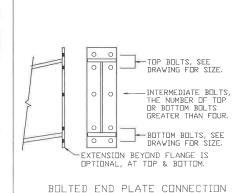








PAGE 5.4



STRUCTURAL BOLTED CONNNECTIONS

REFER TO COVER PAGE "GENERAL NOTES" PARAGRAPH "C", SECTION "9" FOR INSTRUCTIONS ON TIGHTENING ALL A325 AND A490 CONNECTION BOLTS.

TRIM NOTES:

- [1] SEAL TRIM SPLICES WITH TUBE CAULK,
- [2] SECURE GUTTER SPLICES AND END PLUGS WITH RIVETS.
- [3] SECURE ALL OTHER ROOF TRIM SPLICES WITH TRIM SCREWS UNLESS NOTED OTHERWISE.
- [4] TRIM SCREWS ARE LOCATED 24" ON CENTER UNLESS NOTED OTHERWISE.
- [5] STD. TRIM SPLICES ARE 3" TOTAL UNLESS NOTED OTHERWISE.

MORTISE PREPPED PERSONNEL DOORS

ALL MORTISE PREPPED PERSONNEL DOORS COME AS RIGHTHAND REVERSED SWING.

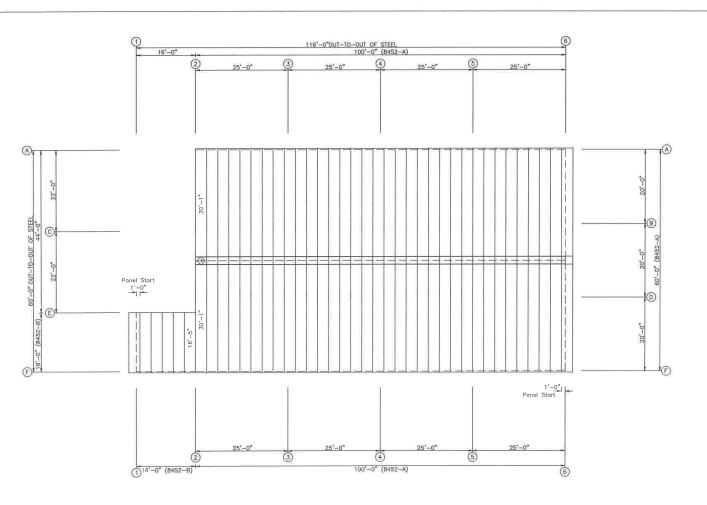
(i.e. STANDING ON THE OUTSIDE OF THE BUILDING FACING THE DOOR, THE LOCK WILL BE ON THE LEFTHAND SIDE OF THE DOOR AND THE DOOR WILL SWING OUTWARD FROM THE BUILDING.)

ANY FIELD MODIFICATIONS ARE THE RE-SPONSIBILITY OF THE ERECTOR AND MBM IS NOT LIABLE FOR LABOR CHARGES NOR DAMAGES DUE TO ERROR.

	BUIL MEMBER	_		D
BEAM	BEAM DEPTH	FLANGE WIDTH	FLANGE THK.	WEB THK.
B	08	5	4	1
B= BUILT-UP	08= 8" 10= 10" 12= 12" 14= 14" ETC.	5,6,8,10 OR 12 (INCHES)	MEASURED IN 16ths. (4= 1/4", 5= 5/16" ETC.)	1= 10ga 3= 3/16" ETC.



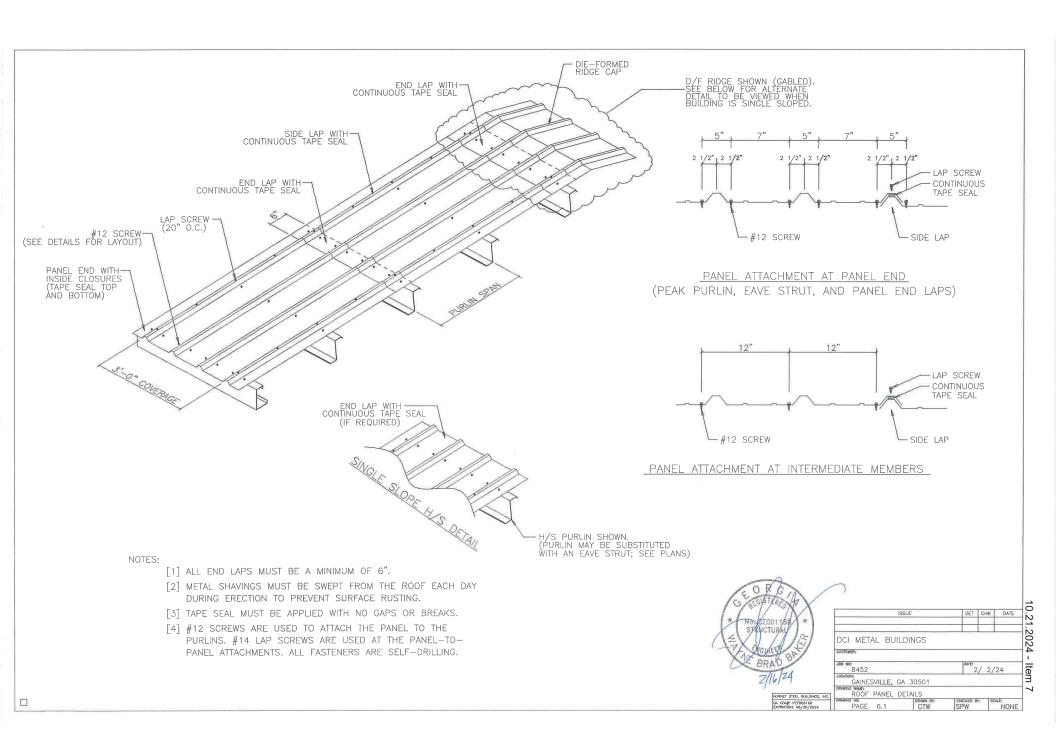
ISS	HE	DET	снк	DATE
133	OE.	DEI	CHK	DATE
DCI METAL BUILD	DINGS		,,,,,,,	.2024
PADEN DUNAGAN	Ĭ.			
8452		DATE:	/ 2/	′24 E
GAINESVILLE, GA	30501			3
FRAMING DETAILS				
PAGE 5.5	CTW	SPW SPW	r: So	NONE

