PRELIMINARY STORMWATER MANAGEMENT REPORT

for

Cagle Mill Residential & Industrial Tracts

4745 Cagle Mill Road Lula, GA 30554



3500 Parkway Lane, Suite 500

Peachtree Corners, Georgia 30092

678-336-7740

www.pondco.com

Pond & Company Project No. 1220366

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PROJECT DESCRIPTION

This Preliminary Hydrology Study has been prepared for Cagle Mill, residential and commercial development in Hall County, Georgia. The property consists of two lots. The lots have been surveyed at 72.67 acres and 68.76 acres and is owned by CBD Investments, LLC. Cornelia Highway (GA State Road #365) abuts the site from the northwest side and Cagle Mill Road from the southeast side. The North Oconee River crosses the site.

Based on FIRM Panel Maps 13139C0225G, dated December 1, 2022, The North Oconee River in our site is within a Special Flood Hazard Area Flood Zone "A" (Appendix A).

This project is intended to develop the site into two different parcels, commercial and residential areas. In the residential area, there will be 214 units of single-family, 97 units of townhomes, 68 cottage houses, a clubhouse, and 2 amenity areas. The commercial space will be adjacent to Cornelia Highway and mostly contain industrial-use buildings with a visitor's parking lot.

The development is anticipated to disturb about 70 percent of the site area. More detailed explanations will be provided in the final report. The site will drain to the North Oconee River on the west side of the property, which is also part of the floodplain.

The new construction will include the installation of a new BMP as follows:

Bioretention Basin: One (1)Stormwater Wet Pond: One (1)

• Enhance Swale: Four (4)

Pond: One (1)

The following report describes the methodology and the associated analysis results to meet the proposed developments above performance criteria.

METHODOLOGY

Hydrograph modeling and pond routing is performed using Hydraflow Hydrographs Extension for AutoCAD Civil 3D 2022. Soil Conservation Service (SCS) hydrograph method is used to generate Type II 24-hour hydrographs for all pre- and post- developed basins during storm events. SCS runoff curve numbers are taken from the Georgia Stormwater Management Manual (GSMM). Curve numbers are selected based on existing and proposed ground cover located on soil group B. In cases where ground cover is not uniform over a basin area, a weighted curve number is calculated based on the area of each type of ground cover. Time of concentration calculations for each basin (pre- and post- developed) are performed in accordance with the procedure found in USDA's Technical Release 55 Urban Hydrology for Small Watersheds (TR-55).

HYDROLOGICAL ANALYSIS

PRE-DEVELOPED CONDITIONS

The existing area of the site is primarily wooded. Onsite basins are modeled using the maximum pre-developed curve number of 55 while offsite basins are modeled using true land cover values in accordance with the Georgia Stormwater Management Manual (GSMM). The soils on site are categorized as Hydrologic Soils Group B.

For the analysis of the pre-developed conditions the site is divided into onsite basins (See Figure 3) leaving the site at four points of analysis:

- Pre-Developed Basin A is 50.20-acres with a maximum allowable CN of 55 and a time of concentration of 22 minutes. The basin is a heavily wooded area with steep slopes. All runoff from this basin is discharged from the project area via the North Oconee River.
- Pre-Developed Basin B is 6.82-acres with a maximum allowable CN of 55 and a time of
 concentration of 25 minutes. The basin is a heavily wooded area with steep slopes. All
 runoff from this basin is discharged from the project area to the northwest of the parcel
 onto a neighboring undeveloped parcel.
- Pre-Developed Basin C is 83.63-acres with a maximum allowable CN of 55 and a time of concentration of 17 minutes. The basin is a heavily wooded area with steep slopes. All runoff from this basin is discharged from the project area via an existing stream that feed into the North Oconee River.
- Pre-Developed Basin D is 0.77-acres with a maximum allowable CN of 55 and a time of concentration of 5 minutes. The basin is a heavily wooded area with steep slopes. All runoff from this basin is discharged from the project area as sheet flow.

Runoff from four offsite basins drain to the points of analyses as follows:

- Pre-Developed Offsite Basin A1 is 7.91-acres with a CN of 61 and a time of concentration of 12 minutes.
- Pre-Developed Offsite Basin A2 is 0.61-acres with a CN of 58 and a time of concentration of 8 minutes.
- Pre-Developed Offsite Basin C1 is 9.13-acres with a CN of 59 and a time of concentration of 6 minutes.
- Pre-Developed Offsite Basin C2 is 36.72-acres with a CN of 57 and a time of concentration of 18 minutes.

While Pre-Developed Basins A, B, C, and D all leave the existing site at different points of analysis, they all converge approximately 556 feet east of the property line downstream along the North Oconee. All combined predeveloped basins can be analyzed as one basin but for the purpose of this preliminary hydrology report, the basins are analyzed as distinct basins to model changes in runoff directly off the subject property.

POST-DEVELOPED CONDITIONS

The proposed site area includes new commercial and residential developments. Onsite and offsite basins are modeled using the true land cover values in accordance with the Georgia Stormwater Management Manual (GSMM). The soils on site are categorized as Hydrologic Soils Group B.

For the analysis of the post-developed conditions the site is divided into onsite and onsite bypass (See Figure 4).

- Post-Developed Basin A to Pond 1 is 42.27-acres with a CN of 77 and a time of concentration of 5 minutes. The basin is comprised of a portion of the proposed residential development with a majority impervious cover. All runoff from this basin is treated by the proposed wet extended detention pond and discharged from a proposed outlet control structure. Runoff within this basin is discharged into the North Oconee.
- Post-Developed Basin A Bypass is 23.91-acres with a CN of 66 and a time of concentration of 5 minutes. The basin is comprised of a proposed industrial development with a majority pervious cover. All runoff from this basin is discharged from the project area into the North Oconee.
- Post-Developed Basin B is 5.70-acres with a CN of 79 and a time of concentration of 10 minutes. The basin is comprised of a portion of the proposed residential development.
 All runoff from this basin is discharged from the project area to the northwest of the parcel onto a neighboring undeveloped parcel. Flows will be managed using a Best Management Practice (BMP) to control runoff and improve water quality.
- Post-Developed Basin C to Pond 2 is 56.47-acres with a CN of 71 and a time of
 concentration of 5 minutes. The basin is comprised of a portion of the proposed
 residential development with a majority impervious cover. All runoff from this basin is
 treated by the proposed dry extended detention pond and discharged from a proposed
 outlet control structure. Runoff within this basin is discharged into the North Oconee.
- Post-Developed Basin C Bypass is 12.53-acres with a CN of 62 and a time of concentration of 5 minutes. The basin is comprised of a portion of the proposed residential development with a majority pervious cover. All runoff from this basin is discharged from the project area into the North Oconee.
- Post-Developed Onsite Basin D is 0.53-acres with a CN of 61 and a time of concentration of 5 minutes. The basin is a grass slope. All runoff from this basin is discharged from the project area as sheet flow into the North Oconee.

OFFSITE CONDITIONS

Runoff from four offsite basins drain to the points of analyses as follows:

- Post-Developed Offsite Basin A to Pond 1 is 1.91-acres with a CN of 59 and a time of concentration of 12 minutes.
- Post-Developed Offsite Basin A1 Bypass is 6.0-acres with a CN of 60 and a time of concentration of 8 minutes.
- Post-Developed Offsite Basin A2 Bypass is 0.62-acres with a CN of 58 and a time of concentration of 8 minutes.
- Post-Developed Offsite Basin C to Pond 2 is 36.72-acres with a CN of 57 and a time of concentration of 18 minutes.
- Post-Developed Offsite Basin C Bypass is 9.13-acres with a CN of 59 and a time of concentration of 6 minutes.

As with the Pre-Developed Basins, although the Post-Developed Basins leave the site at different points of analysis, they all converge approximately 556 feet east of the property line downstream along the North Oconee. All combined predeveloped basins can be analyzed as one basin but for the purpose of this preliminary hydrology report, the basins are analyzed as distinct basins to model changes in runoff directly off the subject property.

BASIN SUMMARY

	Pre-Dev Basin A Flow Summary						
Basin ID	Return Freq.	Pre-Dev A (cfs)	Offsite A1 (cfs)	Offsite A2 (cfs)	Total Pre Dev A (cfs)		
Α	1	6.2	4.0	0.2	8.9		
	2	12.9	6.5	0.4	17.3		
	5	29.1	11.4	0.8	37.0		
	10	46.9	16.1	1.2	58.6		
	25	77.4	23.6	1.8	95.2		
	50	104.4	30.0	2.3	127.5		
	100	134.7	37.1	2.9	163.7		

	Post-Dev Basin A Flow Summary							
Basin ID	Return Freq. (cfs)	Post-Dev A to Pond 1 (cfs)	Offsite A to Pond 1 (cfs)	Pond 1 Routed (cfs)	Post-Dev A Bypass (cfs)	Offsite A1 Byp. (cfs)	Offsite A2 Byp. (cfs)	Total Post-Dev A
Α	1	92.3	0.7	3.6	3.4	3.2	0.2	6.1
	2	115.5	1.3	4.1	5.6	5.2	0.4	8.6
	5	158.1	2.4	4.8	9.9	9.1	0.8	14.1
	10	197.2	3.4	13.6	14.2	13.0	1.2	18.7
	25	255.6	5.2	48.4	20.9	19.1	1.8	61.0
	50	303.4	6.7	96.6	26.7	24.4	2.3	115.8
	100	353.8	8.3	148.6	33.1	30.2	2.9	177.6

Basin B Flow Summary				
Basin ID	Return Freq.	Pre-Dev B (cfs)	Post-Dev B (cfs)	
В	1	0.8	11.5	
	2	1.6	14.4	
	5	3.6	19.6	
	10	5.8	24.3	
	25	9.5	31.3	
	50	12.9	37.0	
	100	16.6	43.0	

	Pre-Dev Basin C Flow Summary						
Basin ID	Return Freq.	Pre-Dev C	Offsite C1	Offsite C2	Total Pre Dev C		
С	1	6.7 cfs	0.1 cfs	30.5 cfs	32.5 cfs		
	2	7.8	0.1	36.6	39.0		
	5	9.8	0.2	47.5	50.5		
	10	11.5	0.2	56.9	60.6		
	25	13.9	0.2	70.8	75.4		
	50	15.9	0.3	82.0	87.4		
	100	17.9	0.3	93.7	99.9		

	Post-Dev Basin C Flow Summary							
Basin ID	Return Freq.	Post-Dev C to Pond 2 (cfs)	Offsite C to Pond 2 (cfs)	Pond 2 Routed (cfs)	Post-Dev C Onsite Bypass (cfs)	Offsite C Byp. (cfs1)	Total Post-Dev C (cfs)	
С	1	89.3	7.6	5.7	9.8	4.9	18.6	
	2	116.9	14.2	10.3	14.6	8.1	27.4	
	5	168.5	29.1	38.7	23.9	14.3	44.1	
	10	215.7	44.7	73.9	32.9	20.4	90.6	
	25	288.2	70.2	143.6	46.9	30.1	180.1	
	50	349.7	92.5	202.0	58.9	38.4	257.7	
	100	415.4	117.4	265.7	71.9	47.5	341.9	

	Basin D Flow Summary				
Basin ID	Return Freq.	Pre-Dev D (cfs)	Post-Dev D (cfs)		
D	1	0.2	0.4		
	2	0.4	0.6		
	5	0.9	1.0		
	10	1.3	1.3		
	25	2.1	1.9		
	50	2.7	2.4		
	100	3.4	2.9		

STORMWATER MANAGEMENT DESIGN

The purpose of this section is to present a method for conformance with the current Clayton County Water Authority's Stormwater Regulations and Georgia's Stormwater Management Manual (GSMM) for the proposed development. The stormwater facilities herein will conform to the current Clayton County Water Authority's regulations as well as Vol 2 of the Georgia Stormwater Management Manual, Latest Edition.

WATER QUALITY / RUNOFF REDUCTION

As of December 2020, the State of Georgia implemented Runoff Reduction practices as a stormwater Best Management Practice used to disconnect impervious and disturbed pervious surfaces from the storm drainage system. The purpose of this is to reduce post-construction

stormwater runoff rates, volumes, and pollutant loads. The Runoff Reduction Volume (RRv) is the retention of the volume calculated to infiltrate, evapotranspirate, harvest and use or otherwise remove runoff from a post-developed condition to mimic the natural hydrological conditions more closely.

The proposed development will utilize a system of bioretention basins, enhanced swales, wet ponds, and dry extended detention basins to achieve the maximum practical extent possible of the required RRv for the project area. As part of the State of Georgia's implementation of the Watershed Protection Plan, developments that do not meet the full RRv requirements are required to capture and treat the first 1.2 inches of rainfall to remove a significant portion of the total pollution load inherent in the stormwater runoff.

To analyze the TSS loading for individual developments, GSMM has provided the Stormwater Quality Site Development Review Tool (See Appendix D).

CHANNEL PROTECTION

The Channel Protection criteria are typically achieved by providing 24 hours of extended detention for runoff generated by the 1-year, 24-hour rainfall event. The overbank flood protection criteria generally are achieved by attenuating the post-development flows not to exceed the predevelopment flows for the 1, 2, 5, 10, and 25-year storm events. The extreme flood protection criteria are typically achieved by safely controlling and conveying the 100-year storm event.

OVERBANK FLOOD PROTECTION

As is shown in the basin summary of this report, the proposed project attenuates peak runoff flows in all return frequency storm events. In addition to attenuation, the project area discharges through a proposed storm network directly into the floodplain of the North Oconee.

EXTREME FLOOD PROTECTION

It is recommended that these criteria are not required for this project due to the nature of the development and the site's proximity to the 100-year floodplain. While this site is anticipated to contribute pollutants via stormwater runoff to be treated by the water quality measures listed above, the expected increase in peak runoff flows will not be significant, and flooding will not be exacerbated. See below for a summary of peak runoff rates for all storm events and Appendix D for Hydraflow Hydrograph model results.

10 % BASIN

As described in the Georgia Stormwater Management Manual, the point at which the site accounts for 10% of the total drainage basin (10% point) typically reveals the most severe issues regarding the timing of hydrographs.

Since the site area (141.43 ac) is less than 10% of the total drainage area (3,251.52 ac), a downstream analysis will be located at POA 10%.

10% Basin Flow Summary					
Basin ID	Return Freq.	10% Pre (cfs)	10% Post (cfs)		
Α	1	8.9	6.1		
	2	17.3	8.6		
	5	37.0	14.1		
	10	58.6	18.7		
	25	95.2	61.0		
	50	127.5	115.8		
	100	169.7	177.6		

CONCLUSION

This hydrology study report demonstrates that the stormwater best management practices designed herein will satisfy stormwater management and storm drainage requirements of Clayton County for the project based on the following conclusions:

- The stormwater management for this project achieves the maximum practical extent possible of the required RRv. To comply with stormwater regulations, the first 1.2 inches of runoff is treated and meets the TSS requirements. Stormwater treatment is proposed via water quality measures listed with the North Georgia Post-Construction Stormwater Technology Assessment Protocol to ensure that it meets the 80% TSS requirement where runoff reduction is not viable.
- 2. Peak flows are anticipated to increase due to this project, but not significantly. In addition, the site is adjacent to the Creek floodplain area. Due to these conditions, it is recommended that Channel Protection, Overbank Flood Protection, and Extreme Flood Protection criteria be waived for this project.

The stormwater management facilities will continue to meet the regulatory requirements discussed herein only if the facilities are properly maintained on a regular basis. Any sediment or trash accumulation in the facilities should be removed immediately.

This report and associated construction plans pertain to those topics expressly stated herein. No evaluation has been made by the designer with regards to any other topic including but not limited to: structural or geotechnical evaluations, environmental evaluations, etc.

FIGURE 3: PRE-DEVELOPED BASINS

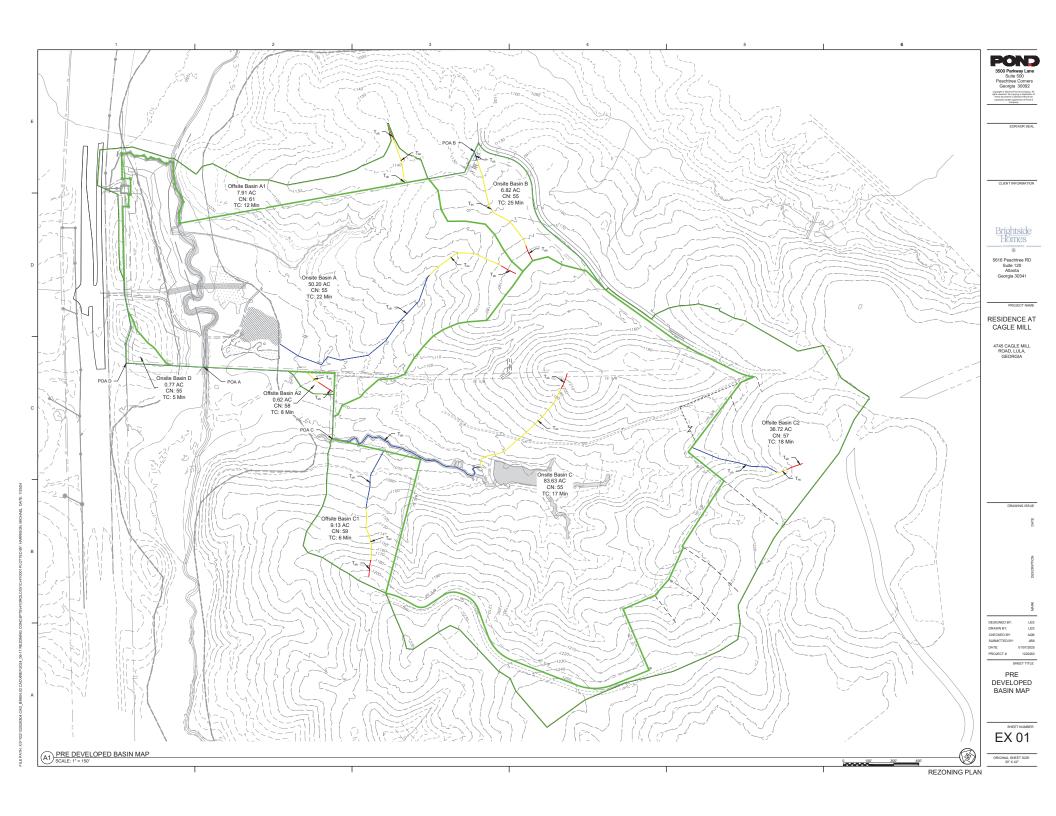
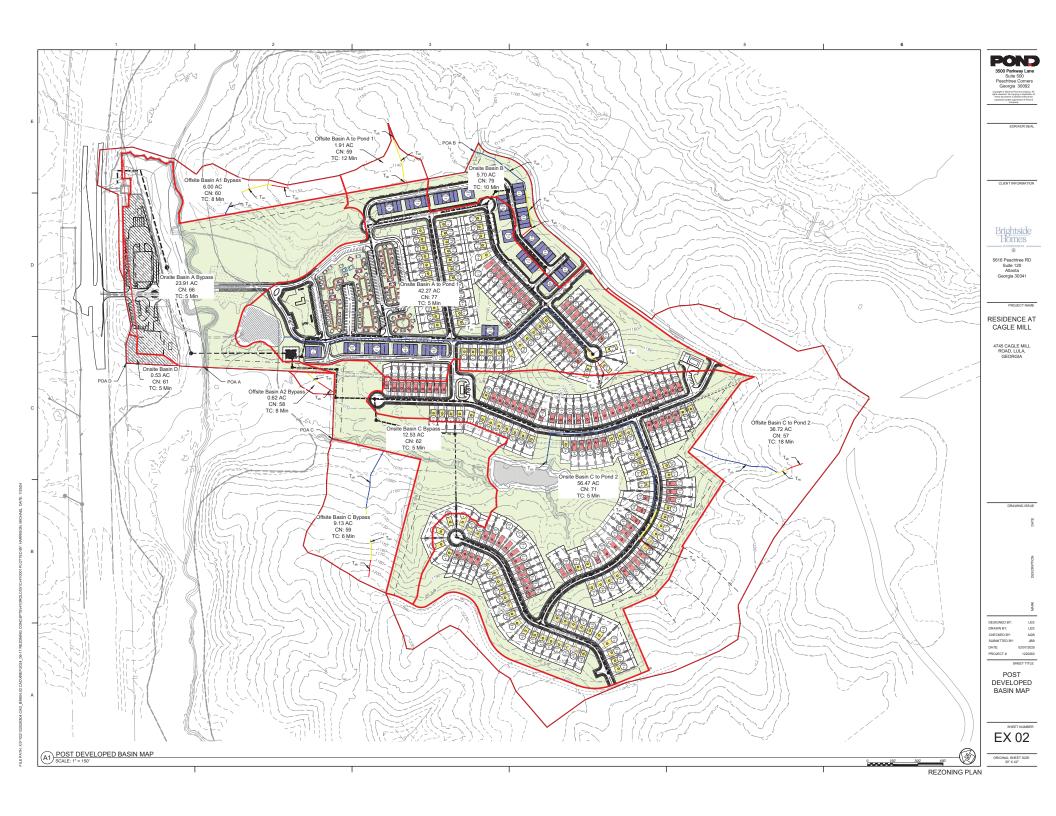


FIGURE 4: POST-DEVELOPED BASINS



APPENDIX A

2/28/24, 10:15 AM StreamStats

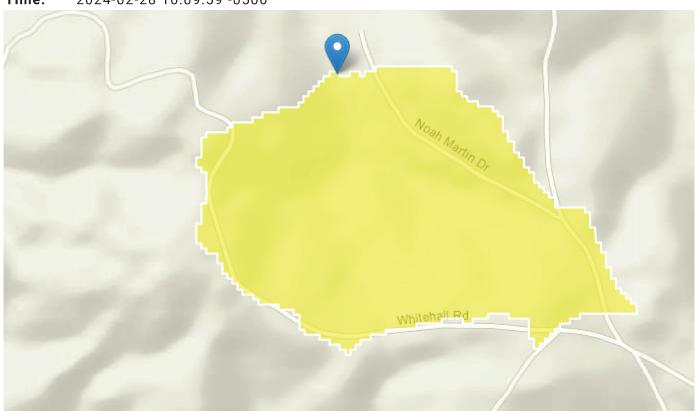
Cagle Mill Phase 2- StreamStats Report

Region ID: GA

Workspace ID: GA20240228150937256000

Clicked Point (Latitude, Longitude): 34.36732, -83.72267

Time: 2024-02-28 10:09:59 -0500



Collapse All ■

➤ Basin Characteristics

Parameter Code	Parameter Description	Value	Unit
BSLDEM10M	Mean basin slope computed from 10 m DEM	15.896	percent
CSL10_85	Change in elevation divided by length between points 10 and 85 percent of distance along main channel to basin divide - main channel method not known	315	feet per mi
DRNAREA	Area that drains to a point on a stream	0.12	square miles
ELEV	Mean Basin Elevation	1180	feet

2/28/24, 10:15 AM StreamStats

Parameter Code	Parameter Description	Value	Unit
ELEVMAX	Maximum basin elevation	1261	feet
GWHEAD	Mean basin elevation minus minimum basin elevation	106	feet
I24H100Y	Maximum 24-hour precipitation that occurs on average once in 100 years	7.76	inches
I24H10Y	Maximum 24-hour precipitation that occurs on average once in 10 years	5.38	inches
I24H25Y	Maximum 24-hour precipitation that occurs on average once in 25 years	6.3	inches
I24H50Y	Maximum 24-hour precipitation that occurs on average once in 50 years	7.02	inches
LC06AGRI	Percent agriculture computed as total of grass, pasture, and crops, NLCD classes 71, 81 and 82	1.153	percent
LC06DEV	Percentage of land-use from NLCD 2006 classes 21-24	12.536	percent
LC06FOREST	Percentage of forest from NLCD 2006 classes 41-43	85.253	percent
LC06IMP	Percentage of impervious area determined from NLCD 2006 impervious dataset	1.12	percent
LC11DEV	Percentage of developed (urban) land from NLCD 2011 classes 21-24	13.1	percent
LC11IMP	Average percentage of impervious area determined from NLCD 2011 impervious dataset	1.49	percent
MINBELEV	Minimum basin elevation	1074	feet
PCTREG1	Percentage of drainage area located in Region 1 - Piedmont / Ridge and Valley	100	percent
PCTREG2	Percentage of drainage area located in Region 2 - Blue Ridge	0	percent
PCTREG3	Percentage of drainage area located in Region 3 - Sandhills	0	percent
PCTREG4	Percentage of drainage area located in Region 4 - Coastal Plains	0	percent
PCTREG5	Percentage of drainage area located in Region 5 - Lower Tifton Uplands	0	percent

2/28/24, 10:15 AM StreamStats

Parameter Code	Parameter Description	Value	Unit
PRECPRIS00	Basin average mean annual precipitation for 1971 to 2000 from PRISM	57.2	inches
RELIEF	Maximum - minimum elevation	187	feet
RRMEAN	Relief ratio defined as (ELEV- MINBELEV)/(ELEVMAX-MINBELEV)	0.569	dimensionless

USGS Data Disclaimer: Unless otherwise stated, all data, metadata and related materials are considered to satisfy the quality standards relative to the purpose for which the data were collected. Although these data and associated metadata have been reviewed for accuracy and completeness and approved for release by the U.S. Geological Survey (USGS), no warranty expressed or implied is made regarding the display or utility of the data for other purposes, nor on all computer systems, nor shall the act of distribution constitute any such warranty.

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Application Version: 4.19.4

StreamStats Services Version: 1.2.22

NSS Services Version: 2.2.1

APPENDIX B

ALTAINSPS SURVEY FOR: CHICAGO TITLE INSURANCE COMPANY & ACM VENTURES, LLC



DAVISENGINEERS COM

277

ACM VENTURES,

AND

71.TLE

E COMPANY AND LULA DISTRICT*
1690th G.M.D.*

ALTA/NSPS LAND TITLE SURVEY FOR

THIS BLOCK RESERVED FOR THE CLERI OF THE SUPERIOR COURT



-ZONING INFORMATION-

~ALTA NOTES~

NO STATE WATERS OR WETLANDS WERE ORSERVED ON SITE AT TIME OF SLIEVEY SITE ADDRESS: 8324 MAIN STREET

~AS-SUR VEYED~ LEGAL DESCRIPTION

All that tract or parcel of land lying and being in the Lula* District, 1690th G.M.D.*, Hall County, Georgia and being more particularly described as follows:

right-of-way of Cornello Sighways and 1/2" above found being the PORT OF ECONNING.

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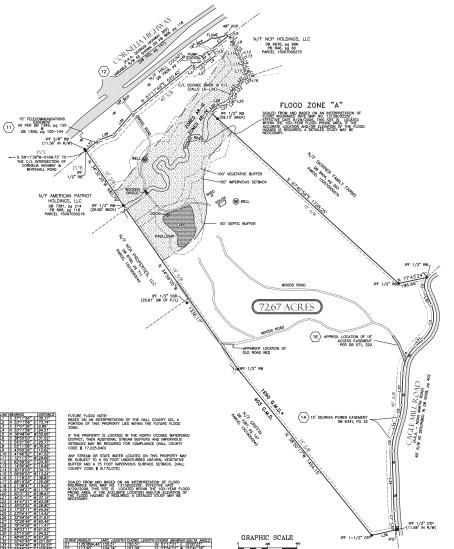
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Said property containing 72.67 acres



(IN FEET) 1 inch = 200 ft.

~SURVEY NOTES~

FIELD MEASUREMENTS WERE TAKEN WITH A

FIELD DATA ADJUSTED USING THE LEAST SQUARES METHOD. THIS PLAT HAS BEEN CALCULATED FOR CLOSURE AND FOUND TO HAVE A PRECISION OF 1 FOOT IN 554,049 FEET.

THIS PLAT DOES NOT CONSTITUTE AND SHALL NOT BE CONSTRUED AS A TITLE SEARCH OR REPORT, AND THUS MAY BE SUBJECT TO ANY EASEMENTS, RIGHT OF WAY OR RESTRICTIONS ONE MAY FIND.

TYPE OF SURVEY: ALTA/NSPS SOURCE OF TITLE DESCRIPTION FOR PROPERTY SHOWN HEREON: DB 8919, pg 632

PROPERTY OWNERS AS OF SURVEY DATE: CBD INVESTMENTS, LLC PARCEL NUMBER: 15007 000254 SITE ADDRESS: 4745 CAGLE MILL ROAD LULA, GEORGIA

~TITLE EXCEPTIONS~

- (17)

(NOI A SUMEY MAILEY)

All matters shown on pict of survey for Roy W. Sweet dolt August 26, 2021, or proposed by Done Engineering & Surveying, Rot reveals the following: 100-year flood zone energy. It is located within the 180 A portion of the property is located within a 25 (C). A portion of the property is located within a 50' yeaptfolive buffer.

(1) A portion of the property is located within a 75' (2) A portion of the property is located within a 75' (2) A portion of the property is located within a 75' (3) Settlocide.