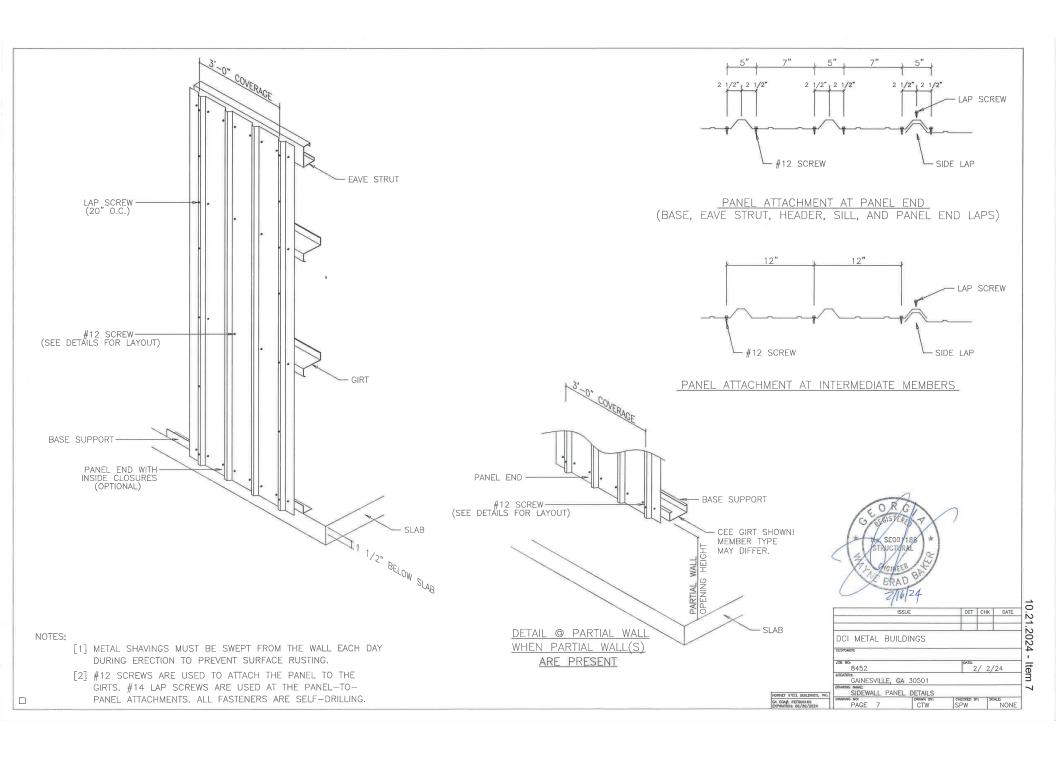


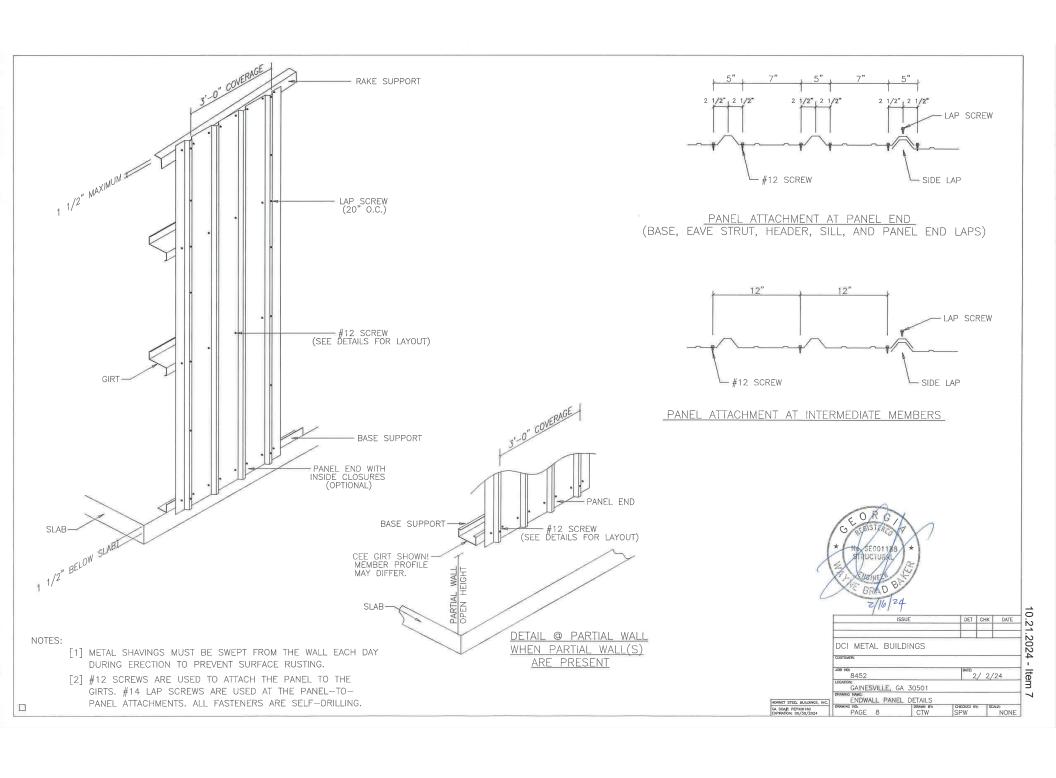
ROOF SHEETING PLAN PANELS: 26 GA. PBR – GALVALUME

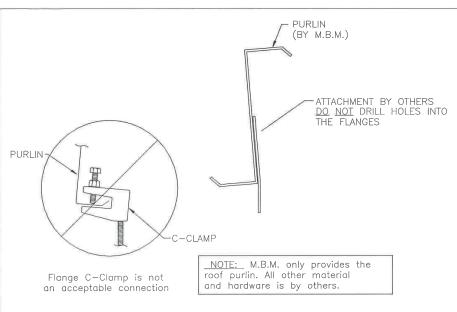


TRIM TABLE
ROOF PLAN
OID PART LENGTH
1 D/F CAP6 3'-0"









Recommended Connection Detail

NOTE

MANY FACTORS BEYOND THE CONTROL OF THE METAL BUILDING SUPPLIER AFFECT THE ABILITY OF A PURLIN TO SAFELY SUPPORT HANGING LOADS COMBINED WITH OTHER REQUIRED ROOF LOADS, DUE TO THE VARIABLES INVOLVED IN HANGING LOADS AND THEIR ATTACHMENTS TO THE PURLINS, THE METAL BUILDING SUPPLIER CANNOT ASSURE THAT THE PURLINS FOR A PARTICULAR BUILDING PROJECT CAN SAFELY SUPPORT THE MAXIMUM ALLOWABLE HANGING LOADS IN COMBINATION WITH OTHER ROOF LOADS.

IT IS THE RESPONSIBILITY OF THE HANGER SYSTEM INSTALLER TO COORDINATE WITH THE ENGINEER OF RECORD FOR THE OVERALL PROJECT TO ENSURE A SAFE HANGING LOAD INSTALLATION. THE METAL BUILDING ENGINEER IS NOT THE ENGINEER OF RECORD FOR THE OVERALL PROJECT. WITHOUT SPECIFIC CERTIFICATION FOR INDIVIDUAL HANGING LOADS, THE NET EFFECTS OF APPLIED HANGER LOADS INSTALLED ON A PARTICULAR PURLIN SHALL NOT EXCEED THE NET EFFECTS OF THE CERTIFIED UNIFORMLY APPLIED DESIGN COLLATERAL LOAD.

HANGING LOADS SHOULD NOT BE APPLIED TO THE PURLIN LIP. WHERE PERMISSIBLE, THE BEST PRACTICE FOR HANGING LOADS IS TO ATTACH TO THE PURLIN WEB USING A BOLT AND NUT, OR SELF—DRILLING SCREWS.

HANGING UNIFORM LOADS SUCH AS SPRINKLER MAINS OR HVAC EQUIPMENT SHOULD BE DISTRIBUTED OVER SEVERAL PURLINS, AND SHOULD NEVER EXCEED THE COLLATERAL LOAD ALLOWANCE FOR THE ROOF SYSTEM. FOR UNIFORM LOADS THAT RUN PARALLEL TO THE PURLINS, IT MAY BE NECESSARY TO USE TRANSVERSE SUPPORT CHANNELS(A.KA. TRAPEZE BEAMS) ATTACHED TO THE WEBS OR FLANGES OF ADJACENT PURLINS TO SPREAD THE LOAD BETWEEN TWO OR MORE PURLINS. IN SUCH CASES, CONTACT THE BUILDING MANUFACTURER OR A LOCAL PROFESSIONAL ENGINEER PRIOR TO ATTEMPTING TO HANG LOADS FROM THE PURLINS