*DEEDS AND PLATS. RECOVERED DURING THE COURSE OF THIS SHRWEY DESCRIBE THE SUBJECT PROPERTY TO LEE WHITH MILLTHEE CORGRAM MILLTH DISTRICTS, LISTED BLOW ARE DOCUMENTS FOUND AND THE CORRESPONDING GAILD. THE PROPERTY IS GITED TO LIE WITHIN. THIS LIST SHOULD NOT BE CONSIDERED CONCLUSIVE, NOR DOES IT CONSTITUTE. A TIAL ESAROH:

DB 3714, PGS 73-75 ~ GLADE DISTRICT, GM.D. 403 DB 3714, PGS 70-72 ~ GLADE DISTRICT, GM.D. 403 DB 1725, PGS 82444 GLADE DISTRICT, GM.D. 403 DB 702, PGS 82444 GLADE DISTRICT, GM.D. 1690 DB 702, PG 147 JULIA DISTRICT, GM.D. 1690 DB 702, PG 147 JULIA DISTRICT, GM.D. 109 DB 702, PG 147 JULIA DISTRICT, GM.D. 403 PB 103, PG 78 ~ GM.N. 1850 PB 872, PG 275 ~ LULA DISTRICT GM.D. 1690

CD.D.T. MICHT-CP-WAY PLAYS FOR PROJECT F-013-1(20) AND SUPPLEMENTAL MICHT-CP-WAY DEEDS SHOW THE PROPERTY OLE WITHIN GAMD, 1900, ADDINONALY, U.S. CORNESS DATA FOR HALL COUNTY FROM 1940 MAKES REFERENCE ONLY TO A DISTRICT 1690, LULA, AND DISTRICT 403, GLADE, WHEREAS THERE IS NO MENTION OF A 1680 DISTRICT.

TO: CHICAGO TITLE INSURANCE COMPANY AND ACM VENTURES, LLC:

THIS IS TO CETTEY THAT THIS MAP OR PLAT AND THE SURVEY ON WHICH IT IS BASED WERE MADE IN ACCORDANCE WITH THE 2021 MINIMUM STANDARD DETAIL REQUIREMENTS FOR ALTA/NOSPS LAND TITLE SURVEYS, JOHNTLY ESTABLISHED AND ADOPTED BY ALTA AND NSPS, AND INCLUDES ITEMS 1, 2, 3, 4, 8, & 13 OF TABLE A THEREOR.

FIFLD WORK WAS COMPLETED ON 8/27/2021. DATE OF PLAT OR MAP: 16th DAY of MAY, 2022

SUR VEYORS CERTIFICATE

DUSTY L. LOWMAN PLS# 3216

(-SURVEYOR CERTIFICATION-



REVISIONS BY: FA REVISION DATE: 05/16/22 REVISIONS BY: DS/MS REVISION DATE: 1/18/2022

DRAWN BY: DS FIELD CREW: AM PLAT DATE: 8/26/2021 FIELD DATE: 7/30/2021

SHEET NO.



NOTES TO USERS

This map is for use in administering the National Food Insurance Prog to less not innecessity family large mass subject fooding, particularly local drainage source's of small size. The community map reposit which be consulted for possible updated or additional flood ha information.

SFECIAL FLOOD HAZARD AREAS (SFHAS) SUBJ

LEGEND

Certain areas not in Special Flood Hazard Areas may be protein external statement. Refer to Section 2.4 *Flood Protection I the Flood Insurance Study report for information on flood com-in this jurisdiction.

COASTAL BARRIER RESOURCES SYSTEM (CBRS)

COASTAL BARRIER RESOURCES SYSTEM (CBRS)

02'08', 93" 0!' 12

(EL 987)

O'HER AREAS

FLOODWAY AREAS IN ZONE AE

bench marks shown on this map, please contact the information about bench marks shown on this map, please contact the Information clickes Branch of the National Geodetic Survey at (301) 713-3242, or visit workeling at ways now now now one.

Base map information shown on this FIRM was derived from Digital Orthophotos produced by Hall County, Georgia and the Center for GIS, Georgia Institute of Technology at a scale of 1.12,000 and dated 2005 or later The profile baselines depicted on this map represent the hydraulic materials that make the third baselines the transmits Study reportered in proceed to programme the properties of the proceed to programme the processine, in a some case from the channel contentine or appear outside the Flood Hazard Area.

Please refer to the separately printed Map Index for an overviously strong the system of map peness, community map reposit and all-sting off Communities table containing National Flood Index for each community as well as a listing of the penels community is located.

Contact the FEMA Map Service Center at 1-800-355-9616 for on mailback products associated with The FEMA, Availables pro include previously issued Letters of Mac Changa, a Food Insur-report, and/or origin vessions of this map. The EEHA Map Services may also be needed by Fax at 1-800-356-950 and their www.letta.gov/miss.

of you have questions about this map or questions concerning the National Flood Insurance Program in general, please call 1-877-FEMA MAP (1-877) or visit the FEMA website at www.fema.cov.

HALL COUNTY,
GEORGIA
AND INCORPORATED AREAS

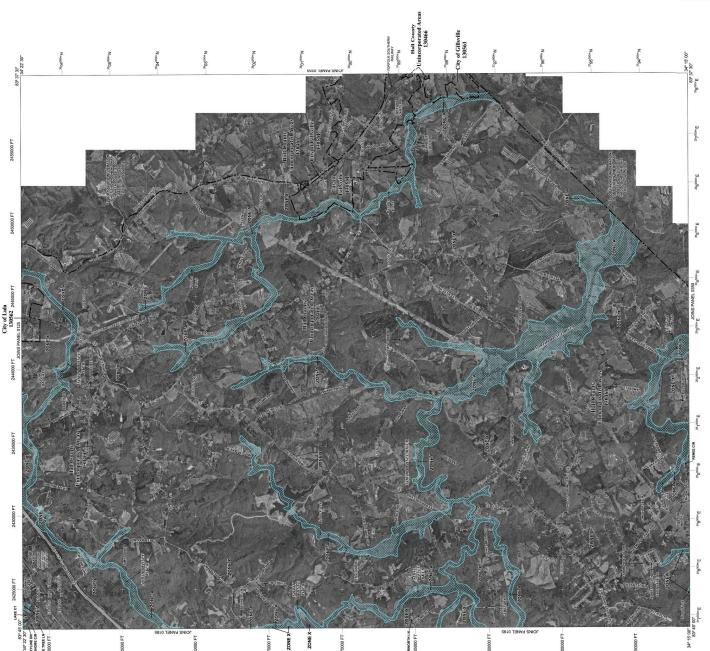
PANEL 225 OF 377

MARIONA EDUANISMI GOODE DANOITAN

FIRM FLOOD INSURANCE RATE MAP

PANEL 0225F

MAP NUMBER 13139C0225F



APPENDIX C



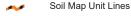
MAP LEGEND

Area of Interest (AOI)

Area of Interest (AOI)

Soils

Soil Map Unit Polygons



Soil Map Unit Points

Special Point Features

Blowout

■ Borrow Pit

Clay Spot

Closed Depression

Gravel Pit

... Gravelly Spot

Landfill

Lava Flow

Marsh or swamp

Mine or Quarry

Miscellaneous Water

Perennial Water

Rock Outcrop

+ Saline Spot

Sandy Spot

Severely Eroded Spot

Sinkhole

Slide or Slip

Sodic Spot

GLND

Spoil Area

Stony Spot

Yery Stony Spot

Wet Spot

△ Other

Special Line Features

Water Features

Streams and Canals

Transportation

+++ Rails

Interstate Highways

US Routes

Major Roads

Local Roads

Background

~

Aerial Photography

MAP INFORMATION

The soil surveys that comprise your AOI were mapped at 1:20.000.

Please rely on the bar scale on each map sheet for map measurements.

Source of Map: Natural Resources Conservation Service Web Soil Survey URL:

Coordinate System: Web Mercator (EPSG:3857)

Maps from the Web Soil Survey are based on the Web Mercator projection, which preserves direction and shape but distorts distance and area. A projection that preserves area, such as the Albers equal-area conic projection, should be used if more accurate calculations of distance or area are required.

This product is generated from the USDA-NRCS certified data as of the version date(s) listed below.

Soil Survey Area: Barrow, Hall, and Jackson Counties, Georgia Survey Area Data: Version 18, Aug 30, 2023

Soil map units are labeled (as space allows) for map scales 1:50,000 or larger.

Date(s) aerial images were photographed: Mar 19, 2022—Apr 20, 2022

The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident.

Map Unit Legend

Map Unit Symbol	Map Unit Name	Acres in AOI	Percent of AOI
Сс	Cartecay and Chewacla soils	18.0	2.5%
MdD	Madison sandy loam, 10 to 15 percent slopes	9.6	1.3%
MdE	Madison sandy loam, 15 to 25 percent slopes	232.6	32.5%
MIC2	Madison sandy clay loam, 6 to 10 percent slopes, moderately eroded	19.6	2.7%
MID2	Madison sandy clay loam, 10 to 15 percent slopes, moderately eroded	225.0	31.5%
MuF	Musella cobbly clay loam, 15 to 35 percent slopes	54.2	7.6%
PaE	Pacolet sandy loam, 15 to 25 percent slopes	25.7	3.6%
PTF	Pacolet-Tallapoosa association, steep	66.0	9.2%
То	Toccoa soils	63.6	8.9%
W	Water	0.8	0.1%
Totals for Area of Interest		715.0	100.0%

Map Unit Descriptions

The map units delineated on the detailed soil maps in a soil survey represent the soils or miscellaneous areas in the survey area. The map unit descriptions, along with the maps, can be used to determine the composition and properties of a unit.

A map unit delineation on a soil map represents an area dominated by one or more major kinds of soil or miscellaneous areas. A map unit is identified and named according to the taxonomic classification of the dominant soils. Within a taxonomic class there are precisely defined limits for the properties of the soils. On the landscape, however, the soils are natural phenomena, and they have the characteristic variability of all natural phenomena. Thus, the range of some observed properties may extend beyond the limits defined for a taxonomic class. Areas of soils of a single taxonomic class rarely, if ever, can be mapped without including areas of other taxonomic classes. Consequently, every map unit is made up of the soils or miscellaneous areas for which it is named and some minor components that belong to taxonomic classes other than those of the major soils.

Most minor soils have properties similar to those of the dominant soil or soils in the map unit, and thus they do not affect use and management. These are called noncontrasting, or similar, components. They may or may not be mentioned in a particular map unit description. Other minor components, however, have properties and behavioral characteristics divergent enough to affect use or to require different

Custom Soil Resource Report

management. These are called contrasting, or dissimilar, components. They generally are in small areas and could not be mapped separately because of the scale used. Some small areas of strongly contrasting soils or miscellaneous areas are identified by a special symbol on the maps. If included in the database for a given area, the contrasting minor components are identified in the map unit descriptions along with some characteristics of each. A few areas of minor components may not have been observed, and consequently they are not mentioned in the descriptions, especially where the pattern was so complex that it was impractical to make enough observations to identify all the soils and miscellaneous areas on the landscape.

The presence of minor components in a map unit in no way diminishes the usefulness or accuracy of the data. The objective of mapping is not to delineate pure taxonomic classes but rather to separate the landscape into landforms or landform segments that have similar use and management requirements. The delineation of such segments on the map provides sufficient information for the development of resource plans. If intensive use of small areas is planned, however, onsite investigation is needed to define and locate the soils and miscellaneous areas

An identifying symbol precedes the map unit name in the map unit descriptions. Each description includes general facts about the unit and gives important soil properties and qualities.

Soils that have profiles that are almost alike make up a *soil series*. Except for differences in texture of the surface layer, all the soils of a series have major horizons that are similar in composition, thickness, and arrangement.

Soils of one series can differ in texture of the surface layer, slope, stoniness, salinity, degree of erosion, and other characteristics that affect their use. On the basis of such differences, a soil series is divided into *soil phases*. Most of the areas shown on the detailed soil maps are phases of soil series. The name of a soil phase commonly indicates a feature that affects use or management. For example, Alpha silt loam, 0 to 2 percent slopes, is a phase of the Alpha series.

Some map units are made up of two or more major soils or miscellaneous areas. These map units are complexes, associations, or undifferentiated groups.

A *complex* consists of two or more soils or miscellaneous areas in such an intricate pattern or in such small areas that they cannot be shown separately on the maps. The pattern and proportion of the soils or miscellaneous areas are somewhat similar in all areas. Alpha-Beta complex, 0 to 6 percent slopes, is an example.

An association is made up of two or more geographically associated soils or miscellaneous areas that are shown as one unit on the maps. Because of present or anticipated uses of the map units in the survey area, it was not considered practical or necessary to map the soils or miscellaneous areas separately. The pattern and relative proportion of the soils or miscellaneous areas are somewhat similar. Alpha-Beta association, 0 to 2 percent slopes, is an example.

An *undifferentiated group* is made up of two or more soils or miscellaneous areas that could be mapped individually but are mapped as one unit because similar interpretations can be made for use and management. The pattern and proportion of the soils or miscellaneous areas in a mapped area are not uniform. An area can be made up of only one of the major soils or miscellaneous areas, or it can be made up of all of them. Alpha and Beta soils, 0 to 2 percent slopes, is an example.

Some surveys include *miscellaneous areas*. Such areas have little or no soil material and support little or no vegetation. Rock outcrop is an example.

Appendix D

Hydrograph Summary Report

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

Hyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description
1	SCS Runoff	6.172	2	732	52,053				Pre Dev Onsite Basin A
2	SCS Runoff	4.047	2	722	14,370				Pre Dev Offite Basin A1
3	SCS Runoff	0.242	2	722	848				Pre Dev Offsite Basin A2
5	Combine	8.904	2	728	67,271	1, 2, 3,			Pre Dev Basin A Total
7	SCS Runoff	92.31	2	718	184,668				Post Dev Onsite Basin A to Pond 1
8	SCS Runoff	0.740	2	724	2,944				Post Dev Offsite Basin A to Pond 1
9	SCS Runoff	3.174	2	720	9,757				Post Dev Offsite Basin A1 Bypass
10	SCS Runoff	0.242	2	722	848				Post Dev Offsite Basin A2 Bypass
12	Combine	92.77	2	718	187,611	7, 8,			Post Dev Basin A to Pond 1
13	Reservoir	3.601	2	832	171,640	12	1056.29	101,286	Post Dev Pond 1 Routed
15	Combine	6.085	2	722	182,245	9, 10, 13,			Post Dev Basin A Total
17	SCS Runoff	0.639	2	734	5,716				Pre Dev Onsite Basin B
18	SCS Runoff	11.52	2	722	30,134				Post Dev Onsite Basin B
20	SCS Runoff	11.31	2	730	85,196				Pre Dev Onsite Basin C
21	SCS Runoff	5.649	2	718	13,919				Pre Dev Offsite Basin C1
22	SCS Runoff	7.613	2	728	45,786				Pre Dev Offsite Basin C2
24	Combine	20.00	2	728	144,901	20, 21, 22,			Pre Dev Basin C Total
26	SCS Runoff	89.33	2	718	180,416				Post Dev Onsite Basin C to Pond 2
27	SCS Runoff	7.613	2	728	45,786				Post Dev Offsite Basin C to Pond 2
28	SCS Runoff	9.782	2	718	22,362				Post Dev Onsite Basin C Bypass
29	SCS Runoff	4.930	2	718	12,792				Post Dev Offsite Basin C Bypass
31	Combine	91.72	2	718	226,203	26, 27,			Post Dev Basin C to Pond 2
32	Reservoir	5.690	2	818	226,184	31	1106.11	93,413	Post Dev Pond 2 Routed
34	Combine	18.69	2	718	261,338	28, 29, 32,			Post Dev Basin C Total
36	SCS Runoff	0.204	2	720	735				Pre Dev Onsite Basin D
37	SCS Runoff	0.370	2	718	875				Post Dev Onsite Basin D
122	20263 Hydrolo	gy Conce	ept.gpw		Return P	eriod: 1 Ye	ear	Friday, 01 /	31 / 2025

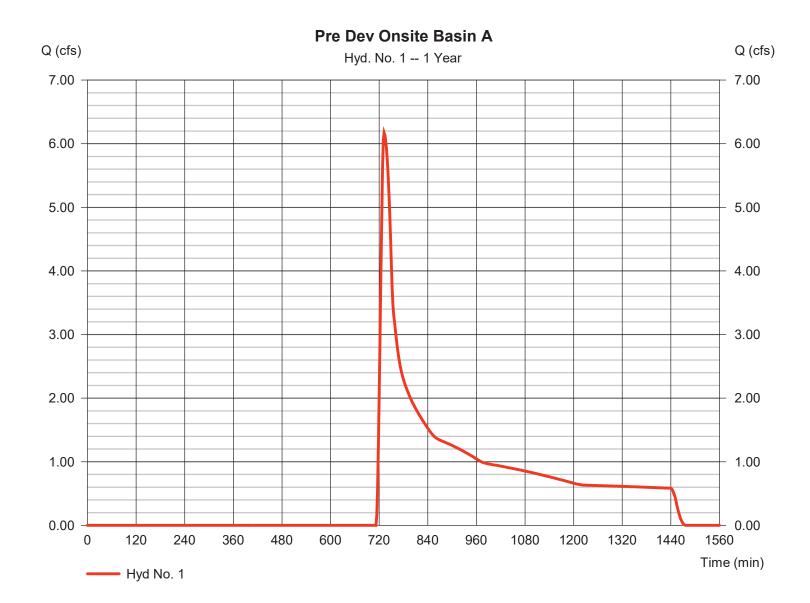
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

Friday, 01 / 31 / 2025

Hyd. No. 1

Pre Dev Onsite Basin A

Hydrograph type = SCS Runoff Peak discharge = 6.172 cfsStorm frequency = 1 yrsTime to peak = 732 min Time interval = 2 min Hyd. volume = 52,053 cuftDrainage area Curve number = 50.200 ac= 55 Hydraulic length Basin Slope = 0.0 %= 0 ftTc method Time of conc. (Tc) = 22.00 min = User Total precip. = 3.30 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



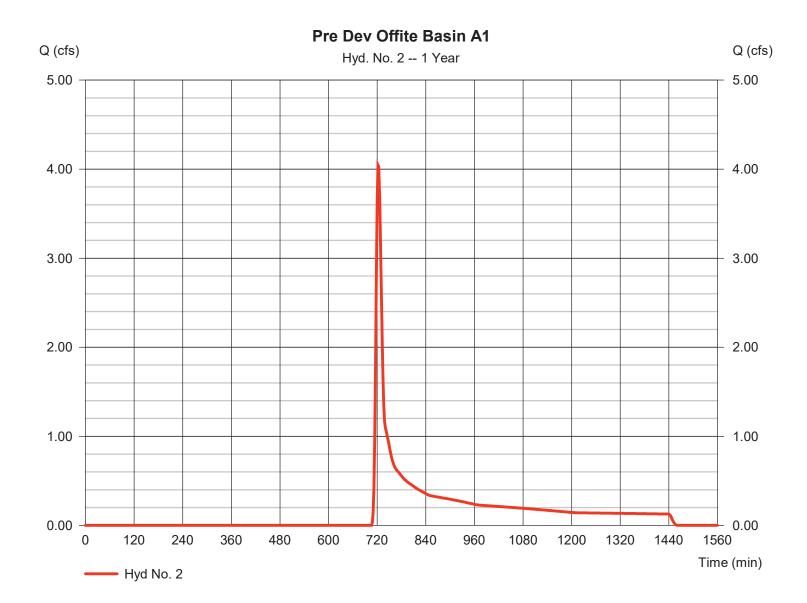
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

Friday, 01 / 31 / 2025

Hyd. No. 2

Pre Dev Offite Basin A1

Hydrograph type = SCS Runoff Peak discharge = 4.047 cfsStorm frequency Time to peak = 722 min = 1 yrsTime interval = 2 min Hyd. volume = 14,370 cuftDrainage area Curve number = 61 = 7.910 acBasin Slope = 0.0 %Hydraulic length = 0 ftTc method Time of conc. (Tc) = 12.00 min = User Total precip. = 3.30 inDistribution = Type II Shape factor Storm duration = 24 hrs = 484



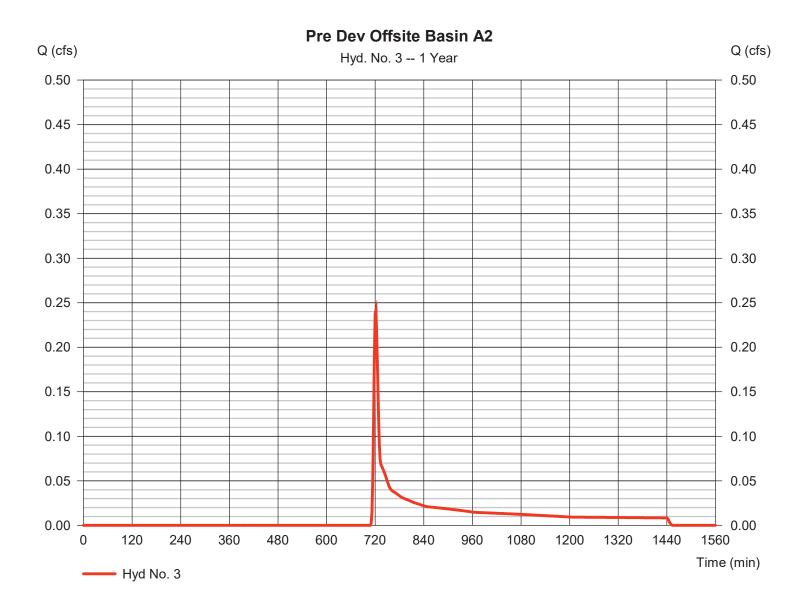
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

Friday, 01 / 31 / 2025

Hyd. No. 3

Pre Dev Offsite Basin A2

Hydrograph type = SCS Runoff Peak discharge = 0.242 cfsStorm frequency Time to peak = 722 min = 1 yrsTime interval = 2 min Hyd. volume = 848 cuft Drainage area = 0.620 acCurve number = 58 Hydraulic length Basin Slope = 0.0 %= 0 ftTc method Time of conc. (Tc) = 8.00 min = User Total precip. = 3.30 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



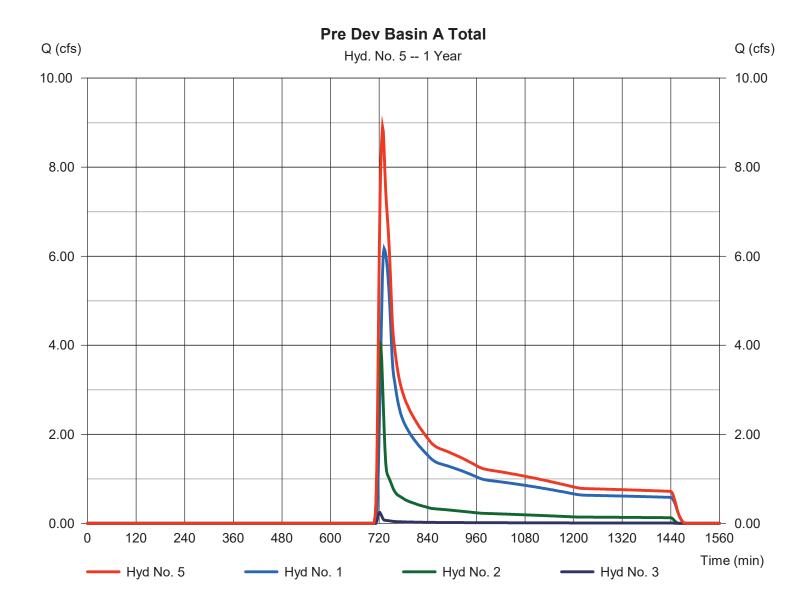
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

Friday, 01 / 31 / 2025

Hyd. No. 5

Pre Dev Basin A Total

Hydrograph type = Combine Peak discharge = 8.904 cfsTime to peak Storm frequency = 1 yrs= 728 min Time interval = 2 min Hyd. volume = 67,271 cuft Inflow hyds. = 1, 2, 3Contrib. drain. area = 58.730 ac



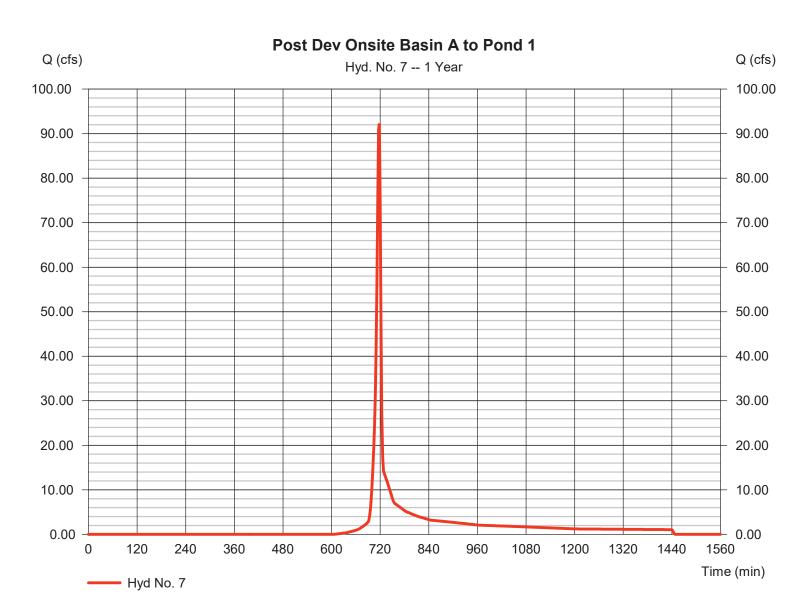
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

Friday, 01 / 31 / 2025

Hyd. No. 7

Post Dev Onsite Basin A to Pond 1

Hydrograph type = SCS Runoff Peak discharge = 92.31 cfsStorm frequency = 1 yrsTime to peak = 718 min Time interval = 2 min Hyd. volume = 184,668 cuft Drainage area = 42.270 ac Curve number = 77 Hydraulic length = 0 ftBasin Slope = 0.0 %Tc method Time of conc. (Tc) $= 5.00 \, \text{min}$ = User Total precip. = 3.30 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



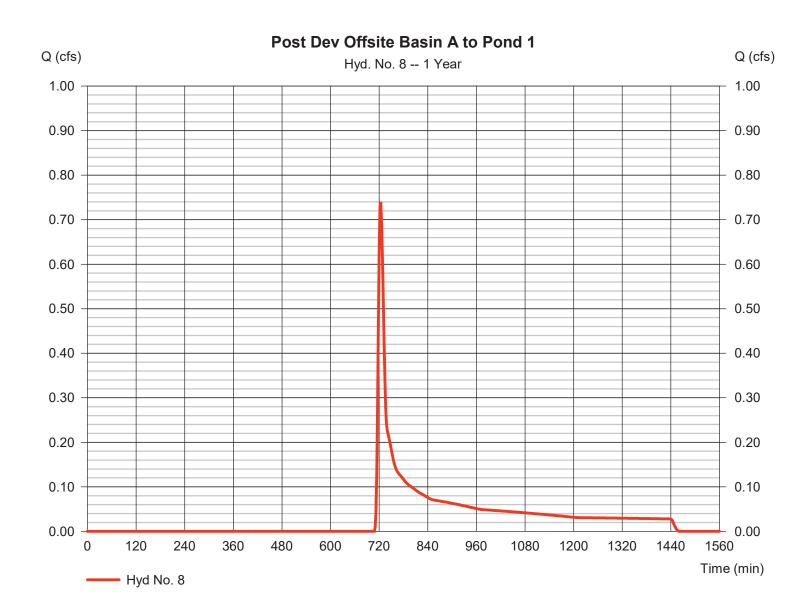
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

Friday, 01 / 31 / 2025

Hyd. No. 8

Post Dev Offsite Basin A to Pond 1

Hydrograph type = SCS Runoff Peak discharge = 0.740 cfsStorm frequency Time to peak = 724 min = 1 yrsTime interval = 2 min Hyd. volume = 2.944 cuft Drainage area Curve number = 1.910 ac= 59 Hydraulic length Basin Slope = 0.0 %= 0 ftTc method Time of conc. (Tc) = 12.00 min = User Total precip. = 3.30 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



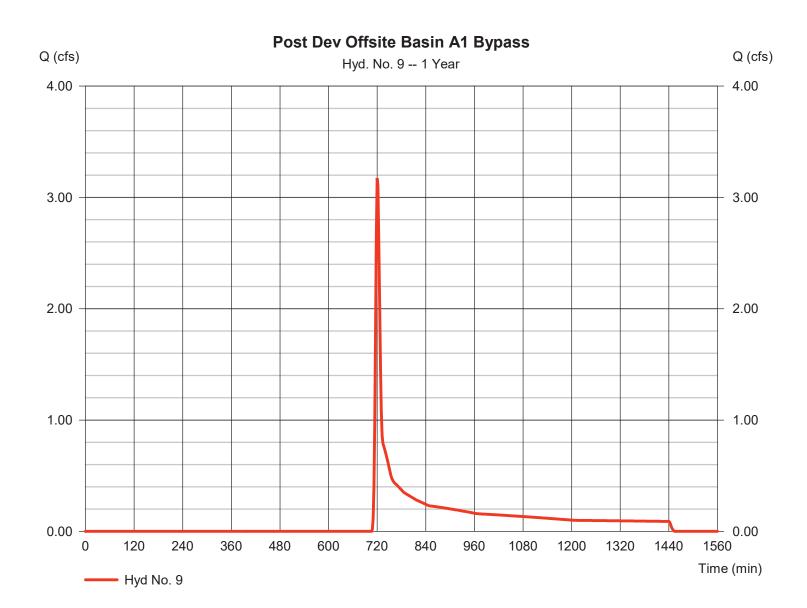
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

Friday, 01 / 31 / 2025

Hyd. No. 9

Post Dev Offsite Basin A1 Bypass

Hydrograph type	= SCS Runoff	Peak discharge	= 3.174 cfs
Storm frequency	= 1 yrs	Time to peak	= 720 min
Time interval	= 2 min	Hyd. volume	= 9,757 cuft
Drainage area	= 6.000 ac	Curve number	= 60
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= User	Time of conc. (Tc)	= 8.00 min
Total precip.	= 3.30 in	Distribution	= Type II
Storm duration	= 24 hrs	Shape factor	= 484



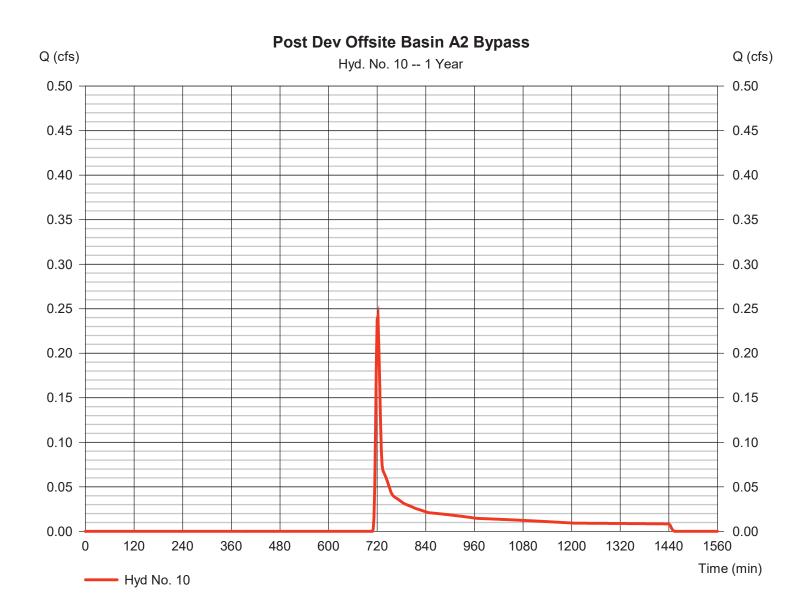
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

Friday, 01 / 31 / 2025

Hyd. No. 10

Post Dev Offsite Basin A2 Bypass

Hydrograph type = SCS Runoff Peak discharge = 0.242 cfsStorm frequency = 1 yrsTime to peak = 722 min Time interval = 2 min Hyd. volume = 848 cuft Drainage area = 0.620 acCurve number = 58 Hydraulic length Basin Slope = 0.0 %= 0 ftTc method Time of conc. (Tc) = 8.00 min = User Total precip. = 3.30 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



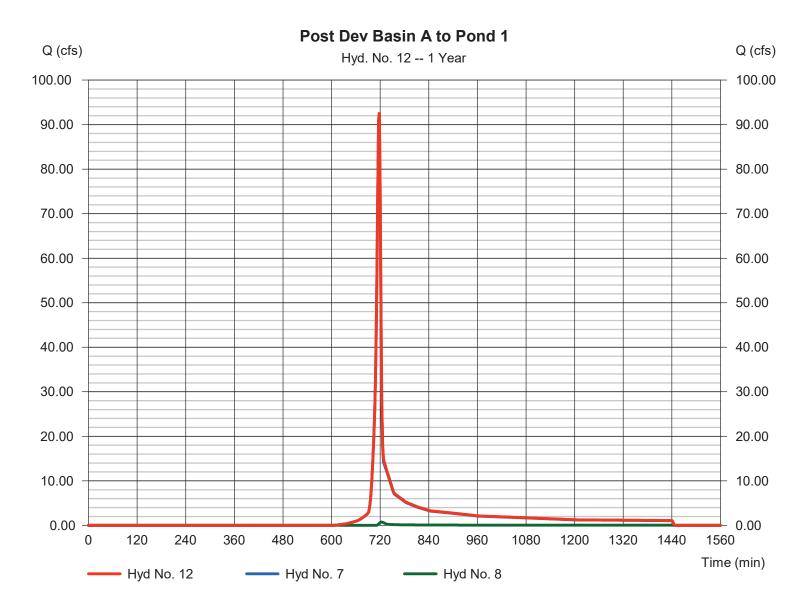
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

Friday, 01 / 31 / 2025

Hyd. No. 12

Post Dev Basin A to Pond 1

Hydrograph type = Combine Peak discharge = 92.77 cfsStorm frequency Time to peak = 1 yrs= 718 min Time interval = 2 min Hyd. volume = 187,611 cuft Inflow hyds. = 7, 8Contrib. drain. area = 44.180 ac



Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

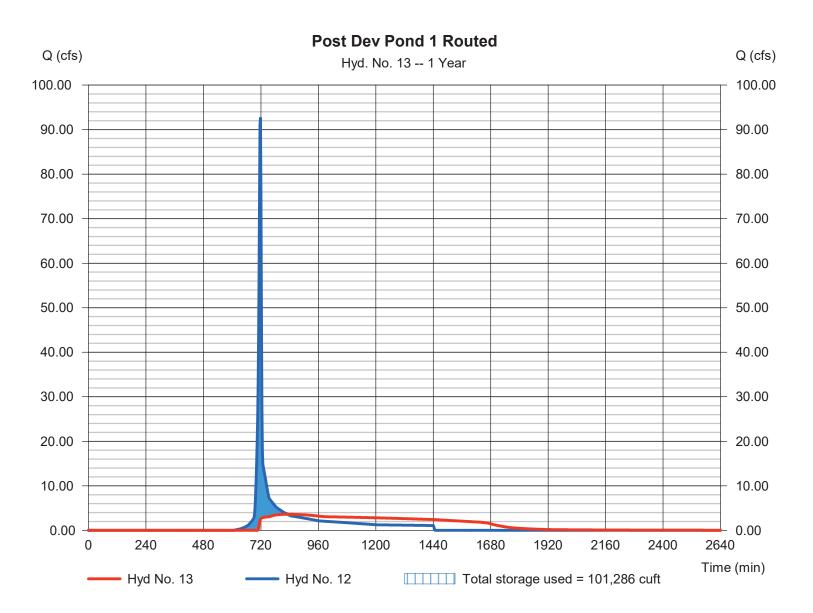
Friday, 01 / 31 / 2025

Hyd. No. 13

Post Dev Pond 1 Routed

Hydrograph type Peak discharge = 3.601 cfs= Reservoir Storm frequency Time to peak = 832 min = 1 yrsTime interval = 2 min Hyd. volume = 171,640 cuft Inflow hyd. No. = 12 - Post Dev Basin A to PondMax. Elevation = 1056.29 ft= Pond 1 Max. Storage Reservoir name = 101,286 cuft

Storage Indication method used.



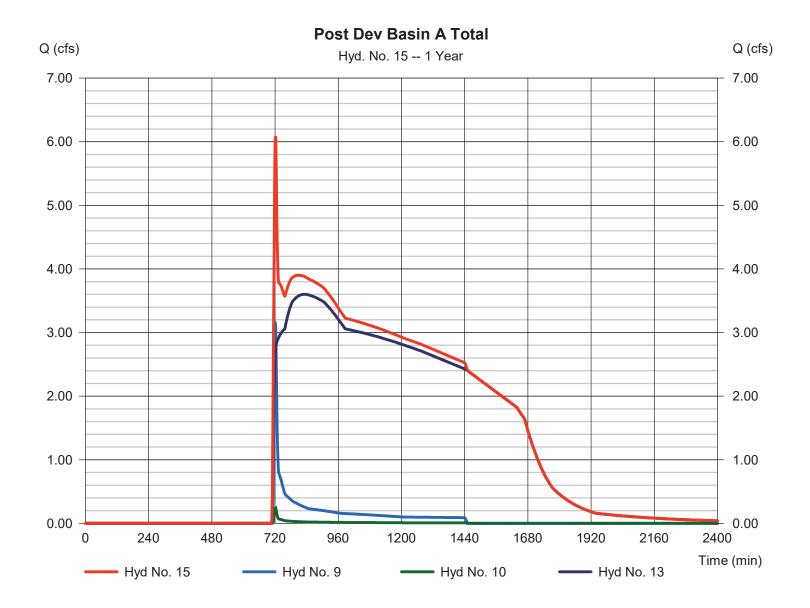
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Friday, 01 / 31 / 2025

Hyd. No. 15

Post Dev Basin A Total

Hydrograph type = Combine Peak discharge = 6.085 cfsStorm frequency Time to peak = 1 yrs= 722 min Time interval = 2 min Hyd. volume = 182.245 cuft Inflow hyds. = 9, 10, 13 Contrib. drain. area = 6.620 ac



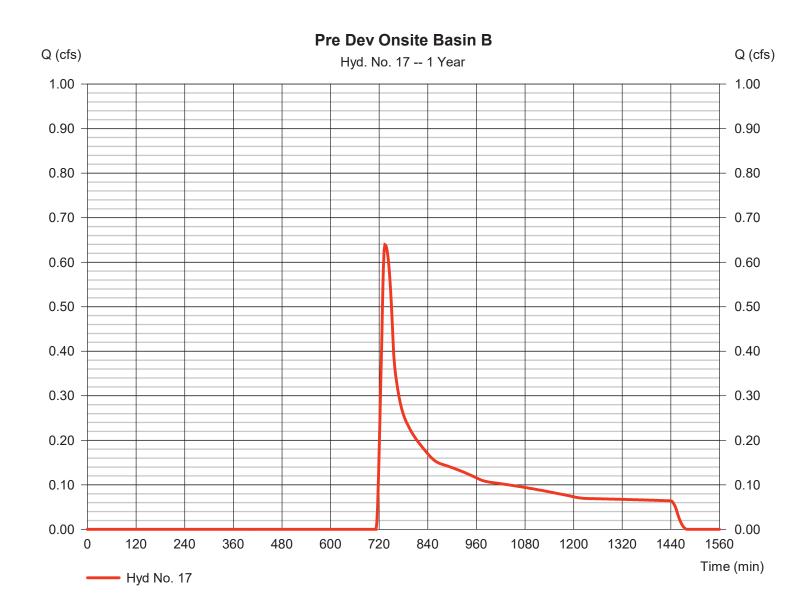
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

Friday, 01 / 31 / 2025

Hyd. No. 17

Pre Dev Onsite Basin B

Hydrograph type = SCS Runoff Peak discharge = 0.639 cfsStorm frequency Time to peak = 734 min = 1 yrsTime interval = 2 min Hyd. volume = 5,716 cuftDrainage area Curve number = 5.700 ac= 55 Hydraulic length Basin Slope = 0.0 %= 0 ftTc method Time of conc. (Tc) = 25.00 min = User Total precip. = 3.30 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



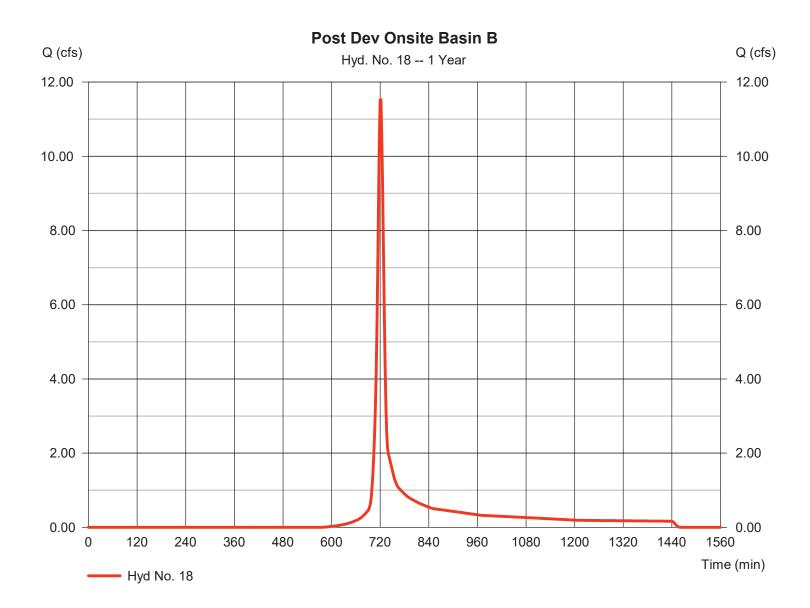
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

Friday, 01 / 31 / 2025

Hyd. No. 18

Post Dev Onsite Basin B

Hydrograph type = SCS Runoff Peak discharge = 11.52 cfsStorm frequency Time to peak = 722 min = 1 yrs= 30,134 cuft Time interval = 2 min Hyd. volume Drainage area Curve number = 5.700 ac= 79 Hydraulic length Basin Slope = 0.0 %= 0 ftTc method Time of conc. (Tc) = 10.00 min = User Total precip. = 3.30 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



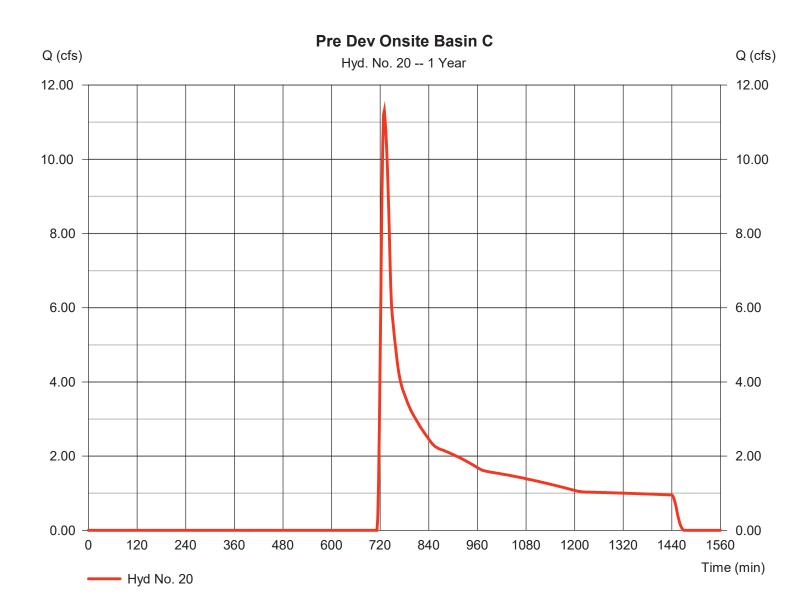
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

Friday, 01 / 31 / 2025

Hyd. No. 20

Pre Dev Onsite Basin C

Hydrograph type = SCS Runoff Peak discharge = 11.31 cfsStorm frequency = 1 yrsTime to peak = 730 min Time interval = 2 min Hyd. volume = 85,196 cuft Drainage area Curve number = 83.630 ac = 55 Basin Slope = 0.0 %Hydraulic length = 0 ftTc method Time of conc. (Tc) = 17.00 min = User Total precip. = 3.30 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



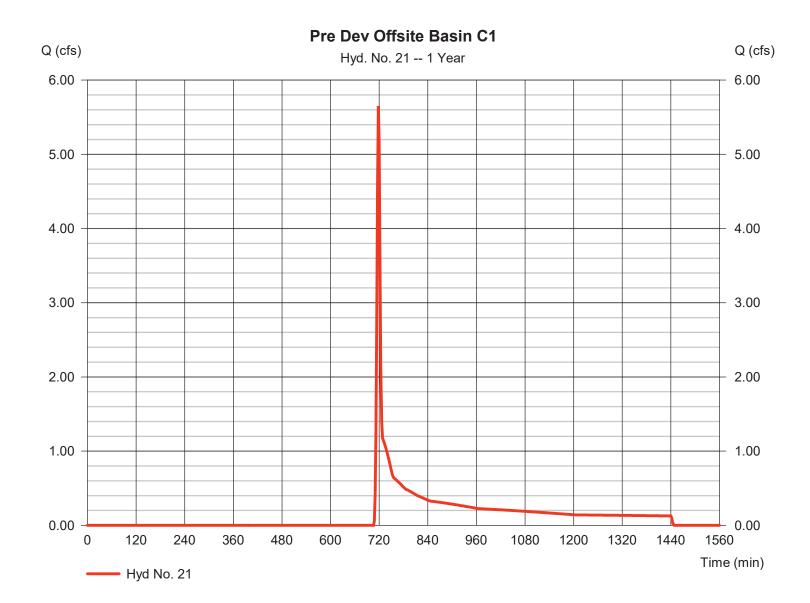
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

Friday, 01 / 31 / 2025

Hyd. No. 21

Pre Dev Offsite Basin C1

Hydrograph type = SCS Runoff Peak discharge = 5.649 cfsStorm frequency Time to peak = 718 min = 1 yrsTime interval = 2 min Hyd. volume = 13.919 cuft Drainage area Curve number = 9.130 ac= 60 Basin Slope = 0.0 %Hydraulic length = 0 ftTc method Time of conc. (Tc) = 6.00 min = User Total precip. = 3.30 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



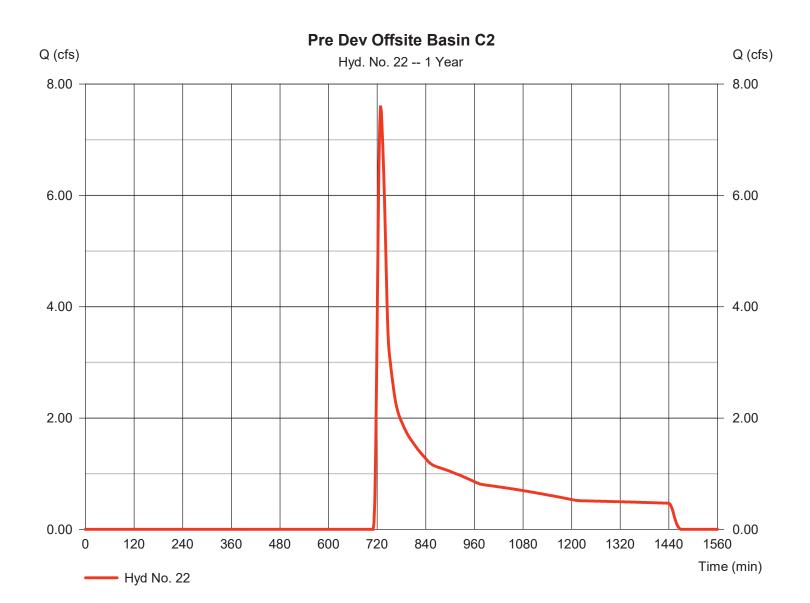
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

Friday, 01 / 31 / 2025

Hyd. No. 22

Pre Dev Offsite Basin C2

Hydrograph type = SCS Runoff Peak discharge = 7.613 cfsStorm frequency = 1 yrsTime to peak = 728 min Time interval = 2 min Hyd. volume = 45,786 cuft = 36.720 ac Drainage area Curve number = 57 Basin Slope = 0.0 %Hydraulic length = 0 ftTc method Time of conc. (Tc) = 18.00 min = User Total precip. = 3.30 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



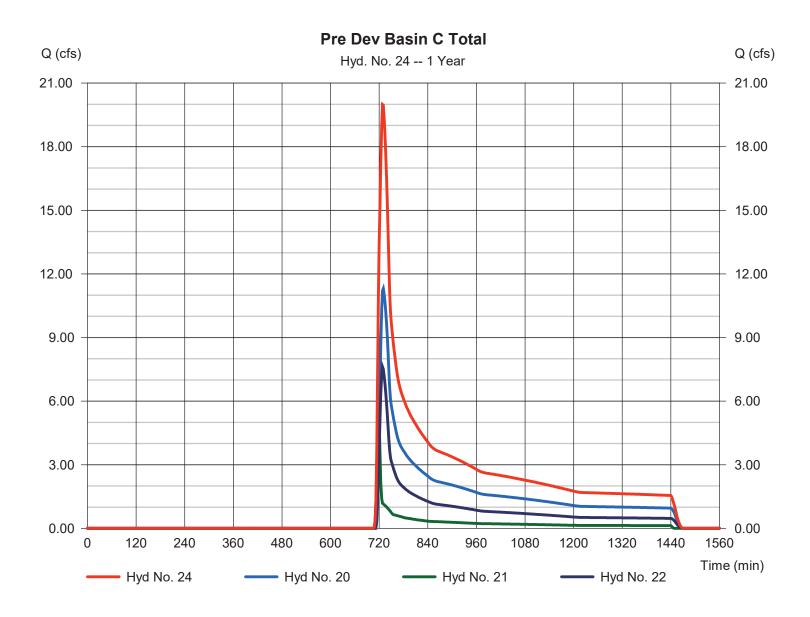
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

Friday, 01 / 31 / 2025

Hyd. No. 24

Pre Dev Basin C Total

Hydrograph type = Combine Peak discharge = 20.00 cfsStorm frequency Time to peak = 1 yrs= 728 min Time interval = 2 min Hyd. volume = 144,901 cuft Inflow hyds. = 20, 21, 22 Contrib. drain. area = 129.480 ac



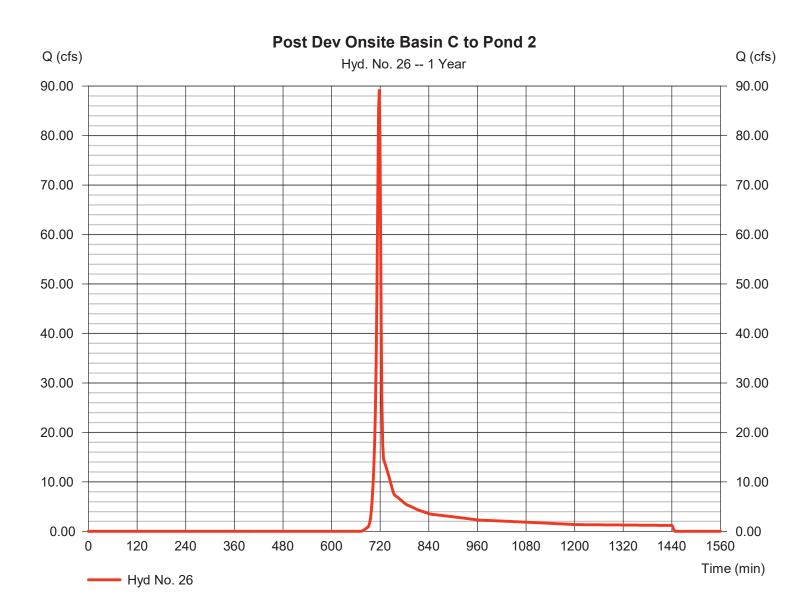
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

Friday, 01 / 31 / 2025

Hyd. No. 26

Post Dev Onsite Basin C to Pond 2

Hydrograph type = SCS Runoff Peak discharge = 89.33 cfsStorm frequency = 1 yrsTime to peak = 718 min Time interval = 2 min Hyd. volume = 180.416 cuft Drainage area = 56.470 ac Curve number = 71 Hydraulic length = 0 ftBasin Slope = 0.0 %Tc method Time of conc. (Tc) $= 5.00 \, \text{min}$ = User Total precip. = 3.30 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



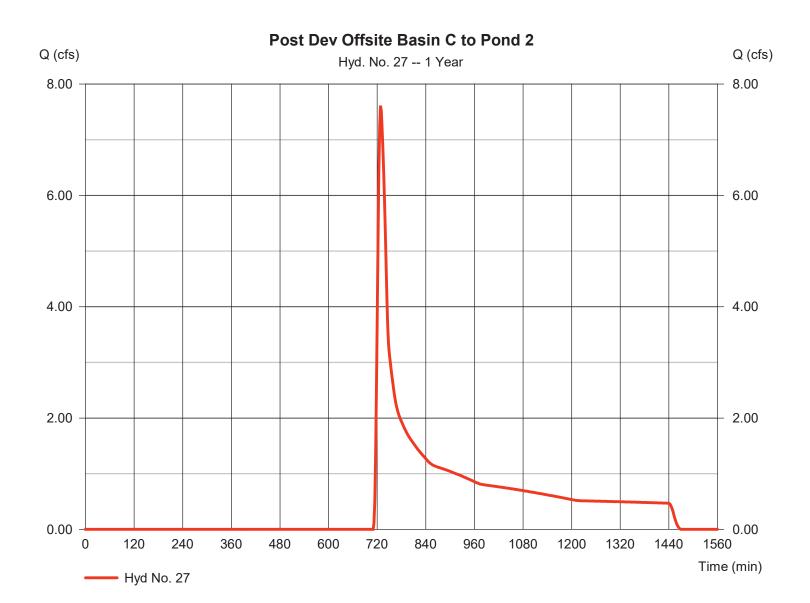
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Friday, 01 / 31 / 2025

Hyd. No. 27

Post Dev Offsite Basin C to Pond 2

= SCS Runoff Hydrograph type Peak discharge = 7.613 cfsStorm frequency = 1 yrsTime to peak = 728 min Time interval = 2 min Hyd. volume = 45,786 cuft Drainage area = 36.720 ac Curve number = 57 Hydraulic length Basin Slope = 0.0 %= 0 ftTc method Time of conc. (Tc) = 18.00 min = User Total precip. = 3.30 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



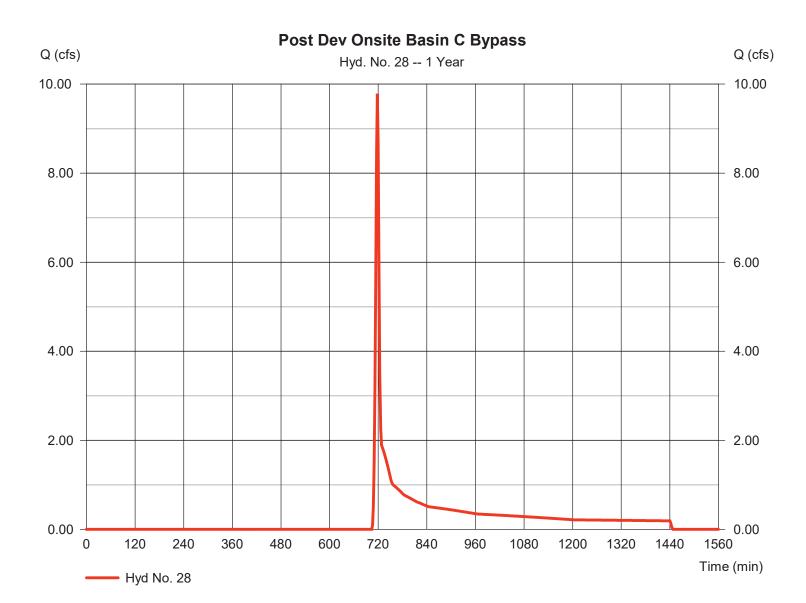
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Friday, 01 / 31 / 2025

Hyd. No. 28

Post Dev Onsite Basin C Bypass

Hydrograph type = SCS Runoff Peak discharge = 9.782 cfsStorm frequency Time to peak = 718 min = 1 yrsTime interval = 2 min Hyd. volume = 22.362 cuft Drainage area = 12.530 acCurve number = 62 Hydraulic length Basin Slope = 0.0 %= 0 ftTc method Time of conc. (Tc) $= 5.00 \, \text{min}$ = User Total precip. = 3.30 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



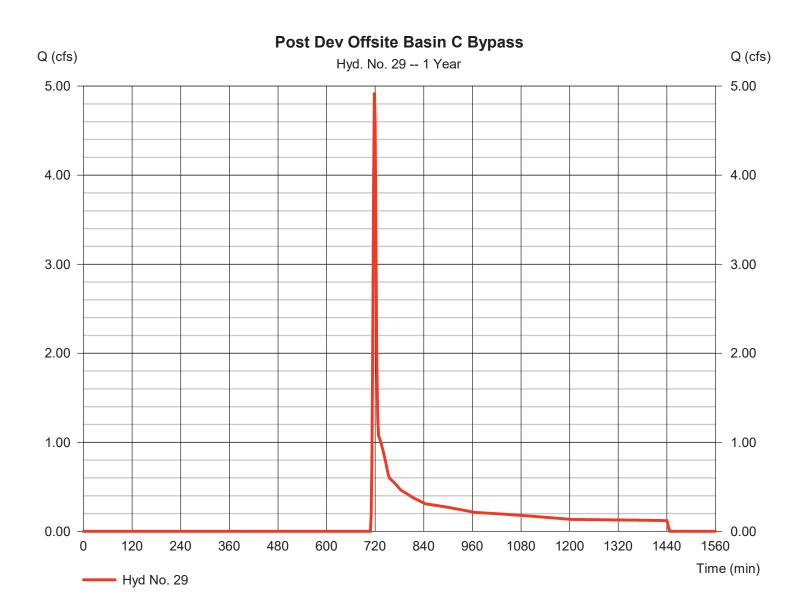
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Friday, 01 / 31 / 2025

Hyd. No. 29

Post Dev Offsite Basin C Bypass

Hydrograph type = SCS Runoff Peak discharge = 4.930 cfsStorm frequency Time to peak = 718 min = 1 yrsTime interval = 2 min Hyd. volume = 12.792 cuft Drainage area Curve number = 9.130 ac= 59 Basin Slope = 0.0 %Hydraulic length = 0 ftTc method Time of conc. (Tc) = 6.00 min = User Total precip. = 3.30 inDistribution = Type II Shape factor Storm duration = 24 hrs = 484



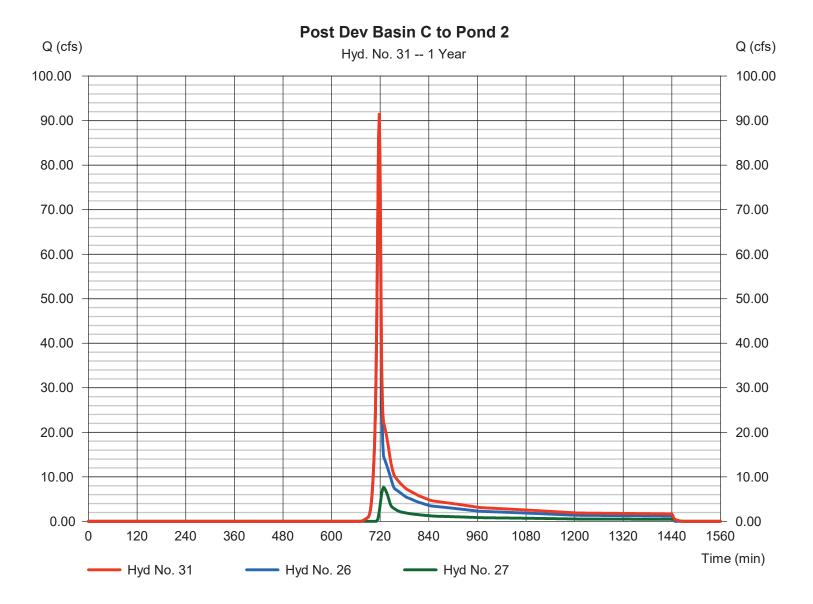
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Friday, 01 / 31 / 2025

Hyd. No. 31

Post Dev Basin C to Pond 2

Hydrograph type = Combine Peak discharge = 91.72 cfsStorm frequency Time to peak = 1 yrs= 718 min Time interval = 2 min Hyd. volume = 226,203 cuft Inflow hyds. = 26, 27Contrib. drain. area = 93.190 ac



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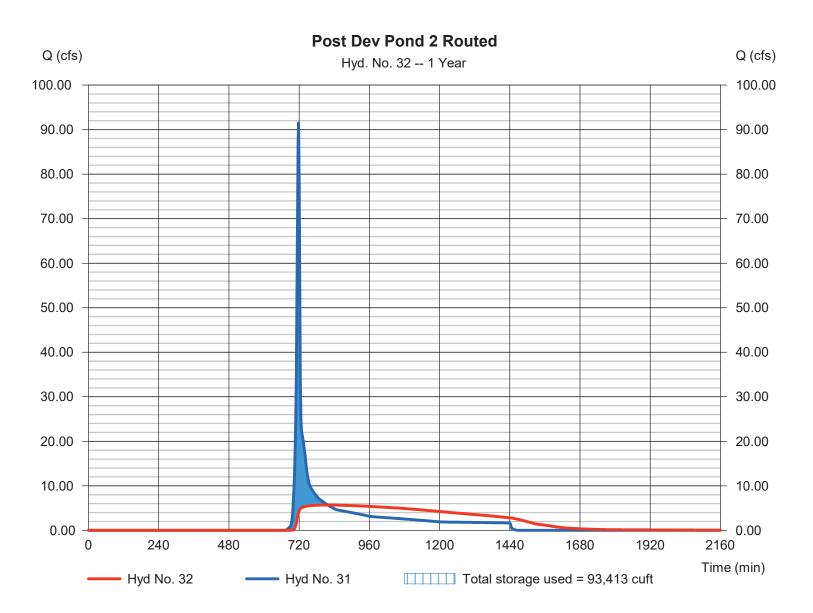
Friday, 01 / 31 / 2025

Hyd. No. 32

Post Dev Pond 2 Routed

Hydrograph type Peak discharge = 5.690 cfs= Reservoir Storm frequency Time to peak = 818 min = 1 yrsTime interval = 2 min Hyd. volume = 226,184 cuft Inflow hyd. No. = 31 - Post Dev Basin C to PondMax. Elevation = 1106.11 ft = Pond 2 Max. Storage Reservoir name = 93,413 cuft

Storage Indication method used.



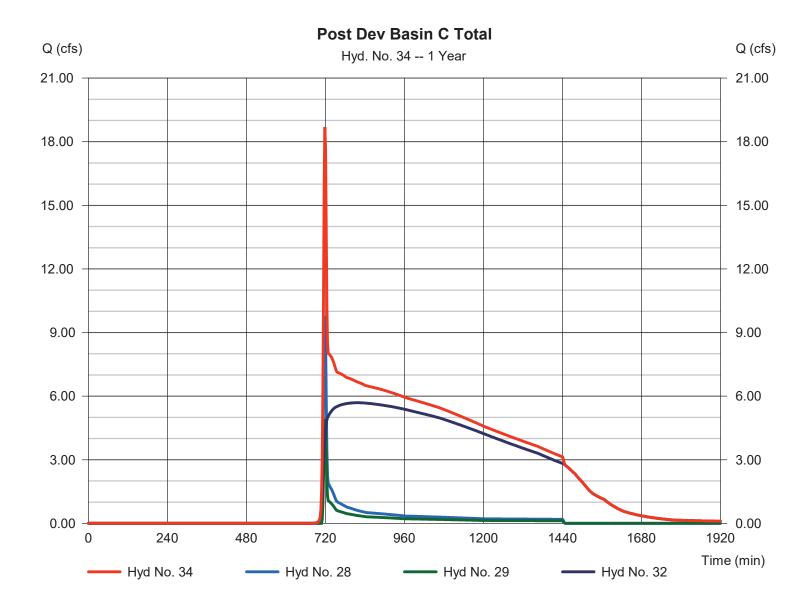
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Friday, 01 / 31 / 2025

Hyd. No. 34

Post Dev Basin C Total

Hydrograph type = Combine Peak discharge = 18.69 cfsStorm frequency Time to peak = 1 yrs= 718 min Time interval = 2 min Hyd. volume = 261,338 cuft Inflow hyds. = 28, 29, 32 Contrib. drain. area = 21.660 ac



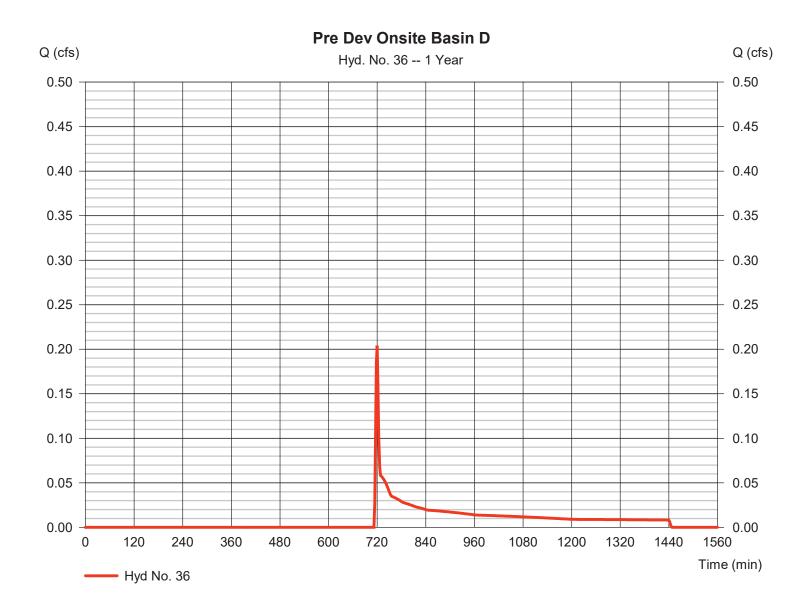
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Friday, 01 / 31 / 2025

Hyd. No. 36

Pre Dev Onsite Basin D

Hydrograph type = SCS Runoff Peak discharge = 0.204 cfsStorm frequency Time to peak = 720 min = 1 yrsTime interval = 2 min Hyd. volume = 735 cuft Drainage area Curve number = 0.770 ac= 55 Hydraulic length Basin Slope = 0.0 %= 0 ftTc method Time of conc. (Tc) = 5.00 min = User Total precip. = 3.30 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



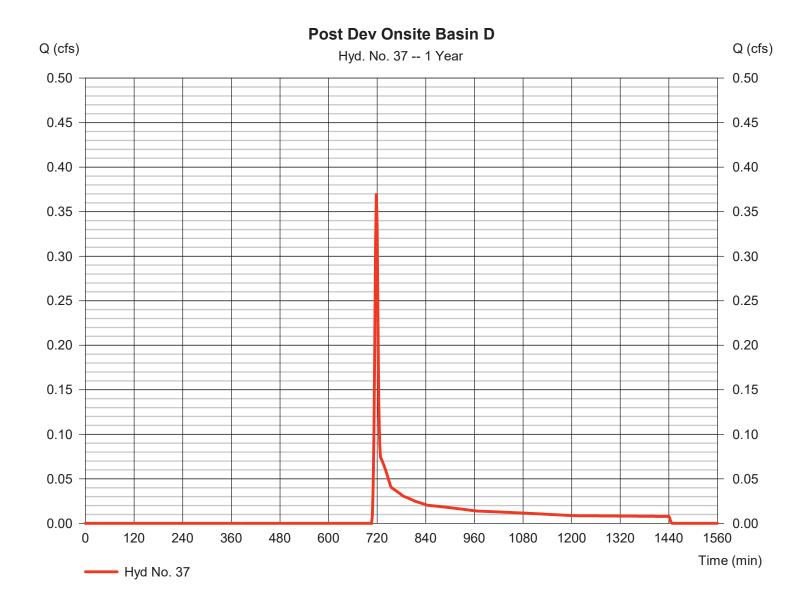
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Friday, 01 / 31 / 2025

Hyd. No. 37

Post Dev Onsite Basin D

Hydrograph type = SCS Runoff Peak discharge = 0.370 cfsStorm frequency Time to peak = 718 min = 1 yrsTime interval = 2 min Hyd. volume = 875 cuft Drainage area = 0.530 acCurve number = 61 Hydraulic length Basin Slope = 0.0 %= 0 ftTc method Time of conc. (Tc) $= 5.00 \, \text{min}$ = User Total precip. = 3.30 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



Hydrograph Summary Report

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

Hyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description
1	SCS Runoff	12.88	2	730	79,788				Pre Dev Onsite Basin A
2	SCS Runoff	6.488	2	722	20,256				Pre Dev Offite Basin A1
3	SCS Runoff	0.426	2	720	1,239				Pre Dev Offsite Basin A2
5	Combine	17.34	2	728	101,283	1, 2, 3,			Pre Dev Basin A Total
7	SCS Runoff	115.48	2	718	231,769				Post Dev Onsite Basin A to Pond 1
8	SCS Runoff	1.259	2	722	4,246				Post Dev Offsite Basin A to Pond 1
9	SCS Runoff	5.166	2	720	13,903				Post Dev Offsite Basin A1 Bypass
10	SCS Runoff	0.426	2	720	1,239				Post Dev Offsite Basin A2 Bypass
12	Combine	116.38	2	718	236,015	7, 8,			Post Dev Basin A to Pond 1
13	Reservoir	4.149	2	838	220,044	12	1057.49	131,311	Post Dev Pond 1 Routed
15	Combine	8.550	2	720	235,186	9, 10, 13,			Post Dev Basin A Total
17	SCS Runoff	1.324	2	732	8,762				Pre Dev Onsite Basin B
18	SCS Runoff	14.39	2	720	37,439				Post Dev Onsite Basin B
20	SCS Runoff	23.85	2	728	130,589				Pre Dev Onsite Basin C
21	SCS Runoff	8.905	2	718	19,834				Pre Dev Offsite Basin C1
22	SCS Runoff	14.18	2	728	67,882				Pre Dev Offsite Basin C2
24	Combine	39.71	2	728	218,305	20, 21, 22,			Pre Dev Basin C Total
26	SCS Runoff	116.93	2	718	234,322				Post Dev Onsite Basin C to Pond 2
27	SCS Runoff	14.18	2	728	67,882				Post Dev Offsite Basin C to Pond 2
28	SCS Runoff	14.57	2	718	31,180				Post Dev Onsite Basin C Bypass
29	SCS Runoff	8.069	2	718	18,453				Post Dev Offsite Basin C Bypass
31	Combine	122.76	2	718	302,204	26, 27,			Post Dev Basin C to Pond 2
32	Reservoir	10.34	2	776	302,184	31	1107.38	125,945	Post Dev Pond 2 Routed
34	Combine	27.42	2	718	351,817	28, 29, 32,			Post Dev Basin C Total
36	SCS Runoff	0.409	2	718	1,127				Pre Dev Onsite Basin D
37	SCS Runoff	0.566	2	718	1,234				Post Dev Onsite Basin D
122	 20263 Hydrold	gy Conce	ept.gpw		Return F	Return Period: 2 Year			/ 31 / 2025

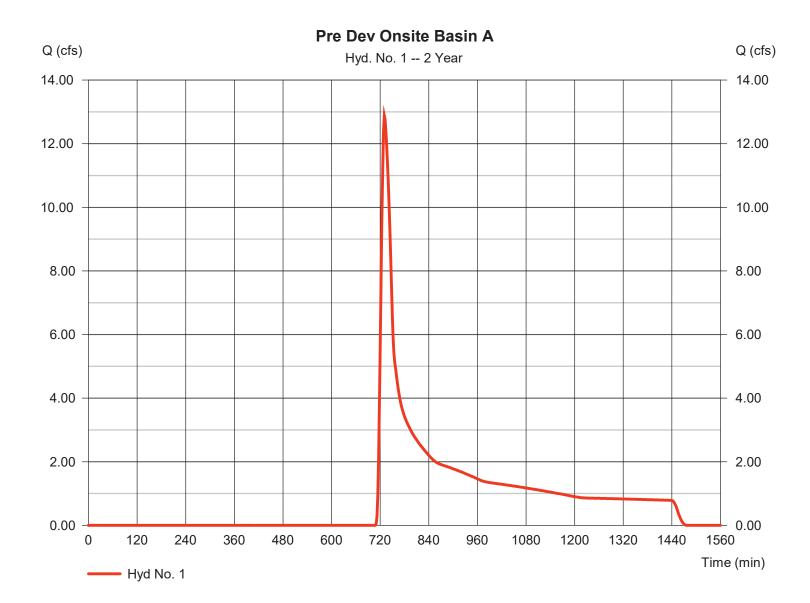
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Friday, 01 / 31 / 2025

Hyd. No. 1

Pre Dev Onsite Basin A

Hydrograph type = SCS Runoff Peak discharge = 12.88 cfsStorm frequency = 2 yrsTime to peak = 730 min Time interval = 2 min Hyd. volume = 79.788 cuft Drainage area Curve number = 50.200 ac= 55 Hydraulic length Basin Slope = 0.0 %= 0 ftTc method Time of conc. (Tc) = 22.00 min = User Total precip. = 3.74 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



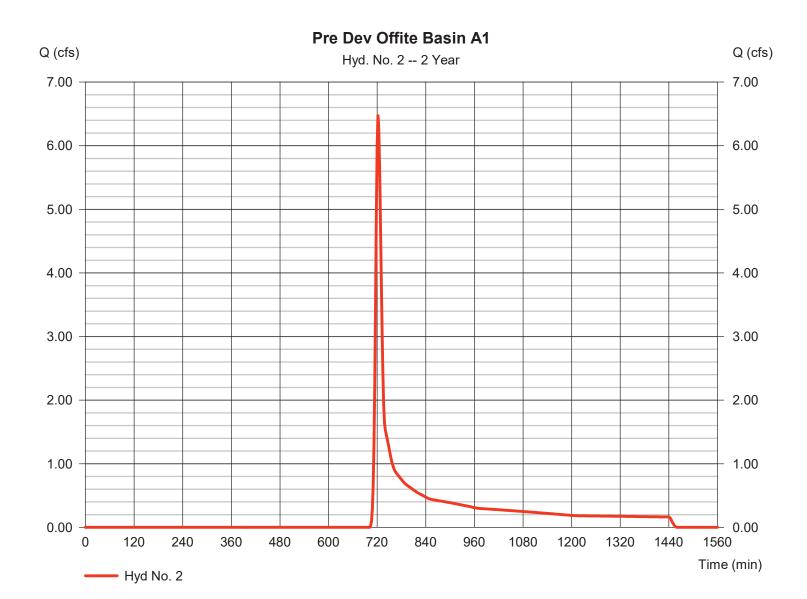
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Friday, 01 / 31 / 2025

Hyd. No. 2

Pre Dev Offite Basin A1

Hydrograph type = SCS Runoff Peak discharge = 6.488 cfsStorm frequency = 2 yrsTime to peak = 722 min Time interval = 2 min Hyd. volume = 20.256 cuft Drainage area = 7.910 acCurve number = 61 Hydraulic length Basin Slope = 0.0 %= 0 ftTc method Time of conc. (Tc) = 12.00 min = User Total precip. = 3.74 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



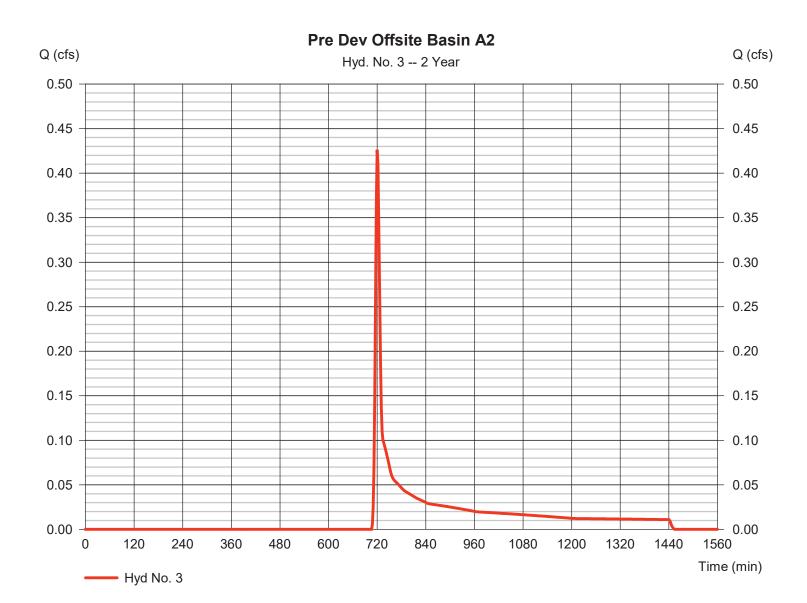
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Friday, 01 / 31 / 2025

Hyd. No. 3

Pre Dev Offsite Basin A2

Hydrograph type = SCS Runoff Peak discharge = 0.426 cfsStorm frequency = 2 yrsTime to peak = 720 min Time interval = 2 min Hyd. volume = 1.239 cuft Drainage area = 0.620 acCurve number = 58 Hydraulic length Basin Slope = 0.0 %= 0 ftTc method Time of conc. (Tc) $= 8.00 \, \text{min}$ = User Total precip. = 3.74 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



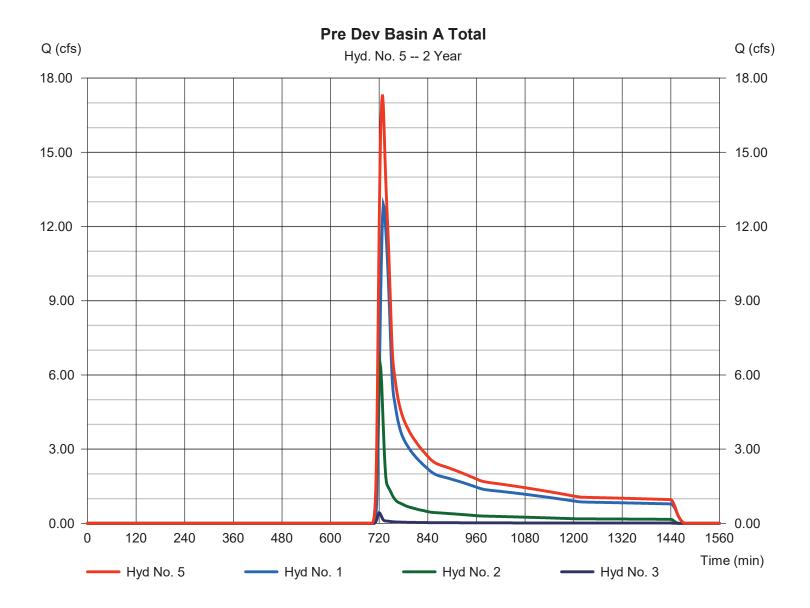
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Friday, 01 / 31 / 2025

Hyd. No. 5

Pre Dev Basin A Total

Hydrograph type = Combine Peak discharge = 17.34 cfsStorm frequency Time to peak = 2 yrs= 728 min Time interval = 2 min Hyd. volume = 101,283 cuft Inflow hyds. = 1, 2, 3Contrib. drain. area = 58.730 ac



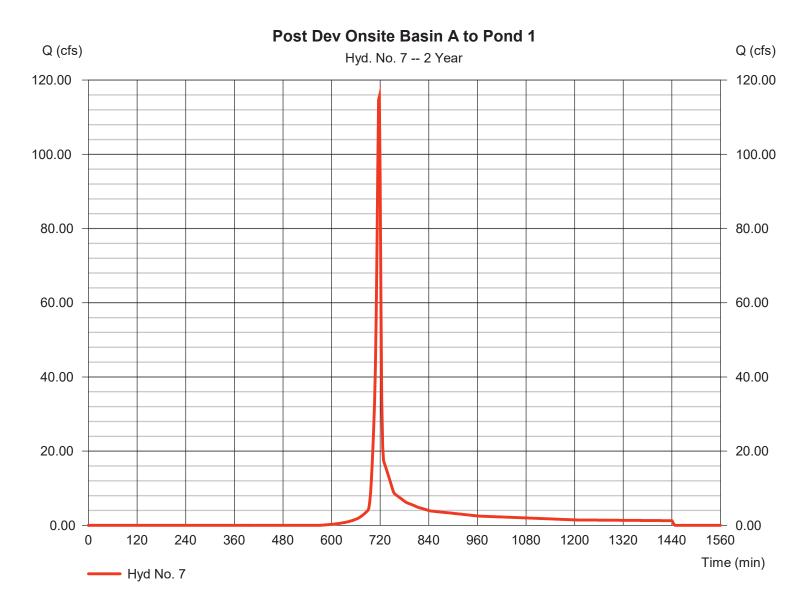
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Friday, 01 / 31 / 2025

Hyd. No. 7

Post Dev Onsite Basin A to Pond 1

Hydrograph type = SCS Runoff Peak discharge = 115.48 cfsStorm frequency = 2 yrsTime to peak = 718 min Time interval = 2 min Hyd. volume = 231,769 cuft Drainage area = 42.270 ac Curve number = 77 Hydraulic length = 0 ftBasin Slope = 0.0 %Tc method Time of conc. (Tc) $= 5.00 \, \text{min}$ = User Total precip. = 3.74 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



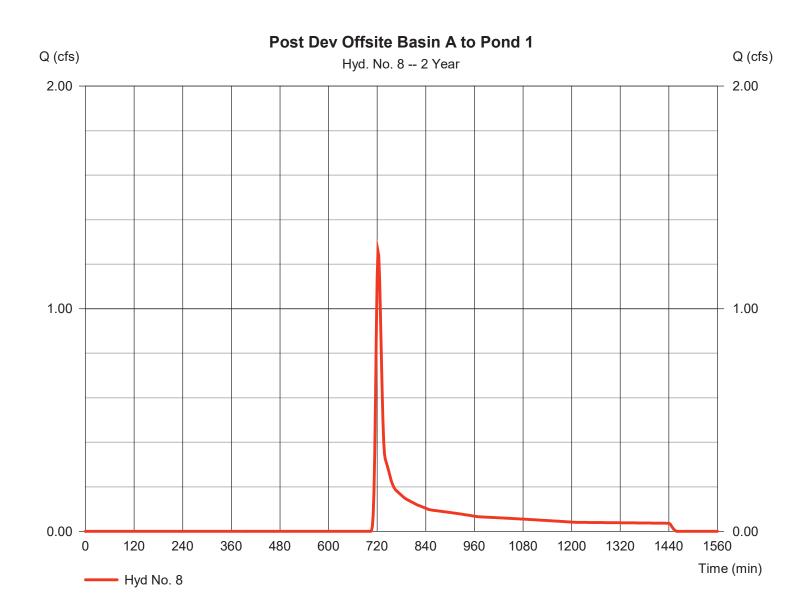
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Friday, 01 / 31 / 2025

Hyd. No. 8

Post Dev Offsite Basin A to Pond 1

= SCS Runoff Hydrograph type Peak discharge = 1.259 cfsStorm frequency = 2 yrsTime to peak = 722 min Time interval = 2 min Hyd. volume = 4.246 cuft Drainage area = 1.910 acCurve number = 59 Hydraulic length Basin Slope = 0.0 %= 0 ftTc method Time of conc. (Tc) = 12.00 min = User Total precip. = 3.74 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



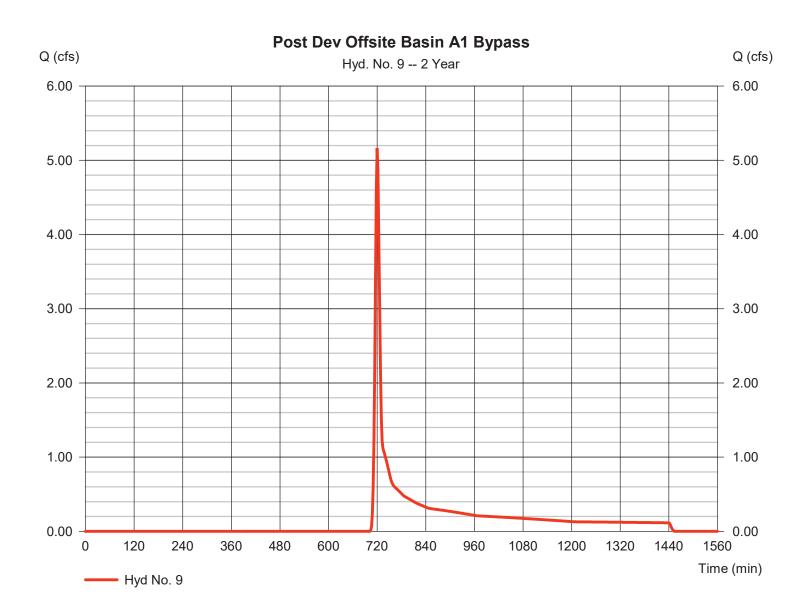
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Friday, 01 / 31 / 2025

Hyd. No. 9

Post Dev Offsite Basin A1 Bypass

Hydrograph type = SCS Runoff Peak discharge = 5.166 cfsStorm frequency = 2 yrsTime to peak = 720 min Time interval = 2 min Hyd. volume = 13,903 cuft Drainage area Curve number = 6.000 ac= 60 Basin Slope = 0.0 %Hydraulic length = 0 ftTc method Time of conc. (Tc) = 8.00 min = User Total precip. = 3.74 inDistribution = Type II Storm duration Shape factor = 24 hrs = 484



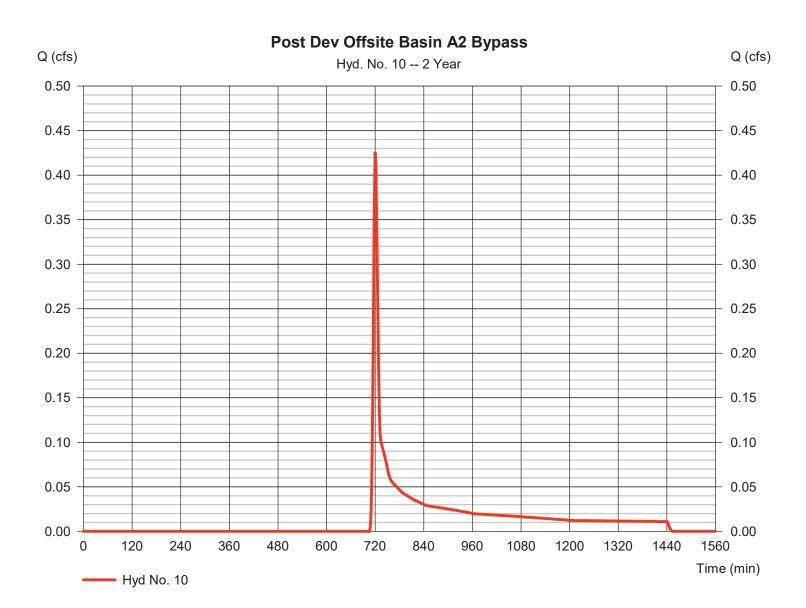
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Friday, 01 / 31 / 2025

Hyd. No. 10

Post Dev Offsite Basin A2 Bypass

Hydrograph type = SCS Runoff Peak discharge = 0.426 cfsStorm frequency = 2 yrsTime to peak = 720 min Time interval = 2 min Hyd. volume = 1.239 cuftDrainage area = 0.620 acCurve number = 58 Hydraulic length Basin Slope = 0.0 %= 0 ftTc method Time of conc. (Tc) = 8.00 min = User Total precip. = 3.74 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



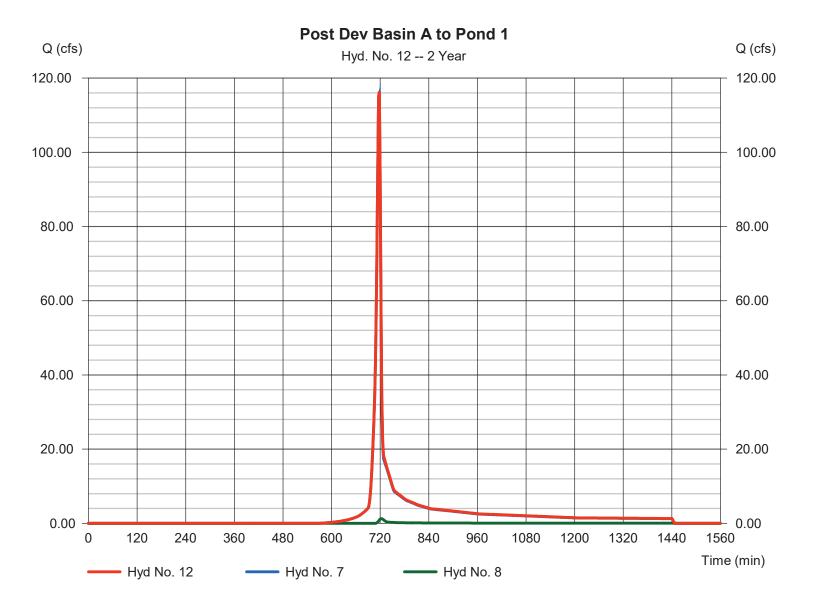
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Friday, 01 / 31 / 2025

Hyd. No. 12

Post Dev Basin A to Pond 1

Hydrograph type = Combine Peak discharge = 116.38 cfsStorm frequency Time to peak = 2 yrs= 718 min Time interval = 2 min Hyd. volume = 236,015 cuft Inflow hyds. = 7,8 Contrib. drain. area = 44.180 ac



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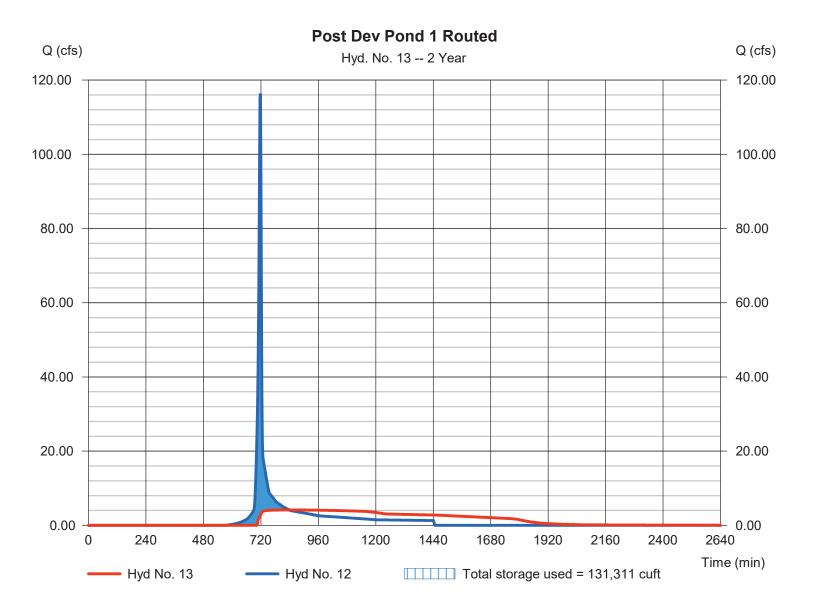
Friday, 01 / 31 / 2025

Hyd. No. 13

Post Dev Pond 1 Routed

Hydrograph type Peak discharge = 4.149 cfs= Reservoir Storm frequency = 2 yrsTime to peak = 838 min Time interval = 2 min Hyd. volume = 220,044 cuft Inflow hyd. No. = 12 - Post Dev Basin A to PondMax. Elevation = 1057.49 ft= Pond 1 Reservoir name Max. Storage = 131,311 cuft

Storage Indication method used.



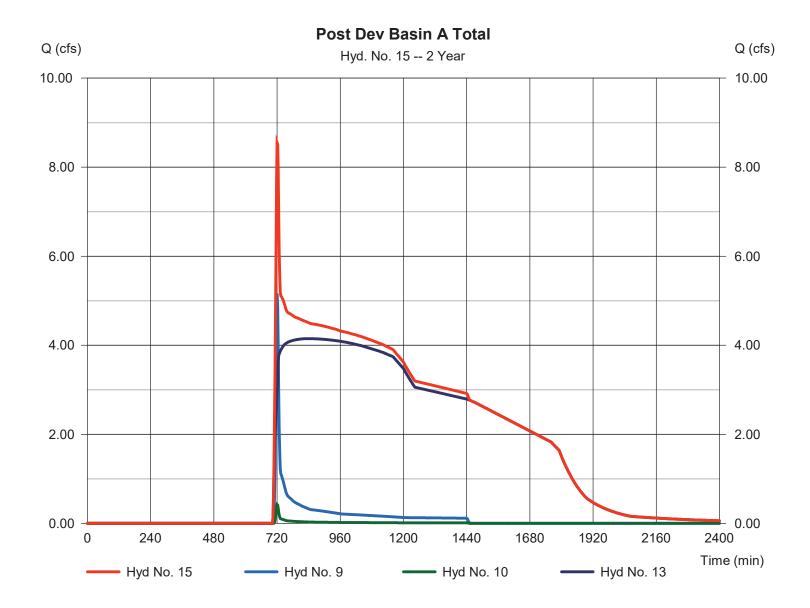
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Friday, 01 / 31 / 2025

Hyd. No. 15

Post Dev Basin A Total

Hydrograph type = Combine Peak discharge = 8.550 cfsStorm frequency Time to peak = 2 yrs= 720 min Time interval = 2 min Hyd. volume = 235,186 cuft Inflow hyds. = 9, 10, 13 Contrib. drain. area = 6.620 ac



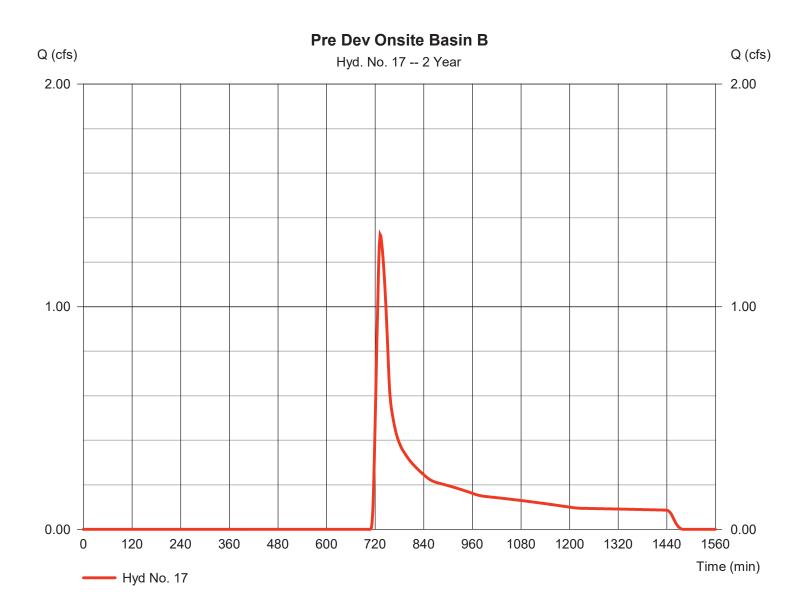
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Friday, 01 / 31 / 2025

Hyd. No. 17

Pre Dev Onsite Basin B

Hydrograph type = SCS Runoff Peak discharge = 1.324 cfsStorm frequency = 2 yrsTime to peak = 732 min Time interval = 2 min Hyd. volume = 8,762 cuft Drainage area = 5.700 acCurve number = 55 Basin Slope = 0.0 %Hydraulic length = 0 ftTc method Time of conc. (Tc) = 25.00 min = User Total precip. = 3.74 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



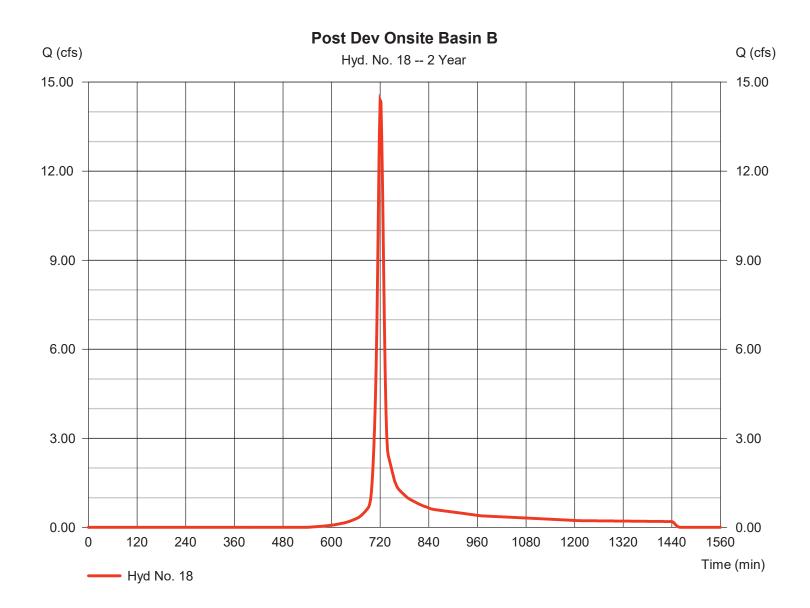
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Friday, 01 / 31 / 2025

Hyd. No. 18

Post Dev Onsite Basin B

Hydrograph type = SCS Runoff Peak discharge = 14.39 cfsStorm frequency = 2 yrsTime to peak = 720 min Time interval = 2 min Hyd. volume = 37.439 cuft Drainage area = 5.700 acCurve number = 79 Hydraulic length Basin Slope = 0.0 %= 0 ftTc method Time of conc. (Tc) = 10.00 min = User Total precip. = 3.74 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



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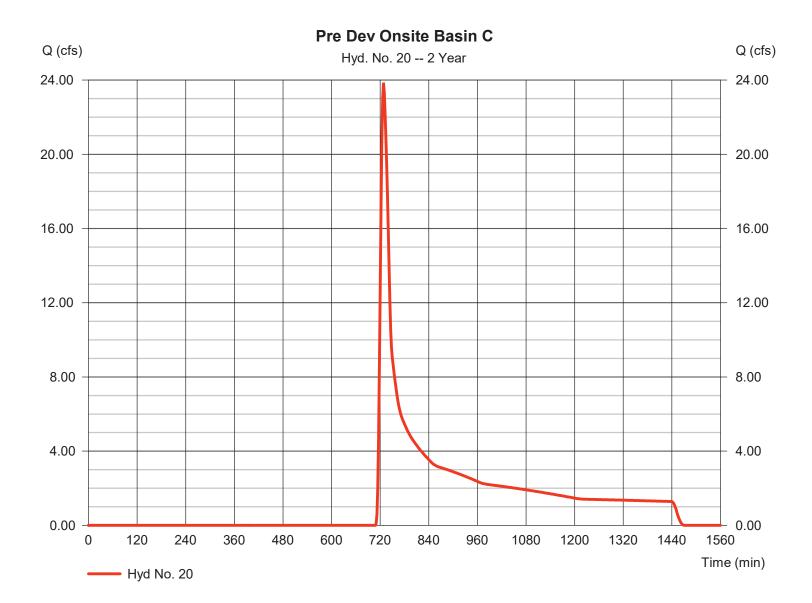
Friday, 01 / 31 / 2025

Hyd. No. 20

Pre Dev Onsite Basin C

Hydrograph type = SCS Runoff Peak discharge = 23.85 cfsStorm frequency = 2 yrsTime to peak = 728 min Time interval = 2 min Hyd. volume = 130,589 cuft Drainage area Curve number = 83.630 ac = 55 Hydraulic length Basin Slope = 0.0 %= 0 ftTc method Time of conc. (Tc) = 17.00 min = User

Total precip. = 3.74 in Distribution = Type II Storm duration = 24 hrs Shape factor = 484



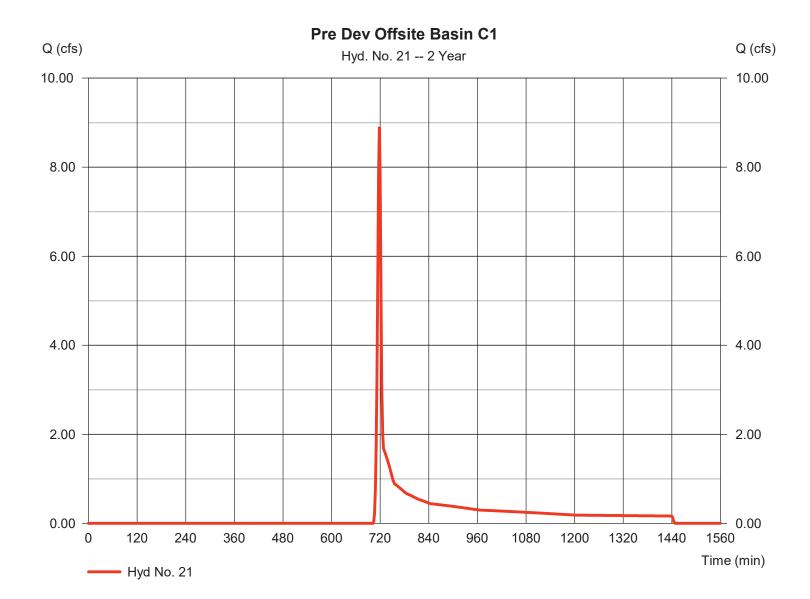
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Friday, 01 / 31 / 2025

Hyd. No. 21

Pre Dev Offsite Basin C1

Hydrograph type = SCS Runoff Peak discharge = 8.905 cfsStorm frequency = 2 yrsTime to peak = 718 min Time interval = 2 min Hyd. volume = 19.834 cuft Drainage area = 9.130 acCurve number = 60 Hydraulic length Basin Slope = 0.0 %= 0 ftTc method Time of conc. (Tc) $= 6.00 \, \text{min}$ = User Total precip. = 3.74 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



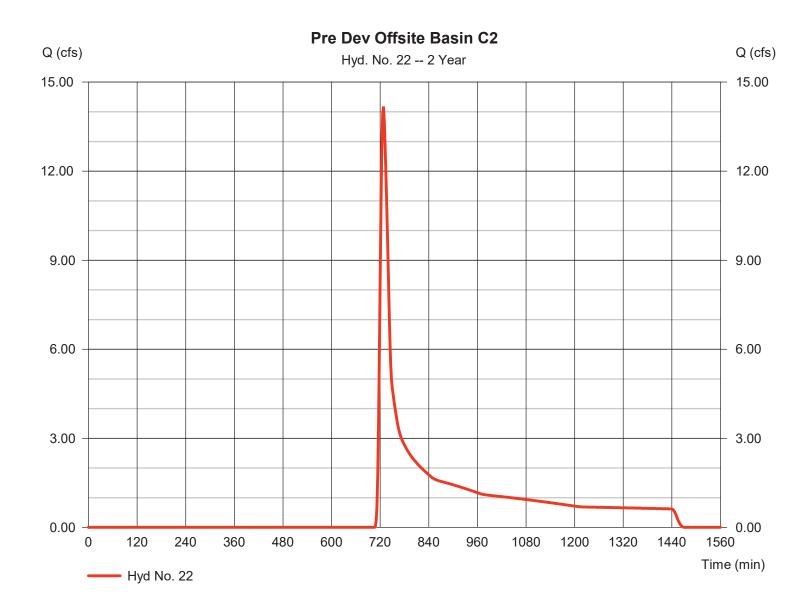
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Friday, 01 / 31 / 2025

Hyd. No. 22

Pre Dev Offsite Basin C2

Hydrograph type = SCS Runoff Peak discharge = 14.18 cfsStorm frequency = 2 yrsTime to peak = 728 min Time interval = 2 min Hyd. volume = 67.882 cuft = 36.720 ac Drainage area Curve number = 57 Hydraulic length Basin Slope = 0.0 %= 0 ftTc method Time of conc. (Tc) = 18.00 min = User Total precip. = 3.74 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



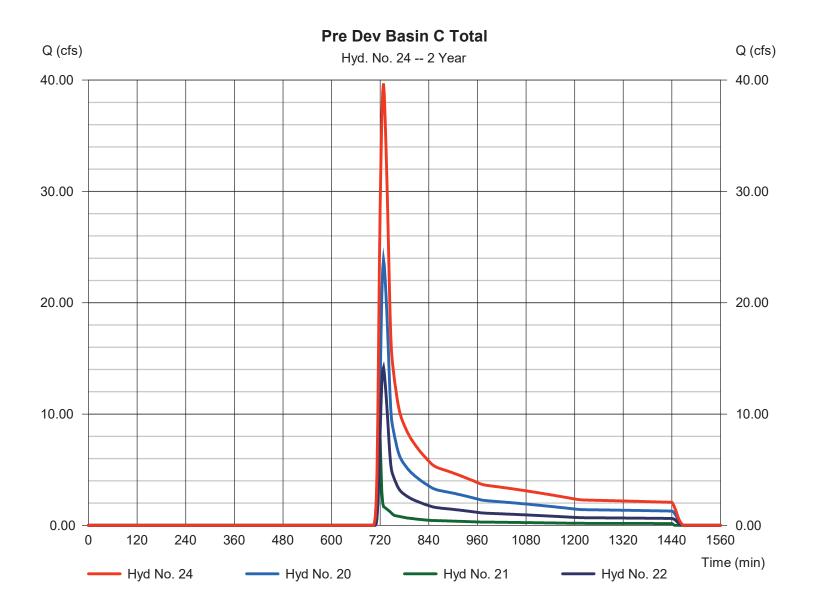
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Friday, 01 / 31 / 2025

Hyd. No. 24

Pre Dev Basin C Total

Hydrograph type = Combine Peak discharge = 39.71 cfsStorm frequency Time to peak = 2 yrs= 728 min Time interval = 2 min Hyd. volume = 218,305 cuft Inflow hyds. = 20, 21, 22 Contrib. drain. area = 129.480 ac



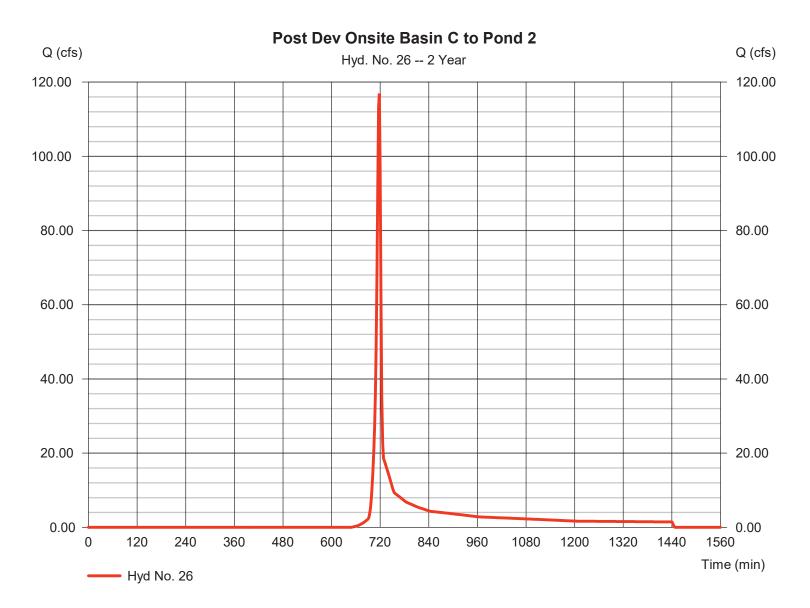
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Friday, 01 / 31 / 2025

Hyd. No. 26

Post Dev Onsite Basin C to Pond 2

Hydrograph type = SCS Runoff Peak discharge = 116.93 cfsStorm frequency = 2 yrsTime to peak = 718 min Time interval = 2 min Hyd. volume = 234,322 cuft Drainage area = 56.470 ac Curve number = 71 Hydraulic length = 0 ftBasin Slope = 0.0 %Tc method Time of conc. (Tc) $= 5.00 \, \text{min}$ = User Total precip. = 3.74 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



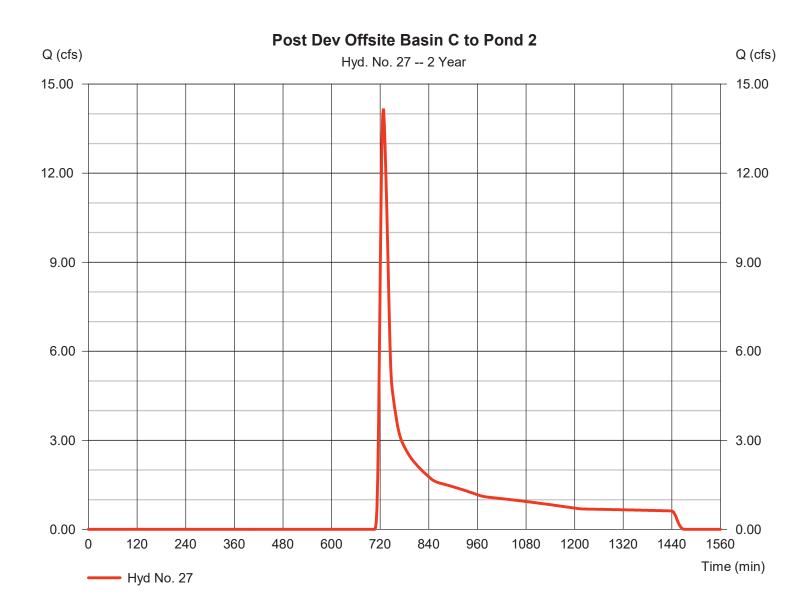
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Friday, 01 / 31 / 2025

Hyd. No. 27

Post Dev Offsite Basin C to Pond 2

= SCS Runoff Hydrograph type Peak discharge = 14.18 cfsStorm frequency = 2 yrsTime to peak = 728 min Time interval = 2 min Hyd. volume = 67.882 cuft Drainage area = 36.720 ac Curve number = 57 Hydraulic length Basin Slope = 0.0 %= 0 ftTc method Time of conc. (Tc) = 18.00 min = User Total precip. = 3.74 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



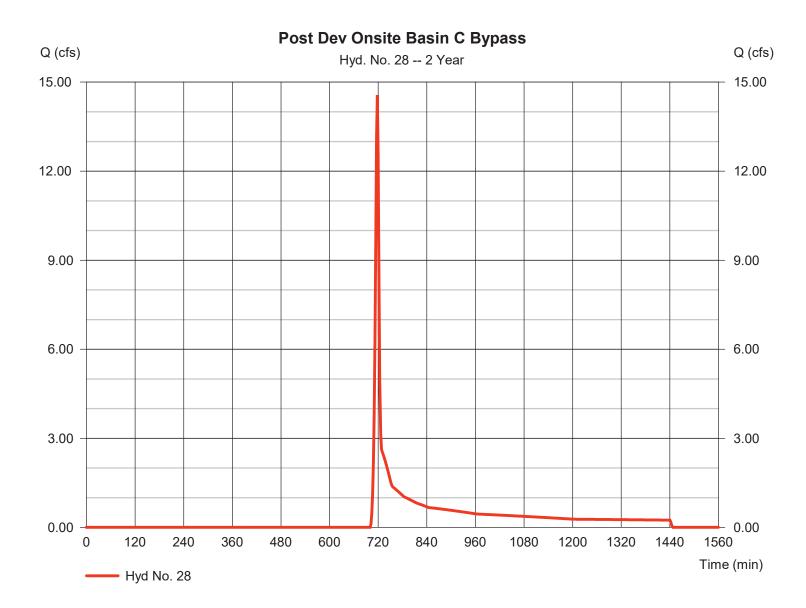
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Friday, 01 / 31 / 2025

Hyd. No. 28

Post Dev Onsite Basin C Bypass

Hydrograph type = SCS Runoff Peak discharge = 14.57 cfsStorm frequency = 2 yrsTime to peak = 718 min Time interval = 2 min Hyd. volume = 31,180 cuft Drainage area = 12.530 acCurve number = 62 Hydraulic length Basin Slope = 0.0 %= 0 ftTc method Time of conc. (Tc) = 5.00 min = User Total precip. = 3.74 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



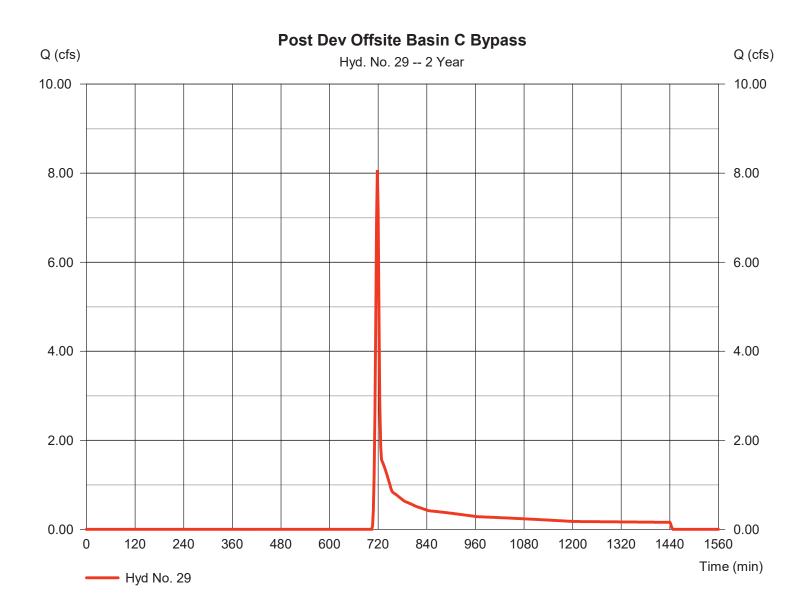
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Friday, 01 / 31 / 2025

Hyd. No. 29

Post Dev Offsite Basin C Bypass

Hydrograph type = SCS Runoff Peak discharge = 8.069 cfsStorm frequency = 2 yrsTime to peak = 718 min Time interval = 2 min Hyd. volume = 18.453 cuft Drainage area = 9.130 acCurve number = 59 Hydraulic length Basin Slope = 0.0 %= 0 ftTc method Time of conc. (Tc) = 6.00 min = User Total precip. = 3.74 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



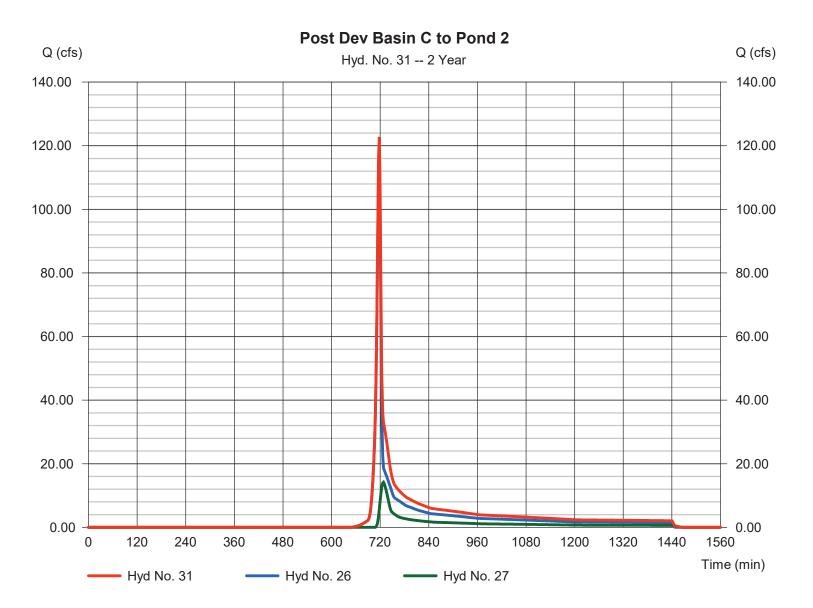
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Friday, 01 / 31 / 2025

Hyd. No. 31

Post Dev Basin C to Pond 2

Hydrograph type = Combine Peak discharge = 122.76 cfsStorm frequency Time to peak = 2 yrs= 718 min Time interval = 2 min Hyd. volume = 302,204 cuft Inflow hyds. = 26, 27Contrib. drain. area = 93.190 ac



Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

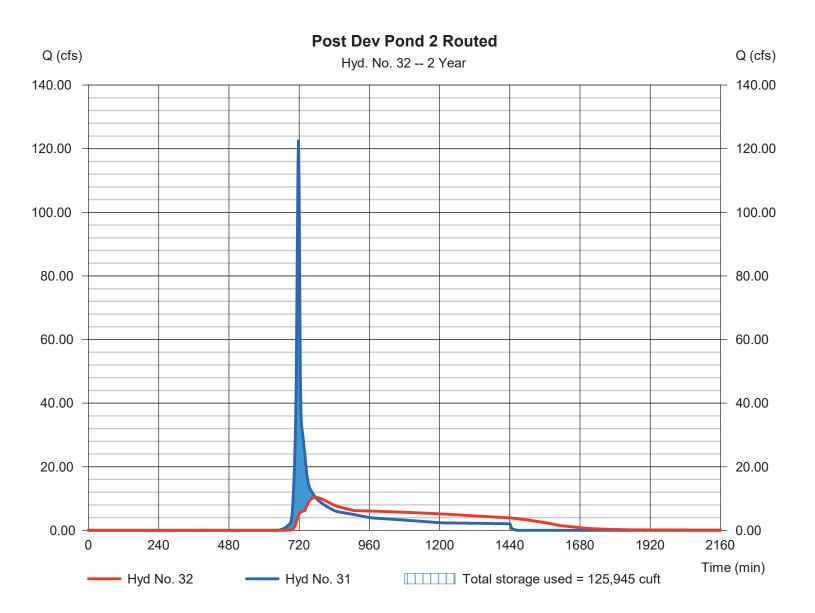
Friday, 01 / 31 / 2025

Hyd. No. 32

Post Dev Pond 2 Routed

Hydrograph type Peak discharge = 10.34 cfs= Reservoir Storm frequency = 2 yrsTime to peak = 776 min Time interval = 2 min Hyd. volume = 302,184 cuft Inflow hyd. No. = 31 - Post Dev Basin C to PondMax. Elevation = 1107.38 ft= Pond 2 Max. Storage Reservoir name = 125,945 cuft

Storage Indication method used.



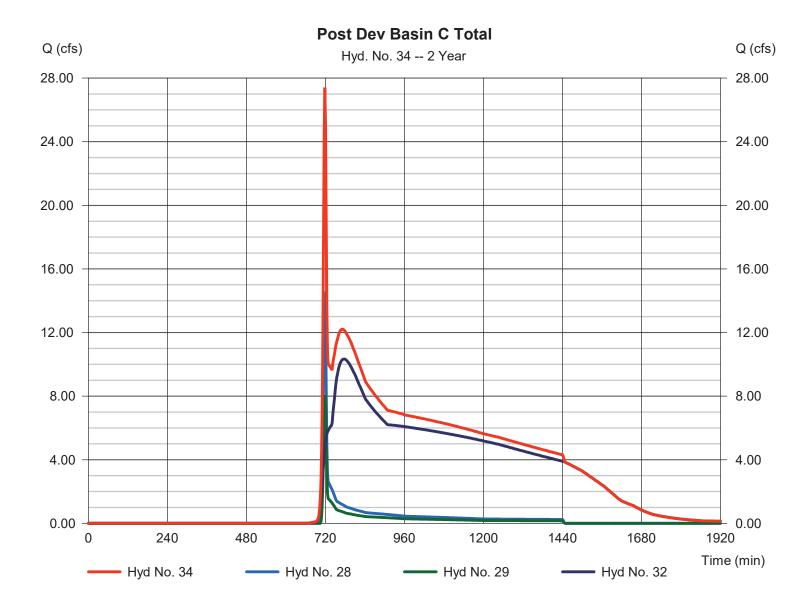
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Friday, 01 / 31 / 2025

Hyd. No. 34

Post Dev Basin C Total

Hydrograph type = Combine Peak discharge = 27.42 cfsStorm frequency Time to peak = 2 yrs= 718 min Time interval = 2 min Hyd. volume = 351,817 cuft Inflow hyds. = 28, 29, 32 Contrib. drain. area = 21.660 ac



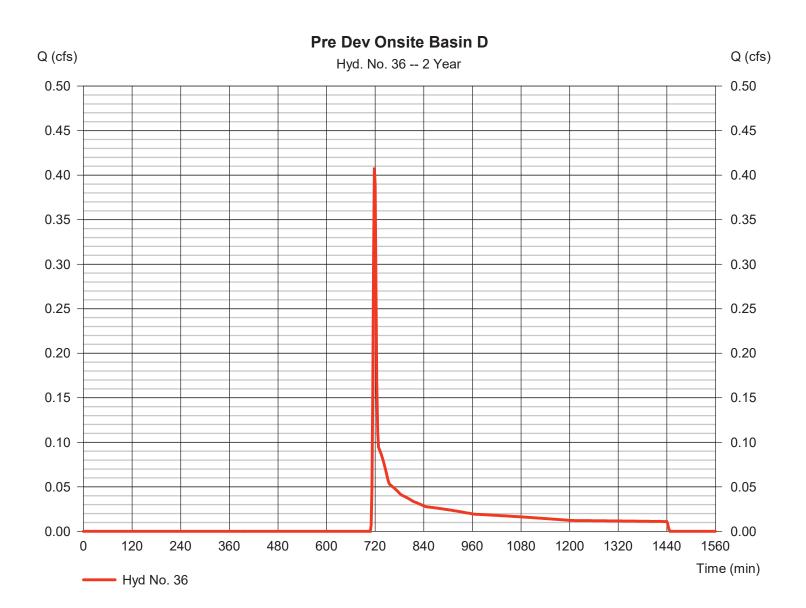
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

Friday, 01 / 31 / 2025

Hyd. No. 36

Pre Dev Onsite Basin D

Hydrograph type = SCS Runoff Peak discharge = 0.409 cfsStorm frequency = 2 yrsTime to peak = 718 min Time interval = 2 min Hyd. volume = 1.127 cuft Drainage area = 0.770 acCurve number = 55 Hydraulic length Basin Slope = 0.0 %= 0 ftTc method Time of conc. (Tc) $= 5.00 \, \text{min}$ = User Total precip. = 3.74 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



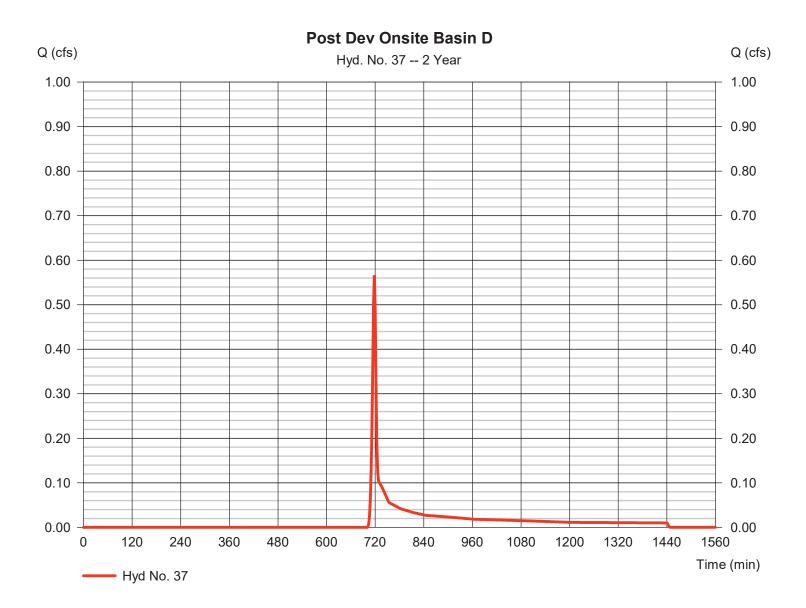
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Friday, 01 / 31 / 2025

Hyd. No. 37

Post Dev Onsite Basin D

Hydrograph type = SCS Runoff Peak discharge = 0.566 cfsStorm frequency = 2 yrsTime to peak = 718 min Time interval = 2 min Hyd. volume = 1.234 cuft Drainage area = 0.530 acCurve number = 61 Hydraulic length Basin Slope = 0.0 %= 0 ftTc method Time of conc. (Tc) = 5.00 min = User Total precip. = 3.74 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



Hydrograph Summary Report

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

Hyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description
1	SCS Runoff	29.06	2	730	138,510				Pre Dev Onsite Basin A
2	SCS Runoff	11.35	2	722	32,123				Pre Dev Offite Basin A1
3	SCS Runoff	0.810	2	720	2,048				Pre Dev Offsite Basin A2
5	Combine	37.00	2	728	172,680	1, 2, 3,			Pre Dev Basin A Total
7	SCS Runoff	158.06	2	716	319,165				Post Dev Onsite Basin A to Pond 1
8	SCS Runoff	2.360	2	722	6,913				Post Dev Offsite Basin A to Pond 1
9	SCS Runoff	9.128	2	720	22,328				Post Dev Offsite Basin A1 Bypass
10	SCS Runoff	0.810	2	720	2,048				Post Dev Offsite Basin A2 Bypass
12	Combine	159.50	2	718	326,078	7, 8,			Post Dev Basin A to Pond 1
13	Reservoir	4.804	2	870	310,107	12	1059.66	193,778	Post Dev Pond 1 Routed
15	Combine	14.07	2	720	334,483	9, 10, 13,			Post Dev Basin A Total
17	SCS Runoff	2.983	2	732	15,210				Pre Dev Onsite Basin B
18	SCS Runoff	19.63	2	720	50,887				Post Dev Onsite Basin B
20	SCS Runoff	53.58	2	726	226,700				Pre Dev Onsite Basin C
21	SCS Runoff	15.35	2	718	31,853				Pre Dev Offsite Basin C1
22	SCS Runoff	29.06	2	726	113,856				Pre Dev Offsite Basin C2
24	Combine	85.81	2	726	372,409	20, 21, 22,			Pre Dev Basin C Total
26	SCS Runoff	168.45	2	718	337,001				Post Dev Onsite Basin C to Pond 2
27	SCS Runoff	29.06	2	726	113,856				Post Dev Offsite Basin C to Pond 2
28	SCS Runoff	23.92	2	718	48,859				Post Dev Onsite Basin C Bypass
29	SCS Runoff	14.32	2	718	30,040				Post Dev Offsite Basin C Bypass
31	Combine	184.02	2	718	450,857	26, 27,			Post Dev Basin C to Pond 2
32	Reservoir	38.65	2	738	450,839	31	1108.53	159,944	Post Dev Pond 2 Routed
34	Combine	44.14	2	718	529,738	28, 29, 32,			Post Dev Basin C Total
36	SCS Runoff	0.870	2	718	1,957				Pre Dev Onsite Basin D
37	SCS Runoff	0.951	2	718	1,957				Post Dev Onsite Basin D
122	⊥ 20263 Hydrolo	ogy Conce	ept.gpw	l	Return F	⊥ Period: 5 Ye	ear	Friday, 01	/ 31 / 2025

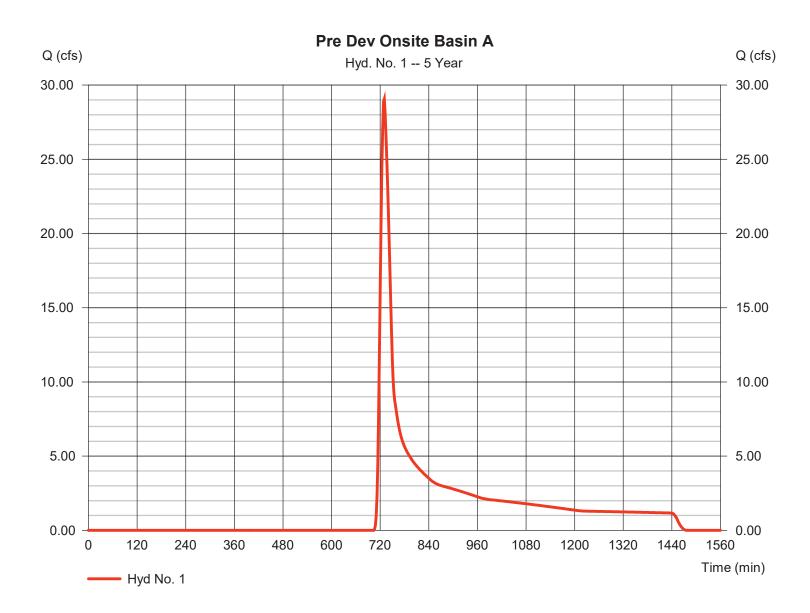
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

Friday, 01 / 31 / 2025

Hyd. No. 1

Pre Dev Onsite Basin A

Hydrograph type = SCS Runoff Peak discharge = 29.06 cfsStorm frequency = 5 yrsTime to peak = 730 min Time interval = 2 min Hyd. volume = 138,510 cuft Drainage area Curve number = 50.200 ac= 55 Hydraulic length Basin Slope = 0.0 %= 0 ftTc method Time of conc. (Tc) = 22.00 min = User Total precip. = 4.51 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



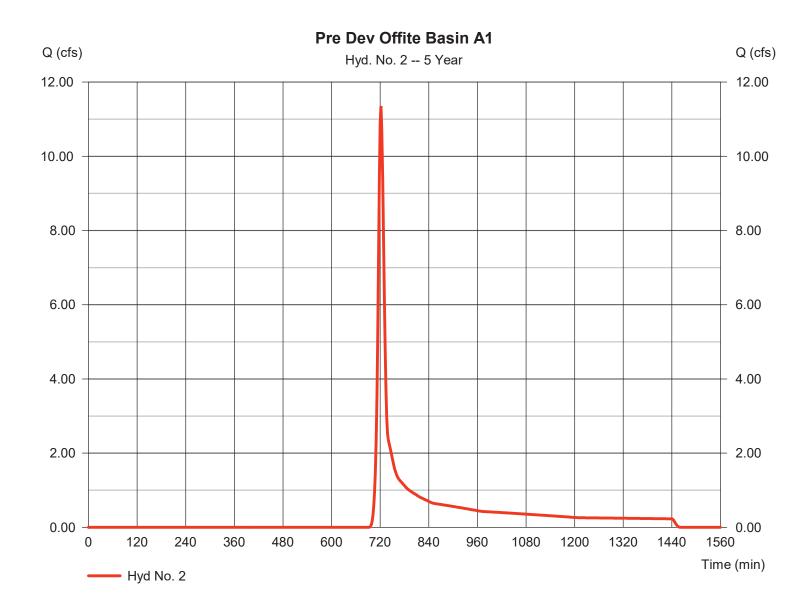
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Friday, 01 / 31 / 2025

Hyd. No. 2

Pre Dev Offite Basin A1

Hydrograph type = SCS Runoff Peak discharge = 11.35 cfsStorm frequency = 5 yrsTime to peak = 722 min Time interval = 2 min Hyd. volume = 32.123 cuft Drainage area Curve number = 7.910 ac= 61 Hydraulic length Basin Slope = 0.0 %= 0 ftTc method Time of conc. (Tc) = 12.00 min = User Total precip. = 4.51 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



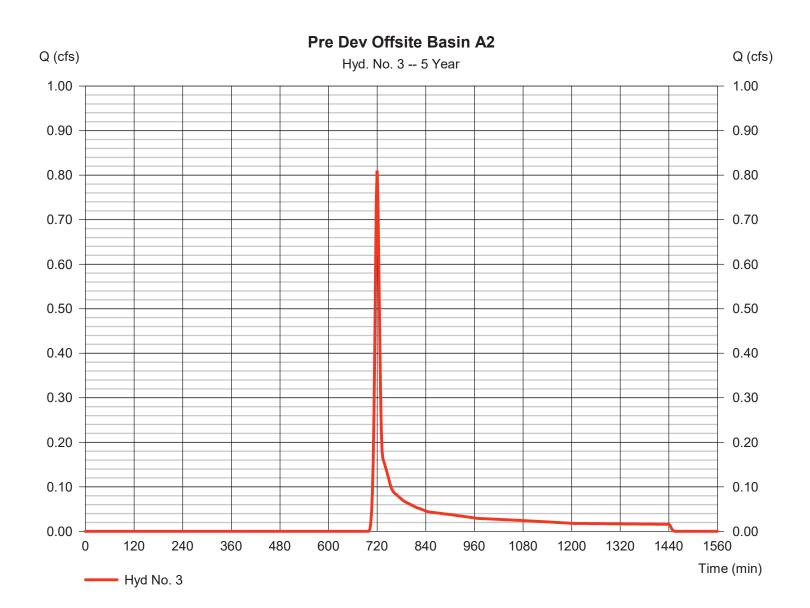
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

Friday, 01 / 31 / 2025

Hyd. No. 3

Pre Dev Offsite Basin A2

Hydrograph type = SCS Runoff Peak discharge = 0.810 cfsStorm frequency = 5 yrsTime to peak = 720 min Time interval = 2 min Hyd. volume = 2.048 cuft Drainage area = 0.620 acCurve number = 58 Hydraulic length Basin Slope = 0.0 %= 0 ftTc method Time of conc. (Tc) $= 8.00 \, \text{min}$ = User Total precip. = 4.51 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



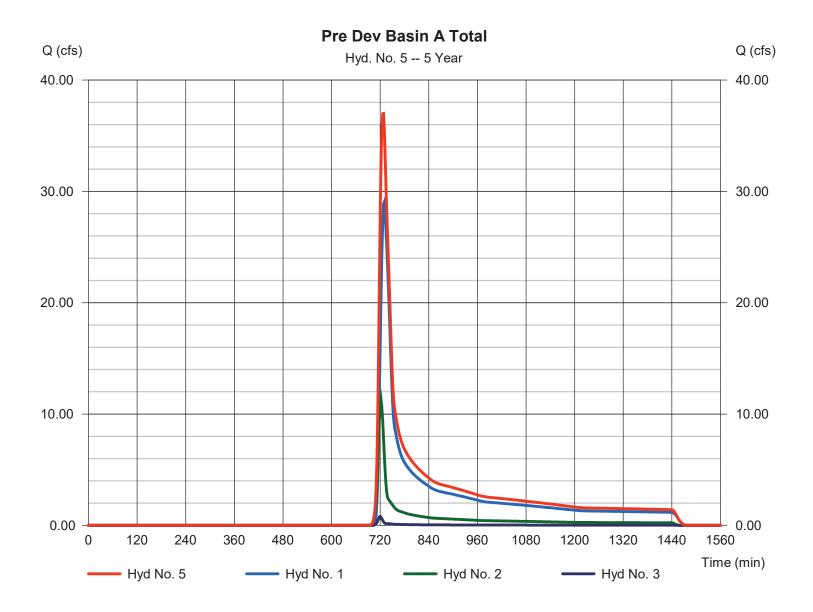
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

Friday, 01 / 31 / 2025

Hyd. No. 5

Pre Dev Basin A Total

Hydrograph type = Combine Peak discharge = 37.00 cfsTime to peak Storm frequency = 5 yrs= 728 min Time interval = 2 min Hyd. volume = 172,680 cuft Inflow hyds. = 1, 2, 3Contrib. drain. area = 58.730 ac



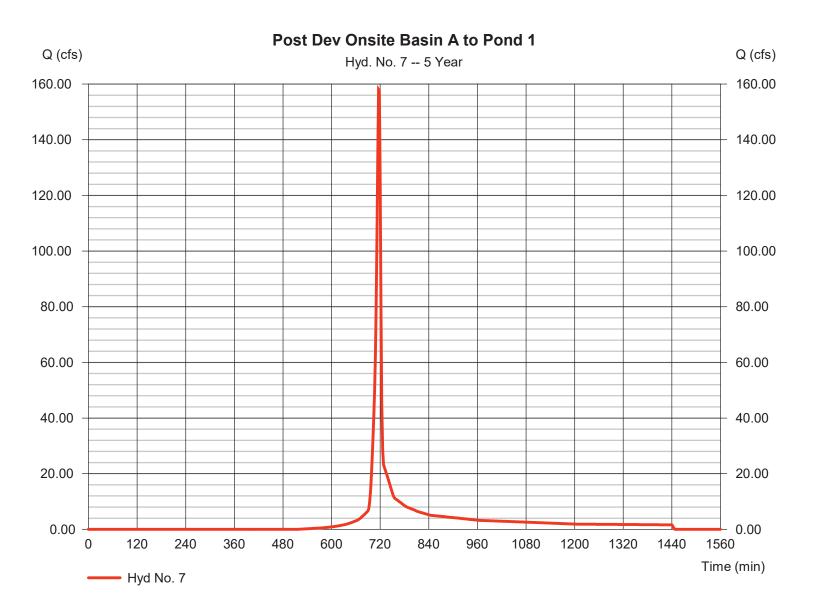
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Friday, 01 / 31 / 2025

Hyd. No. 7

Post Dev Onsite Basin A to Pond 1

Hydrograph type = SCS Runoff Peak discharge = 158.06 cfsStorm frequency = 5 yrsTime to peak = 716 min Time interval = 2 min Hyd. volume = 319,165 cuft Drainage area = 42.270 ac Curve number = 77 = 0 ftHydraulic length Basin Slope = 0.0 %Tc method Time of conc. (Tc) $= 5.00 \, \text{min}$ = User Total precip. = 4.51 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



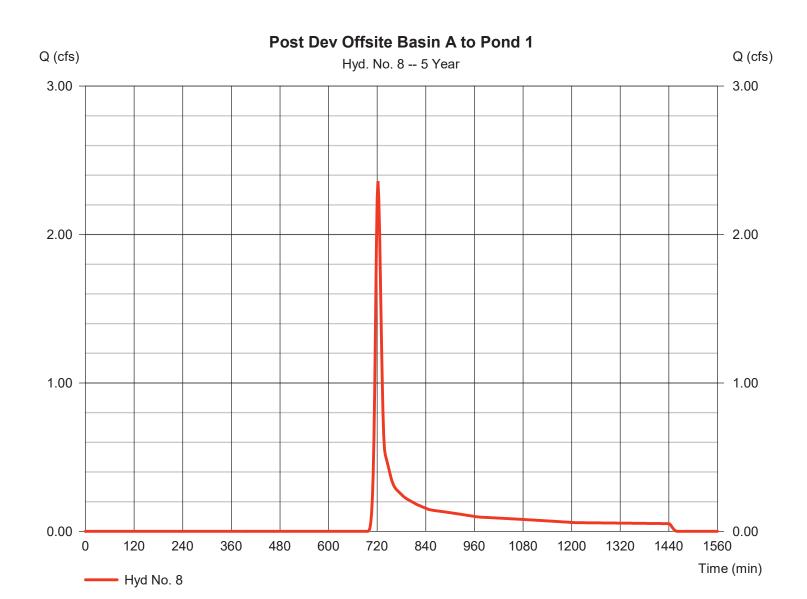
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Friday, 01 / 31 / 2025

Hyd. No. 8

Post Dev Offsite Basin A to Pond 1

= SCS Runoff Hydrograph type Peak discharge = 2.360 cfsStorm frequency = 5 yrsTime to peak = 722 min Time interval = 2 min Hyd. volume = 6,913 cuftDrainage area Curve number = 1.910 ac= 59 Basin Slope = 0.0 %Hydraulic length = 0 ftTc method Time of conc. (Tc) = 12.00 min = User Total precip. = 4.51 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



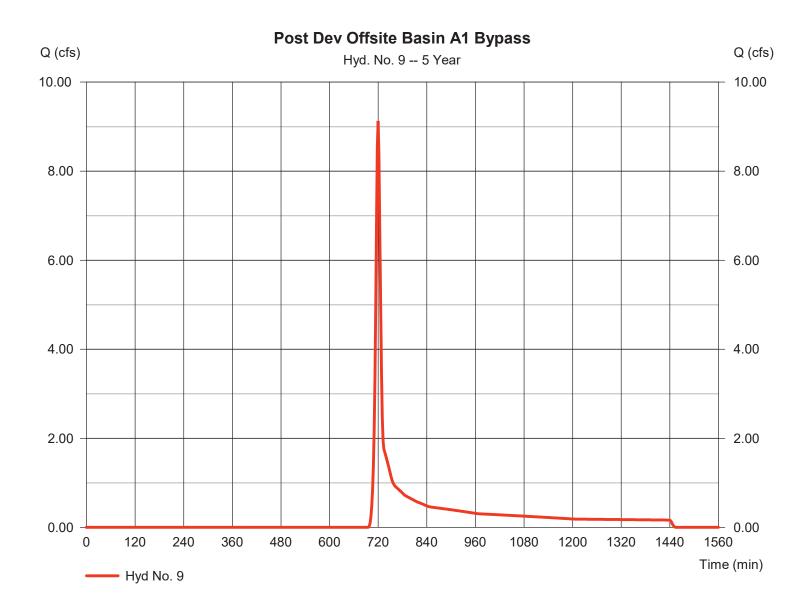
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Friday, 01 / 31 / 2025

Hyd. No. 9

Post Dev Offsite Basin A1 Bypass

Hydrograph type	= SCS Runoff	Peak discharge	= 9.128 cfs
Storm frequency	= 5 yrs	Time to peak	= 720 min
Time interval	= 2 min	Hyd. volume	= 22,328 cuft
Drainage area	= 6.000 ac	Curve number	= 60
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= User	Time of conc. (Tc)	= 8.00 min
Total precip.	= 4.51 in	Distribution	= Type II
Storm duration	= 24 hrs	Shape factor	= 484



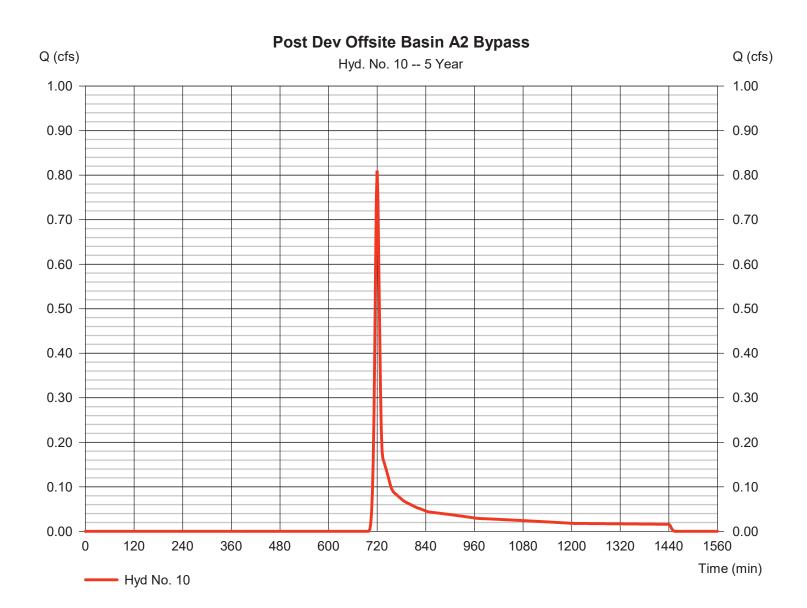
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Friday, 01 / 31 / 2025

Hyd. No. 10

Post Dev Offsite Basin A2 Bypass

Hydrograph type = SCS Runoff Peak discharge = 0.810 cfsStorm frequency = 5 yrsTime to peak = 720 min Time interval = 2 min Hyd. volume = 2.048 cuft Drainage area = 0.620 acCurve number = 58 Hydraulic length Basin Slope = 0.0 %= 0 ftTc method Time of conc. (Tc) = 8.00 min = User Total precip. = 4.51 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



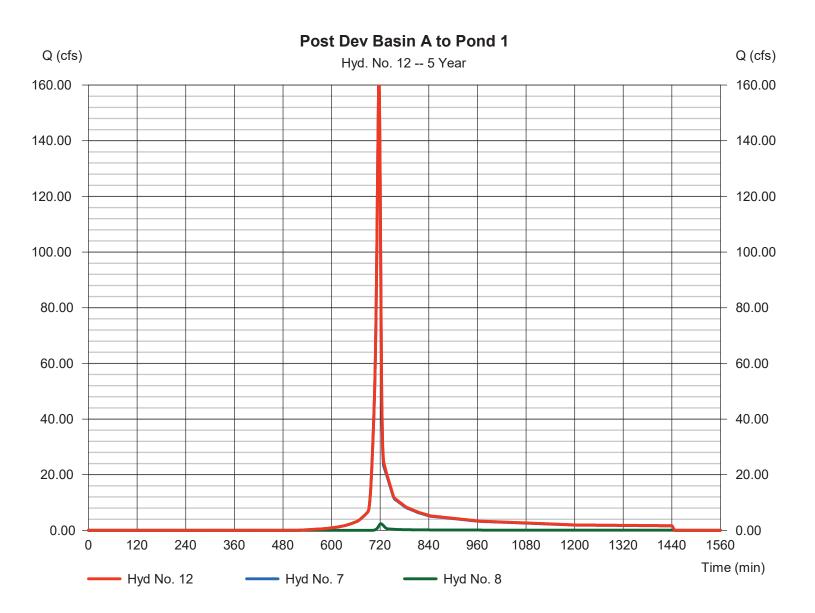
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Friday, 01 / 31 / 2025

Hyd. No. 12

Post Dev Basin A to Pond 1

Hydrograph type = Combine Peak discharge = 159.50 cfsStorm frequency Time to peak = 5 yrs= 718 min Time interval = 2 min Hyd. volume = 326,078 cuft Inflow hyds. = 7,8 Contrib. drain. area = 44.180 ac



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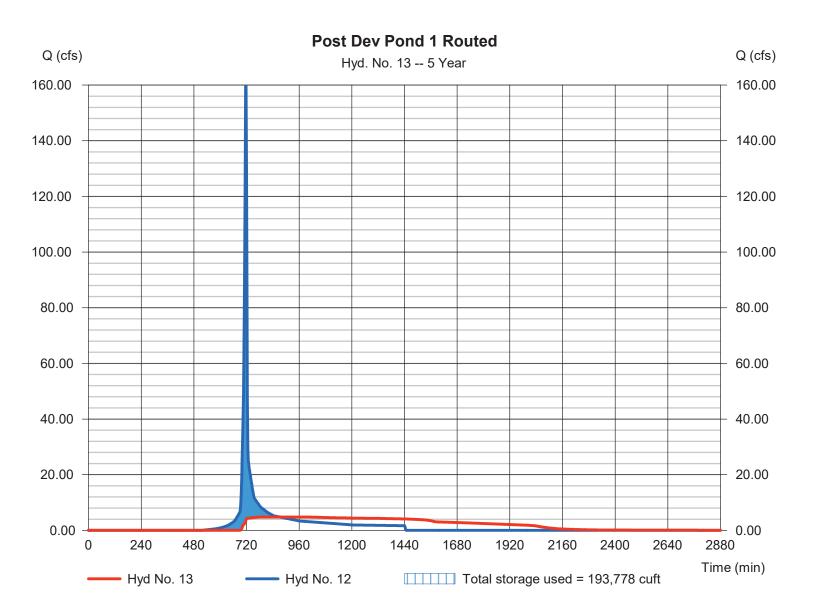
Friday, 01 / 31 / 2025

Hyd. No. 13

Post Dev Pond 1 Routed

Hydrograph type Peak discharge = 4.804 cfs= Reservoir Storm frequency Time to peak = 5 yrs= 870 min Time interval = 2 min Hyd. volume = 310,107 cuft Inflow hyd. No. = 12 - Post Dev Basin A to PondMax. Elevation = 1059.66 ftReservoir name = Pond 1 Max. Storage = 193,778 cuft

Storage Indication method used.



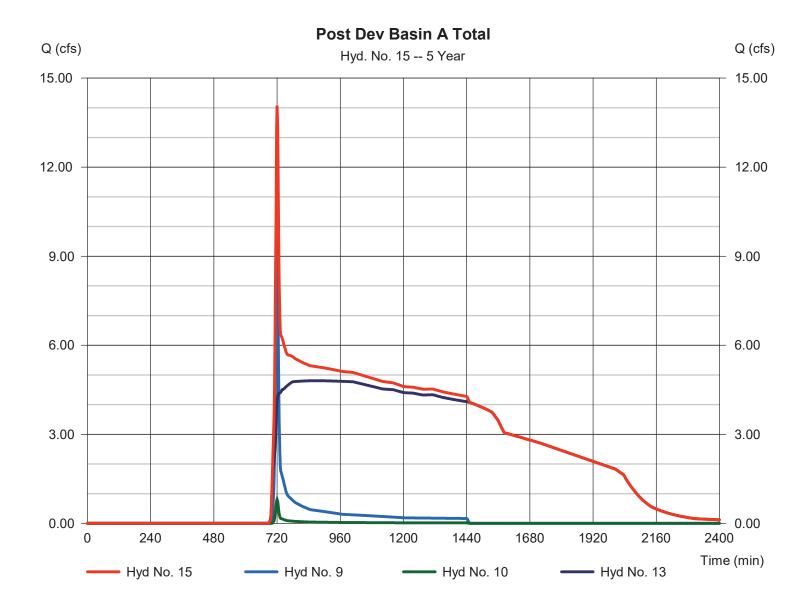
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Friday, 01 / 31 / 2025

Hyd. No. 15

Post Dev Basin A Total

Hydrograph type = Combine Peak discharge = 14.07 cfsTime to peak Storm frequency = 5 yrs= 720 min Time interval = 2 min Hyd. volume = 334,483 cuft Inflow hyds. = 9, 10, 13Contrib. drain. area = 6.620 ac



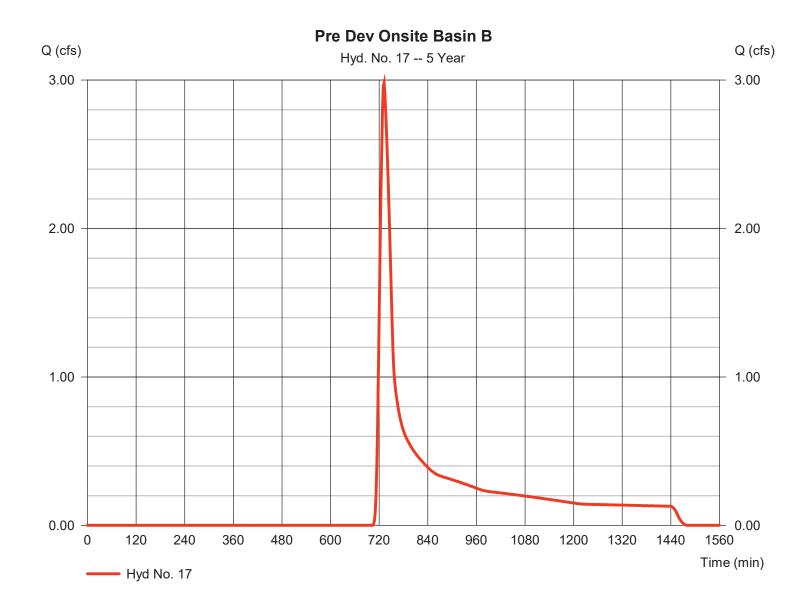
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Friday, 01 / 31 / 2025

Hyd. No. 17

Pre Dev Onsite Basin B

Hydrograph type = SCS Runoff Peak discharge = 2.983 cfsStorm frequency = 5 yrsTime to peak = 732 min Time interval = 2 min Hyd. volume = 15,210 cuftDrainage area Curve number = 5.700 ac= 55 Basin Slope = 0.0 %Hydraulic length = 0 ftTc method Time of conc. (Tc) = 25.00 min = User Total precip. = 4.51 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



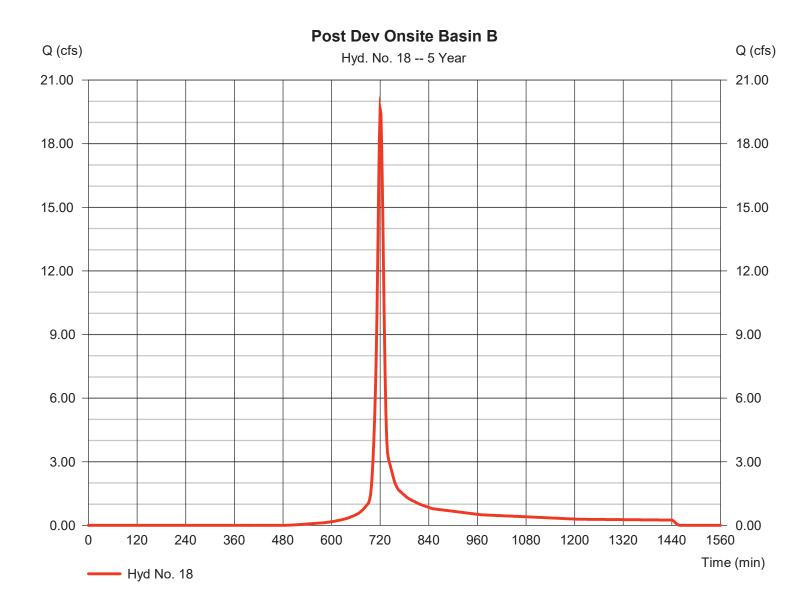
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Friday, 01 / 31 / 2025

Hyd. No. 18

Post Dev Onsite Basin B

Hydrograph type = SCS Runoff Peak discharge = 19.63 cfsStorm frequency = 5 yrsTime to peak = 720 min Time interval = 2 min Hyd. volume = 50.887 cuft Drainage area = 5.700 acCurve number = 79 Hydraulic length Basin Slope = 0.0 %= 0 ftTc method Time of conc. (Tc) = 10.00 min = User Total precip. = 4.51 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

= 4.51 in

= 24 hrs

Friday, 01 / 31 / 2025

= Type II

= 484

Hyd. No. 20

Total precip.

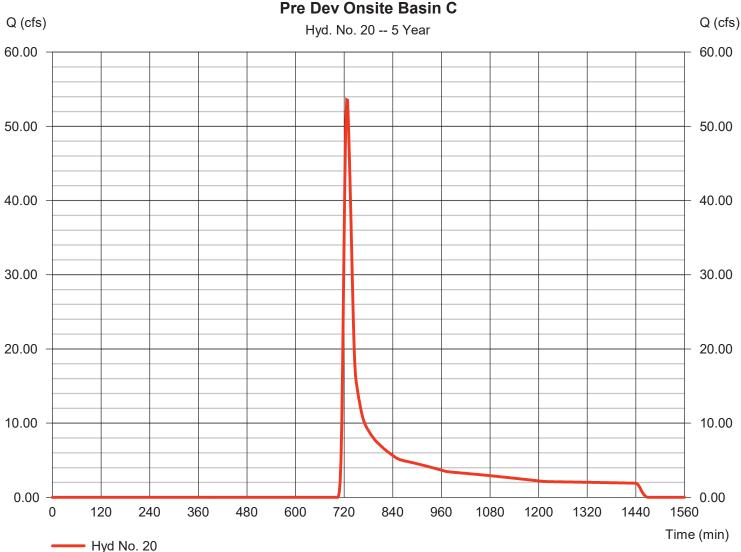
Storm duration

Pre Dev Onsite Basin C

Hydrograph type = SCS Runoff Peak discharge = 53.58 cfsStorm frequency = 5 yrsTime to peak = 726 min Time interval = 2 min Hyd. volume = 226,700 cuftDrainage area Curve number = 83.630 ac = 55 Basin Slope = 0.0 %Hydraulic length = 0 ftTc method Time of conc. (Tc) = 17.00 min = User

Distribution

Shape factor



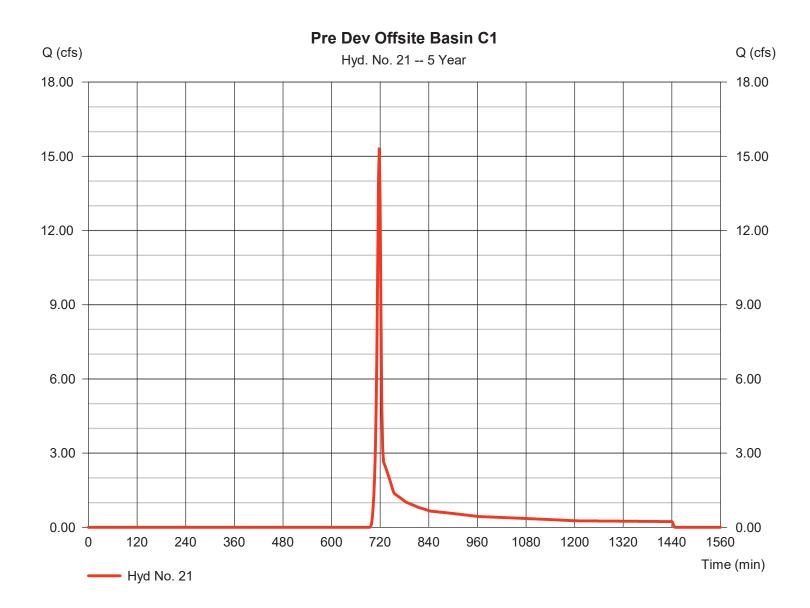
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

Friday, 01 / 31 / 2025

Hyd. No. 21

Pre Dev Offsite Basin C1

Hydrograph type = SCS Runoff Peak discharge = 15.35 cfsStorm frequency = 5 yrsTime to peak = 718 min = 31,853 cuft Time interval = 2 min Hyd. volume Drainage area = 9.130 acCurve number = 60 Hydraulic length Basin Slope = 0.0 %= 0 ftTc method Time of conc. (Tc) = 6.00 min = User Total precip. = 4.51 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

= 24 hrs

Friday, 01 / 31 / 2025

= 484

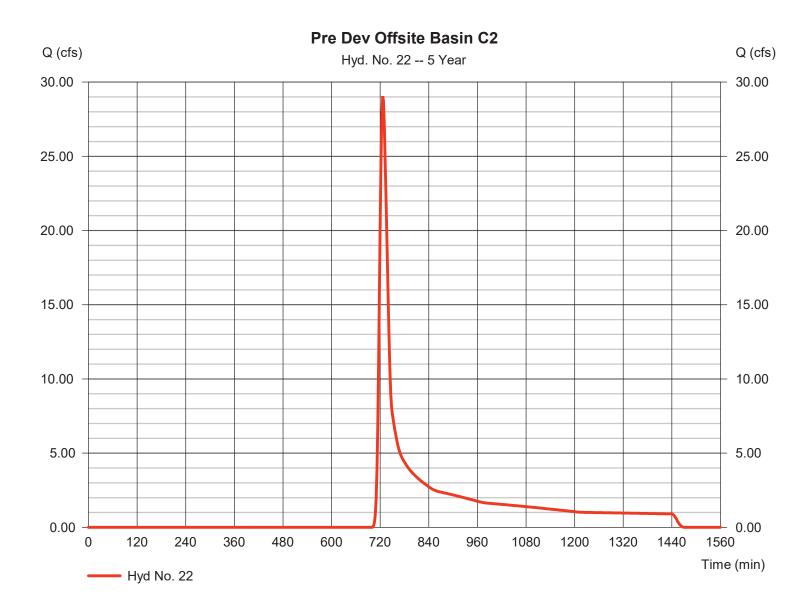
Hyd. No. 22

Storm duration

Pre Dev Offsite Basin C2

Hydrograph type = SCS Runoff Peak discharge = 29.06 cfsStorm frequency = 5 yrsTime to peak = 726 min Time interval = 2 min Hyd. volume = 113,856 cuft = 36.720 ac Drainage area Curve number = 57 Hydraulic length Basin Slope = 0.0 %= 0 ftTc method Time of conc. (Tc) = 18.00 min = User Total precip. = 4.51 inDistribution = Type II

Shape factor



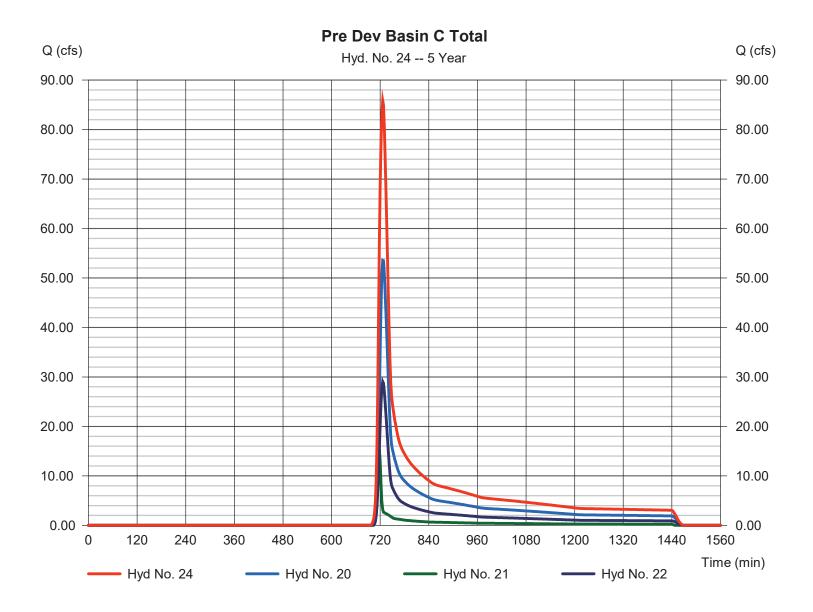
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Friday, 01 / 31 / 2025

Hyd. No. 24

Pre Dev Basin C Total

Hydrograph type = Combine Peak discharge = 85.81 cfsStorm frequency Time to peak = 5 yrs= 726 min Time interval = 2 min Hyd. volume = 372,409 cuft Inflow hyds. = 20, 21, 22 Contrib. drain. area = 129.480 ac



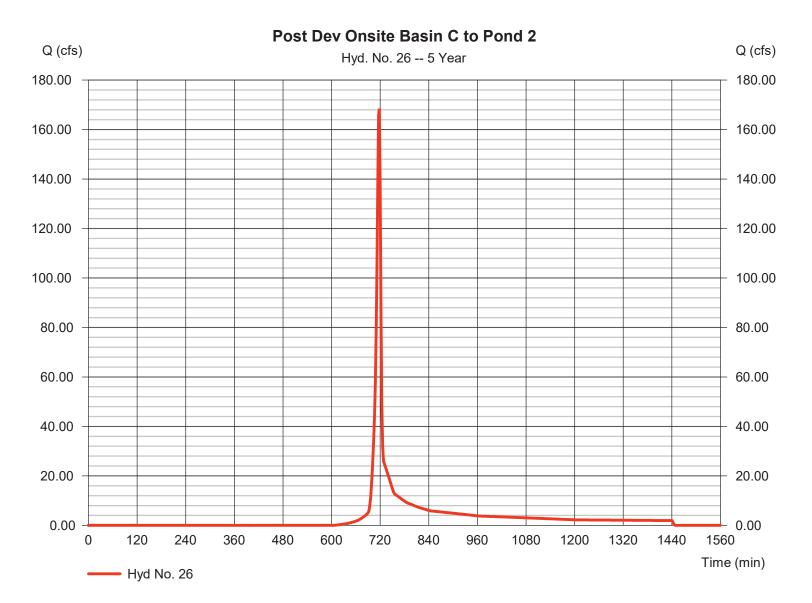
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Friday, 01 / 31 / 2025

Hyd. No. 26

Post Dev Onsite Basin C to Pond 2

Hydrograph type = SCS Runoff Peak discharge = 168.45 cfsStorm frequency Time to peak = 5 yrs= 718 min Time interval = 2 min Hyd. volume = 337,001 cuft Drainage area = 56.470 ac Curve number = 71 = 0 ftHydraulic length Basin Slope = 0.0 %Tc method Time of conc. (Tc) $= 5.00 \, \text{min}$ = User Total precip. = 4.51 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



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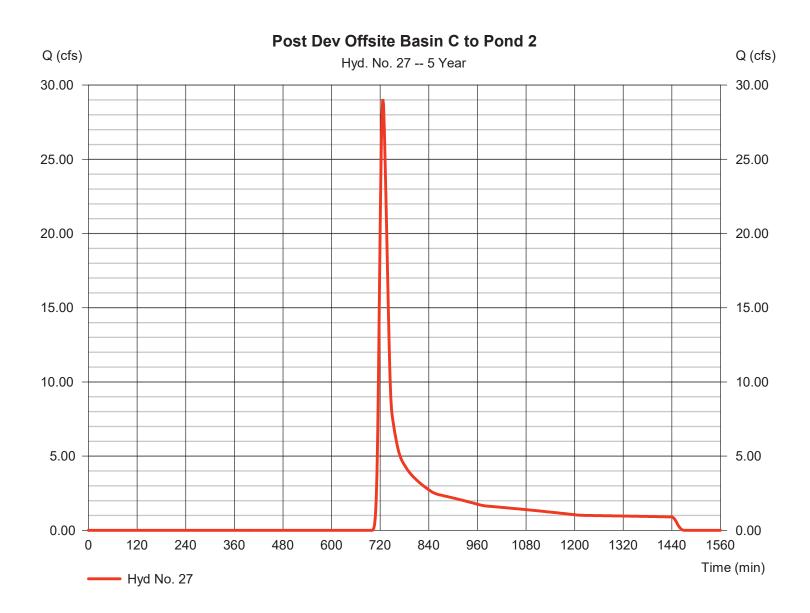
Friday, 01 / 31 / 2025

Hyd. No. 27

Post Dev Offsite Basin C to Pond 2

Hydrograph type = SCS Runoff Peak discharge = 29.06 cfsStorm frequency = 5 yrsTime to peak = 726 min Time interval = 2 min Hyd. volume = 113,856 cuft Drainage area = 36.720 ac Curve number = 57 Hydraulic length Basin Slope = 0.0 %= 0 ftTc method Time of conc. (Tc) = 18.00 min = User

Total precip. = 4.51 in Distribution = Type II
Storm duration = 24 hrs Shape factor = 484



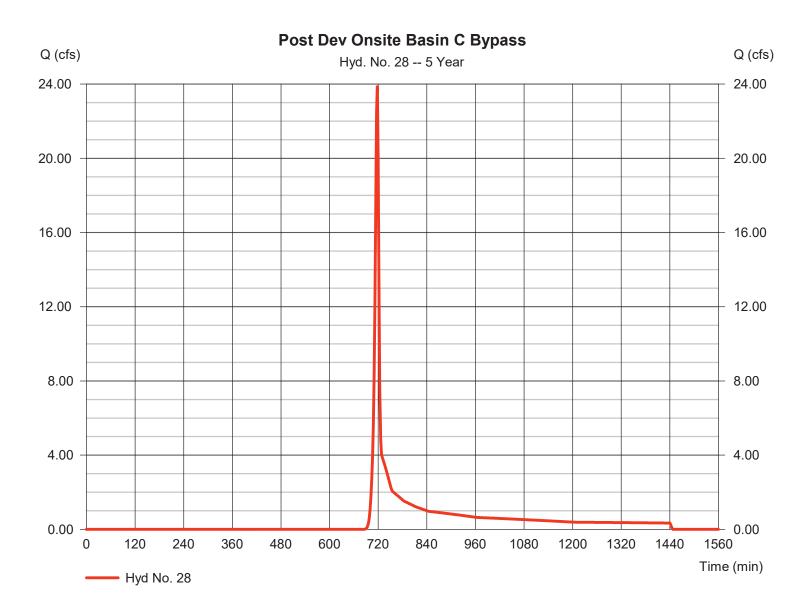
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Friday, 01 / 31 / 2025

Hyd. No. 28

Post Dev Onsite Basin C Bypass

Hydrograph type = SCS Runoff Peak discharge = 23.92 cfsStorm frequency = 5 yrsTime to peak = 718 min Time interval = 2 min Hyd. volume = 48.859 cuft Drainage area = 12.530 ac Curve number = 62 Hydraulic length Basin Slope = 0.0 %= 0 ftTc method Time of conc. (Tc) = 5.00 min = User Total precip. = 4.51 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



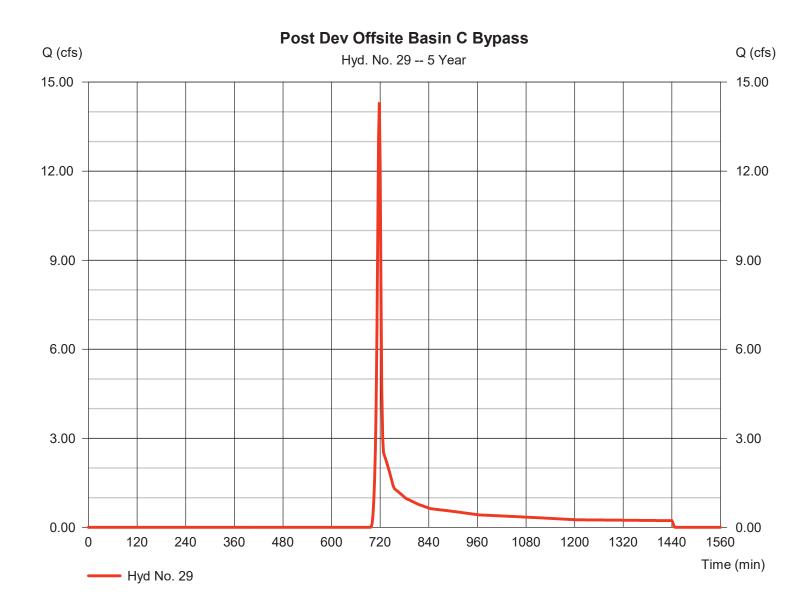
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Friday, 01 / 31 / 2025

Hyd. No. 29

Post Dev Offsite Basin C Bypass

Hydrograph type = SCS Runoff Peak discharge = 14.32 cfsStorm frequency = 5 yrsTime to peak = 718 min Time interval = 2 min Hyd. volume = 30.040 cuftDrainage area = 9.130 acCurve number = 59 Hydraulic length Basin Slope = 0.0 %= 0 ftTc method Time of conc. (Tc) = 6.00 min = User Total precip. = 4.51 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



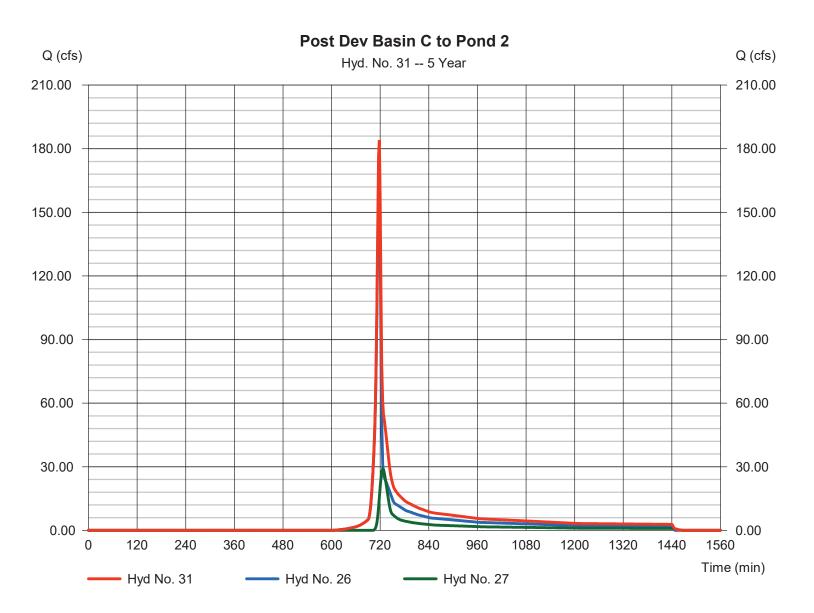
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Friday, 01 / 31 / 2025

Hyd. No. 31

Post Dev Basin C to Pond 2

Hydrograph type = Combine Peak discharge = 184.02 cfsStorm frequency Time to peak = 5 yrs= 718 min Time interval = 2 min Hyd. volume = 450,857 cuft Inflow hyds. = 26, 27Contrib. drain. area = 93.190 ac



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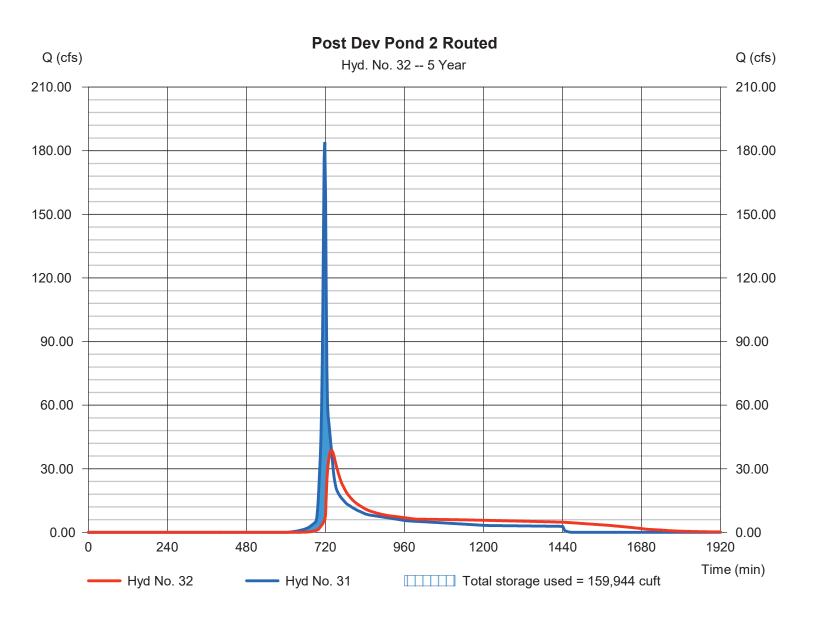
Friday, 01 / 31 / 2025

Hyd. No. 32

Post Dev Pond 2 Routed

Hydrograph type Peak discharge = 38.65 cfs= Reservoir Storm frequency = 5 yrsTime to peak = 738 min Time interval = 2 min Hyd. volume = 450,839 cuft Inflow hyd. No. = 31 - Post Dev Basin C to PondMax. Elevation = 1108.53 ft= Pond 2 Max. Storage Reservoir name = 159,944 cuft

Storage Indication method used.



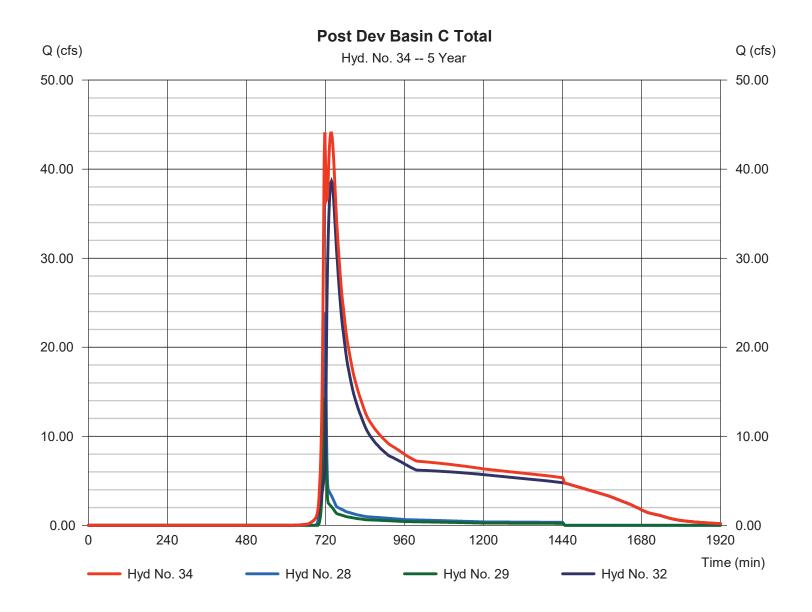
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Friday, 01 / 31 / 2025

Hyd. No. 34

Post Dev Basin C Total

Hydrograph type = Combine Peak discharge = 44.14 cfsStorm frequency = 5 yrsTime to peak = 718 min Time interval = 2 min Hyd. volume = 529,738 cuft Inflow hyds. = 28, 29, 32 Contrib. drain. area = 21.660 ac



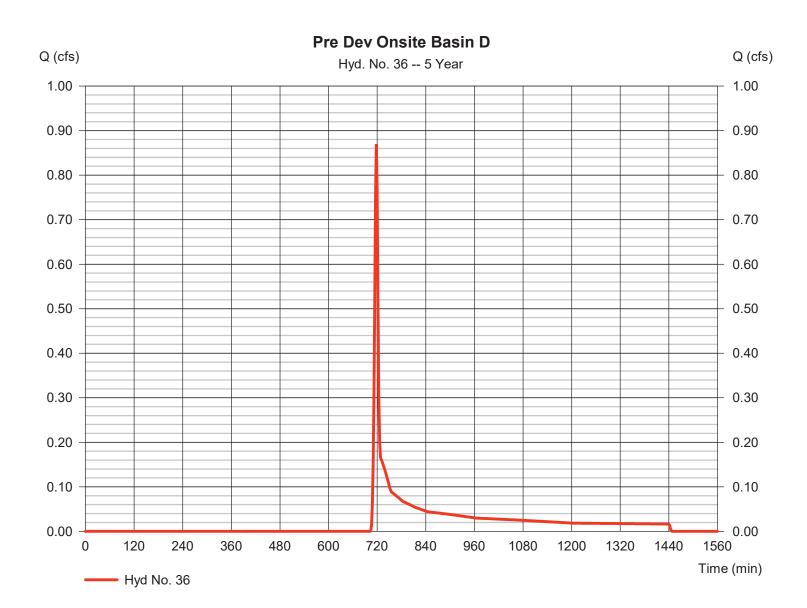
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Friday, 01 / 31 / 2025

Hyd. No. 36

Pre Dev Onsite Basin D

Hydrograph type = SCS Runoff Peak discharge = 0.870 cfsStorm frequency = 5 yrsTime to peak = 718 min Time interval = 2 min Hyd. volume = 1.957 cuft Drainage area Curve number = 0.770 ac= 55 Hydraulic length Basin Slope = 0.0 %= 0 ftTc method Time of conc. (Tc) $= 5.00 \, \text{min}$ = User Total precip. = 4.51 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



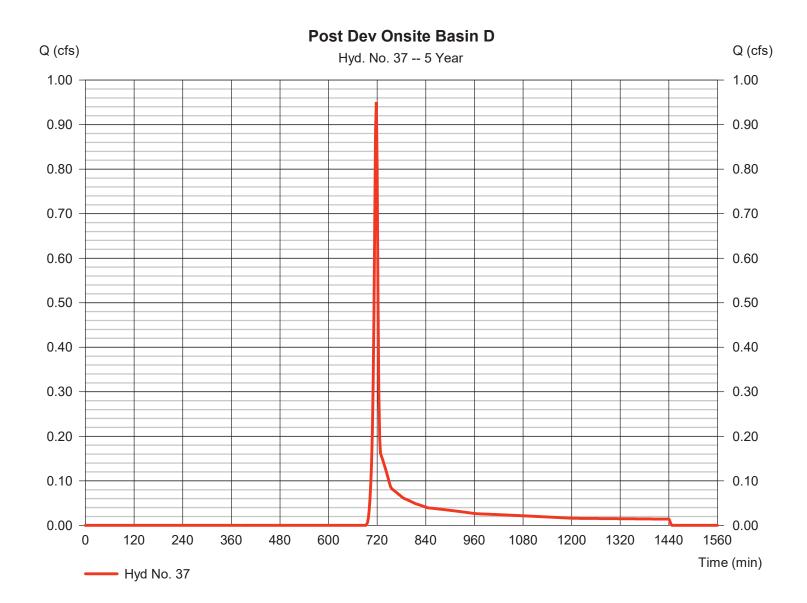
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Friday, 01 / 31 / 2025

Hyd. No. 37

Post Dev Onsite Basin D

Hydrograph type = SCS Runoff Peak discharge = 0.951 cfsStorm frequency = 5 yrsTime to peak = 718 min Time interval = 2 min Hyd. volume = 1.957 cuft Drainage area = 0.530 acCurve number = 61 Hydraulic length Basin Slope = 0.0 %= 0 ftTc method Time of conc. (Tc) = 5.00 min = User Total precip. = 4.51 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



Hydrograph Summary Report

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

Hyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description	
1	SCS Runoff	46.88	2	728	198,629				Pre Dev Onsite Basin A	
2	SCS Runoff	16.07	2	722	43,777				Pre Dev Offite Basin A1	
3	SCS Runoff	1.187	2	720	2,856				Pre Dev Offsite Basin A2	
5	Combine	58.61	2	726	245,263	1, 2, 3,			Pre Dev Basin A Total	
7	SCS Runoff	197.16	2	716	399,080				Post Dev Onsite Basin A to Pond 1	
8	SCS Runoff	3.440	2	722	9,564				Post Dev Offsite Basin A to Pond 1	
9	SCS Runoff	12.97	2	720	30,654				Post Dev Offsite Basin A1 Bypass	
10	SCS Runoff	1.187	2	720	2,856				Post Dev Offsite Basin A2 Bypass	
12	Combine	199.44	2	716	408,644	7, 8,			Post Dev Basin A to Pond 1	
13	Reservoir	13.61	2	758	392,673	12	1060.52	221,174	Post Dev Pond 1 Routed	
15	Combine	18.67	2	720	426,183	9, 10, 13,			Post Dev Basin A Total	
17	SCS Runoff	4.808	2	730	21,812				Pre Dev Onsite Basin B	
18	SCS Runoff	24.32	2	720	63,100				Post Dev Onsite Basin B	
20	SCS Runoff	86.56	2	726	325,098				Pre Dev Onsite Basin C	
21	SCS Runoff	21.56	2	718	43,730				Pre Dev Offsite Basin C1	
22	SCS Runoff	44.67	2	726	160,184				Pre Dev Offsite Basin C2	
24	Combine	135.52	2	726	529,012	20, 21, 22,			Pre Dev Basin C Total	
26	SCS Runoff	215.68	2	718	433,064				Post Dev Onsite Basin C to Pond 2	
27	SCS Runoff	44.67	2	726	160,184				Post Dev Offsite Basin C to Pond 2	
28	SCS Runoff	32.86	2	718	66,122				Post Dev Onsite Basin C Bypass	
29	SCS Runoff	20.39	2	718	41,561				Post Dev Offsite Basin C Bypass	
31	Combine	242.62	2	718	593,249	26, 27,			Post Dev Basin C to Pond 2	
32	Reservoir	73.86	2	730	593,229	31	1109.51	191,818	Post Dev Pond 2 Routed	
34	Combine	90.60	2	722	700,912	28, 29, 32,			Post Dev Basin C Total	
36	SCS Runoff	1.328	2	718	2,806				Pre Dev Onsite Basin D	
37	SCS Runoff	1.321	2	718	2,667				Post Dev Onsite Basin D	
1220263 Hydrology Concept.gpw					Return F	Return Period: 10 Year			Friday, 01 / 31 / 2025	

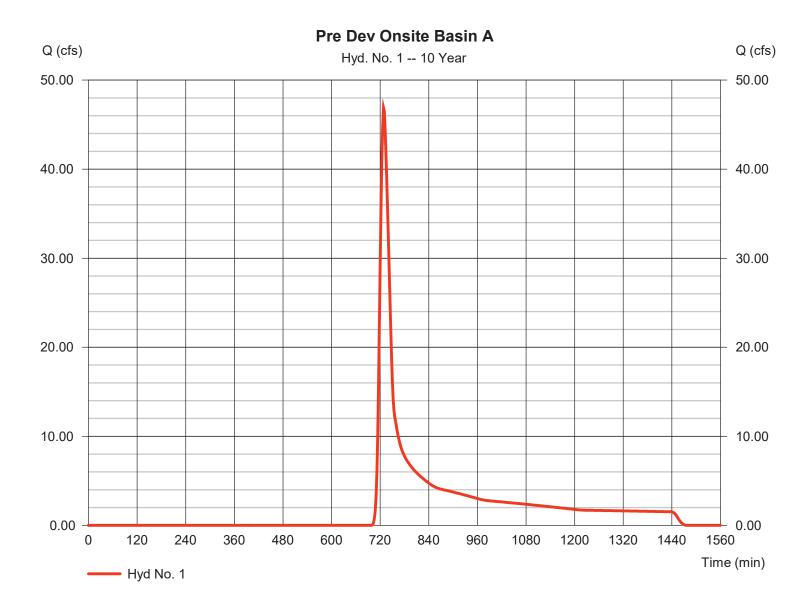
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Friday, 01 / 31 / 2025

Hyd. No. 1

Pre Dev Onsite Basin A

Hydrograph type = SCS Runoff Peak discharge = 46.88 cfsStorm frequency = 10 yrsTime to peak = 728 min Time interval = 2 min Hyd. volume = 198,629 cuft Drainage area Curve number = 50.200 ac= 55 Basin Slope = 0.0 %Hydraulic length = 0 ftTc method Time of conc. (Tc) = 22.00 min = User Total precip. = 5.18 inDistribution = Type II Shape factor Storm duration = 24 hrs = 484



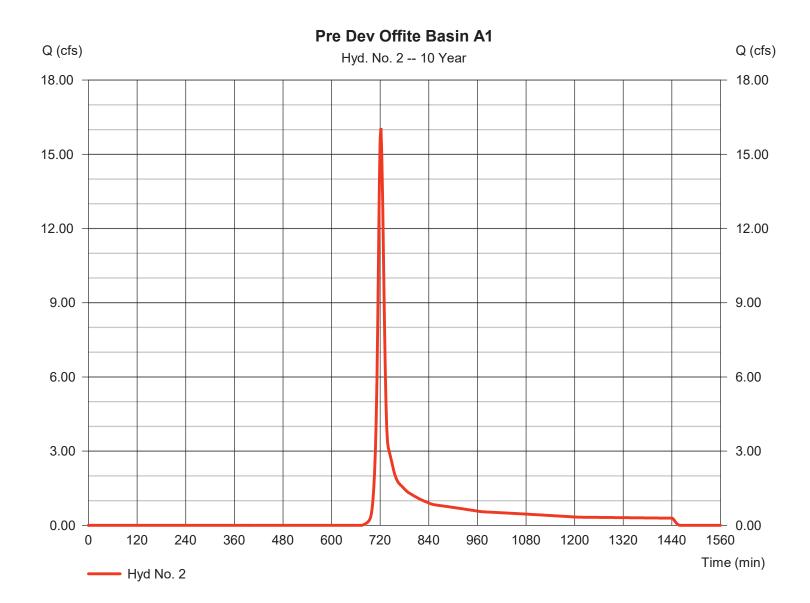
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Friday, 01 / 31 / 2025

Hyd. No. 2

Pre Dev Offite Basin A1

Hydrograph type = SCS Runoff Peak discharge = 16.07 cfsStorm frequency = 10 yrsTime to peak = 722 min Time interval = 2 min Hyd. volume = 43,777 cuft Drainage area = 7.910 acCurve number = 61 = 0.0 % Hydraulic length Basin Slope = 0 ftTc method Time of conc. (Tc) = 12.00 min = User Total precip. = 5.18 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



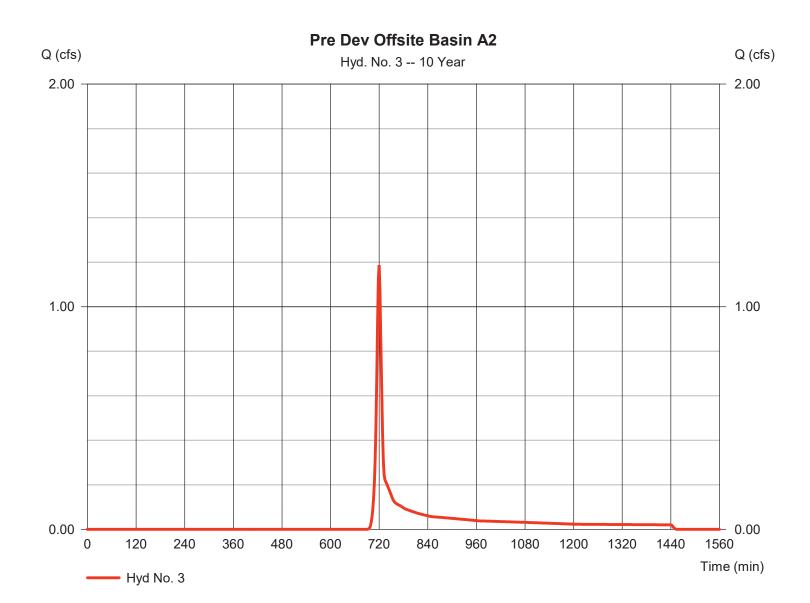
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Friday, 01 / 31 / 2025

Hyd. No. 3

Pre Dev Offsite Basin A2

Hydrograph type = SCS Runoff Peak discharge = 1.187 cfsStorm frequency = 10 yrsTime to peak = 720 min Time interval = 2 min Hyd. volume = 2,856 cuftDrainage area = 0.620 acCurve number = 58 Basin Slope = 0.0 %Hydraulic length = 0 ftTc method Time of conc. (Tc) = 8.00 min = User Total precip. = 5.18 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



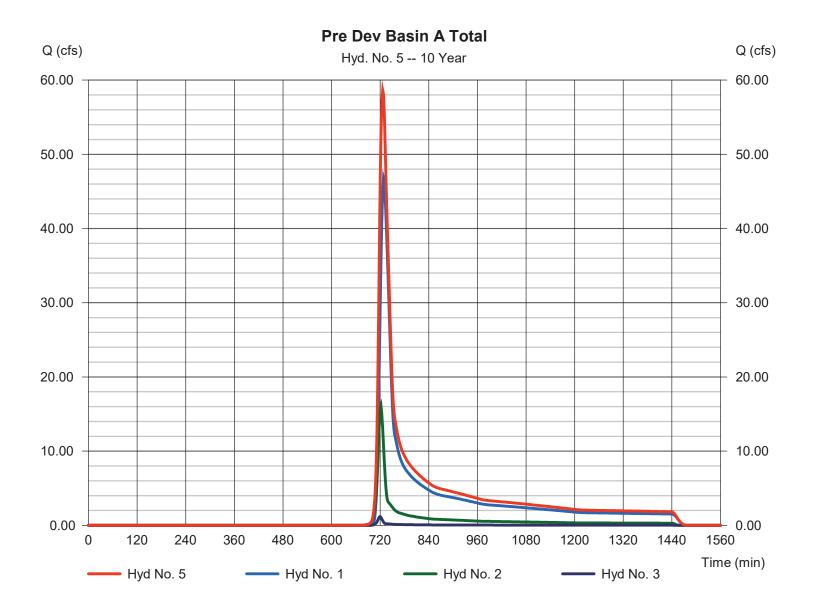
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Friday, 01 / 31 / 2025

Hyd. No. 5

Pre Dev Basin A Total

Hydrograph type = Combine Peak discharge = 58.61 cfsStorm frequency Time to peak = 10 yrs= 726 min Time interval = 2 min Hyd. volume = 245,263 cuft Inflow hyds. = 1, 2, 3Contrib. drain. area = 58.730 ac



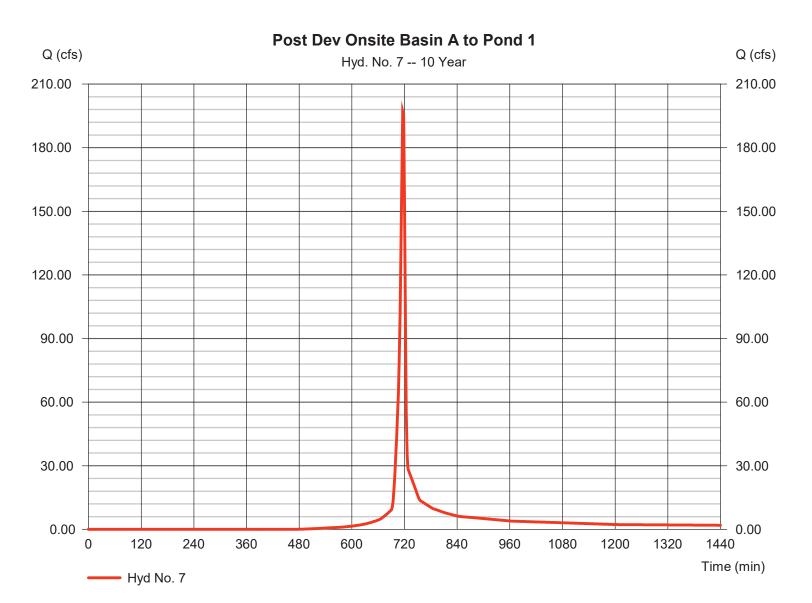
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Friday, 01 / 31 / 2025

Hyd. No. 7

Post Dev Onsite Basin A to Pond 1

Hydrograph type = SCS Runoff Peak discharge = 197.16 cfsStorm frequency Time to peak = 10 yrs= 716 min Time interval = 2 min Hyd. volume = 399,080 cuft Drainage area = 42.270 ac Curve number = 77 = 0 ftHydraulic length Basin Slope = 0.0 %Tc method Time of conc. (Tc) $= 5.00 \, \text{min}$ = User Total precip. = 5.18 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



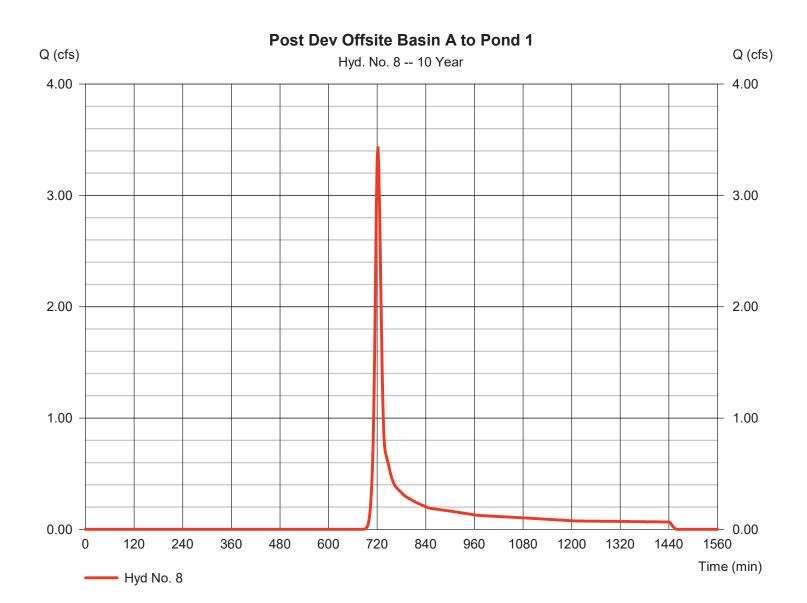
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Friday, 01 / 31 / 2025

Hyd. No. 8

Post Dev Offsite Basin A to Pond 1

Hydrograph type = SCS Runoff Peak discharge = 3.440 cfsStorm frequency = 10 yrsTime to peak = 722 min Time interval = 2 min Hyd. volume = 9.564 cuft Drainage area = 1.910 acCurve number = 59 Hydraulic length Basin Slope = 0.0 %= 0 ftTc method Time of conc. (Tc) = 12.00 min = User Total precip. = 5.18 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



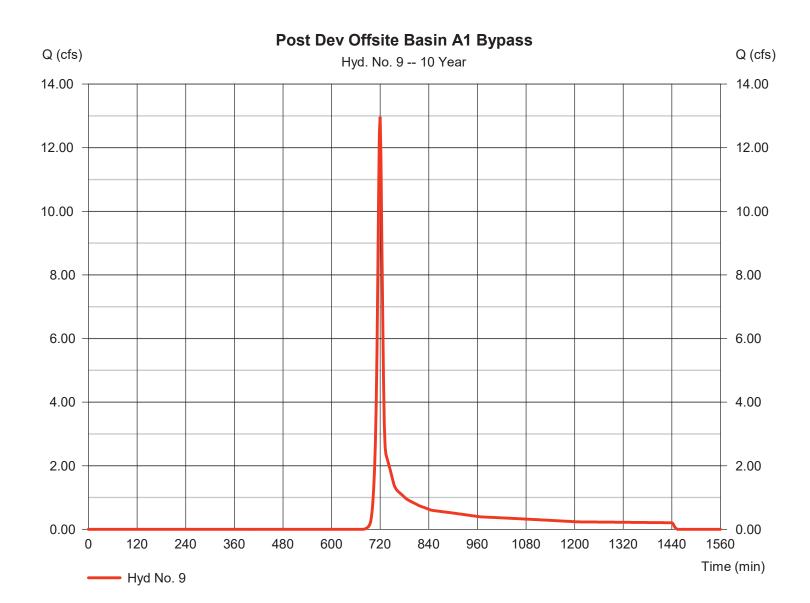
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Friday, 01 / 31 / 2025

Hyd. No. 9

Post Dev Offsite Basin A1 Bypass

Hydrograph type = SCS Runoff Peak discharge = 12.97 cfsStorm frequency = 10 yrsTime to peak = 720 min Time interval = 2 min Hyd. volume = 30.654 cuft Drainage area Curve number = 6.000 ac= 60 Hydraulic length Basin Slope = 0.0 %= 0 ftTc method Time of conc. (Tc) $= 8.00 \, \text{min}$ = User Total precip. = 5.18 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



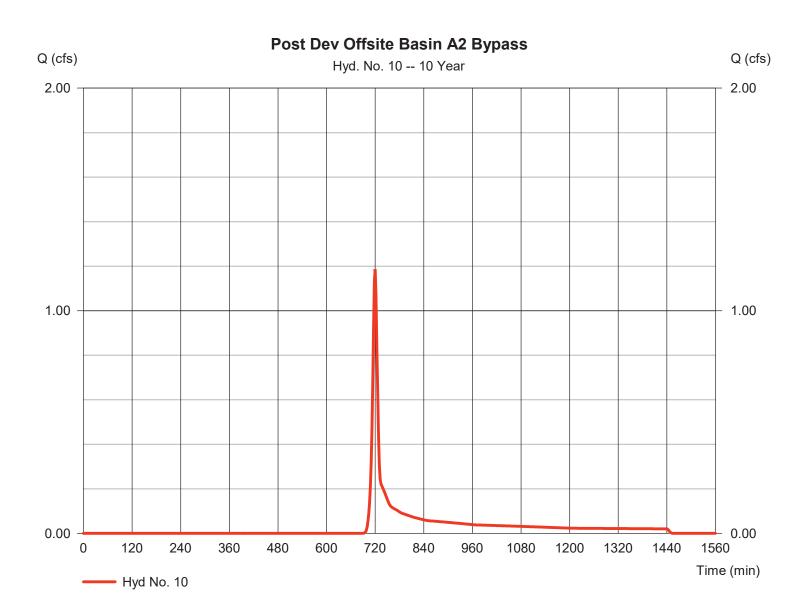
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Friday, 01 / 31 / 2025

Hyd. No. 10

Post Dev Offsite Basin A2 Bypass

= SCS Runoff	Peak discharge	= 1.187 cfs
= 10 yrs	Time to peak	= 720 min
= 2 min	Hyd. volume	= 2,856 cuft
= 0.620 ac	Curve number	= 58
= 0.0 %	Hydraulic length	= 0 ft
= User	Time of conc. (Tc)	= 8.00 min
= 5.18 in	Distribution	= Type II
= 24 hrs	Shape factor	= 484
	= 10 yrs = 2 min = 0.620 ac = 0.0 % = User = 5.18 in	= 10 yrs = 2 min = 0.620 ac = 0.0 % = User = 5.18 in Time to peak Hyd. volume Curve number Hydraulic length Time of conc. (Tc) Distribution



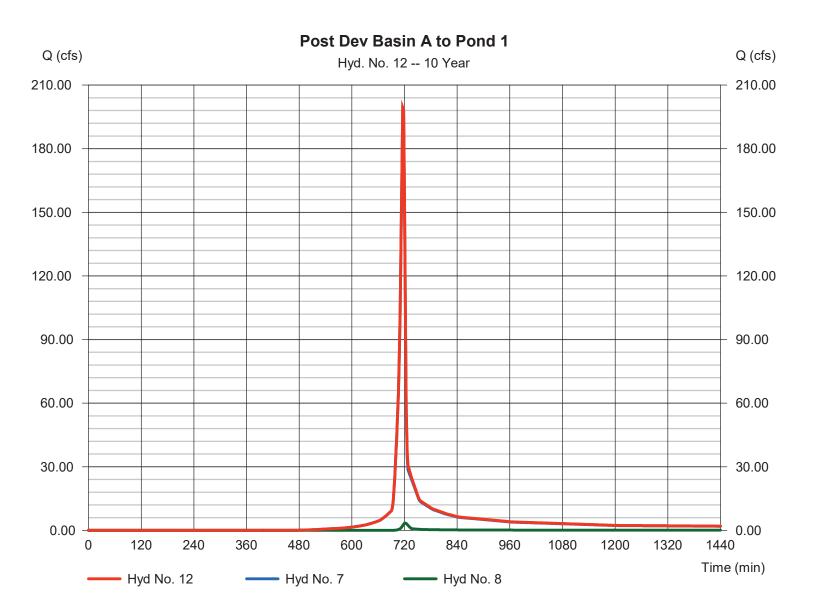
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Friday, 01 / 31 / 2025

Hyd. No. 12

Post Dev Basin A to Pond 1

Hydrograph type = Combine Peak discharge = 199.44 cfsStorm frequency Time to peak = 10 yrs= 716 min Time interval = 2 min Hyd. volume = 408,644 cuft Inflow hyds. = 7,8 Contrib. drain. area = 44.180 ac



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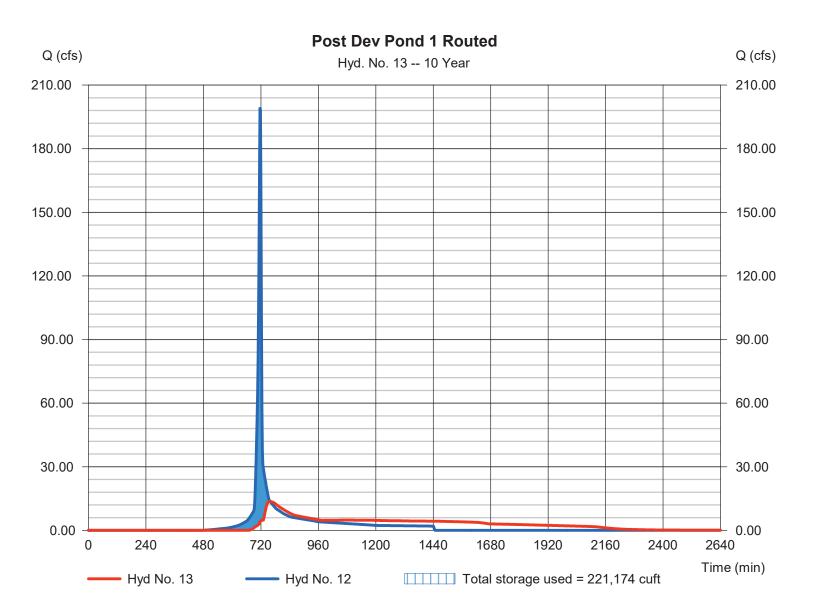
Friday, 01 / 31 / 2025

Hyd. No. 13

Post Dev Pond 1 Routed

Hydrograph type Peak discharge = 13.61 cfs= Reservoir Storm frequency = 10 yrsTime to peak = 758 min Time interval = 2 min Hyd. volume = 392,673 cuft Inflow hyd. No. = 12 - Post Dev Basin A to PondMax. Elevation = 1060.52 ft= Pond 1 Max. Storage Reservoir name = 221,174 cuft

Storage Indication method used.



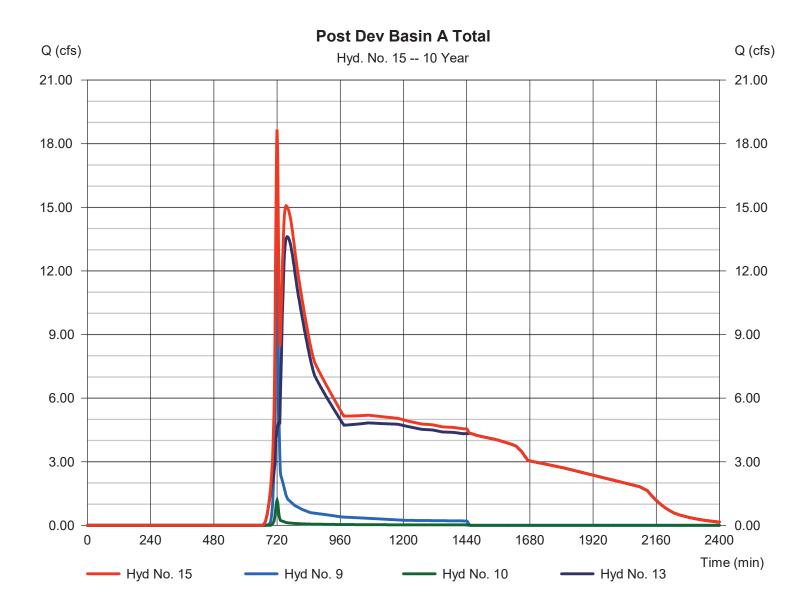
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Friday, 01 / 31 / 2025

Hyd. No. 15

Post Dev Basin A Total

Hydrograph type = Combine Peak discharge = 18.67 cfsStorm frequency Time to peak = 10 yrs= 720 min Time interval = 2 min Hyd. volume = 426,183 cuft Inflow hyds. = 9, 10, 13Contrib. drain. area = 6.620 ac



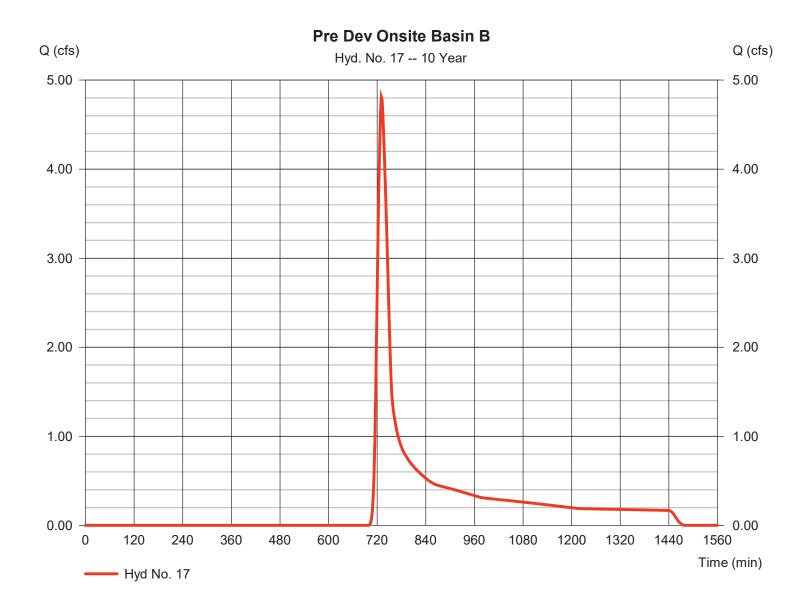
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Friday, 01 / 31 / 2025

Hyd. No. 17

Pre Dev Onsite Basin B

Hydrograph type = SCS Runoff Peak discharge = 4.808 cfsStorm frequency = 10 yrsTime to peak = 730 min Time interval = 2 min Hyd. volume = 21.812 cuft Drainage area Curve number = 5.700 ac= 55 Basin Slope = 0.0 %Hydraulic length = 0 ftTc method Time of conc. (Tc) = 25.00 min = User Total precip. = 5.18 inDistribution = Type II Shape factor Storm duration = 24 hrs = 484



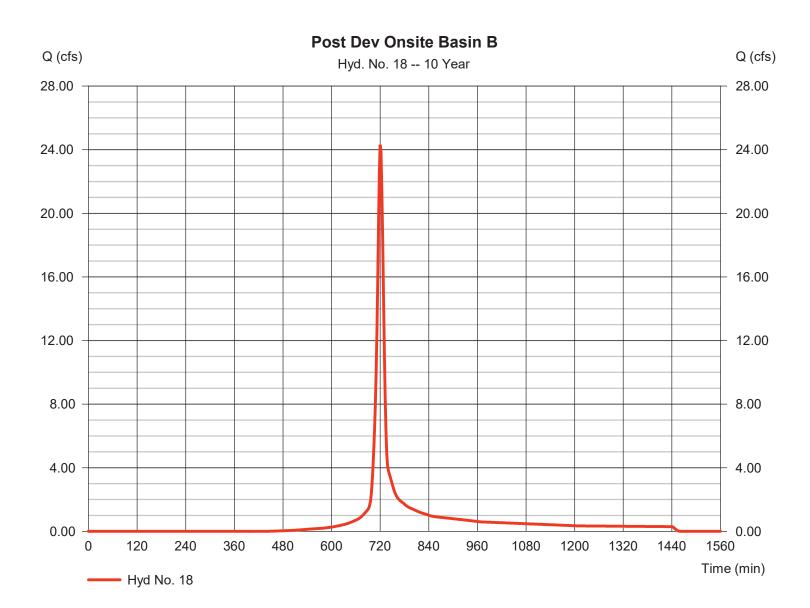
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Friday, 01 / 31 / 2025

Hyd. No. 18

Post Dev Onsite Basin B

Hydrograph type = SCS Runoff Peak discharge = 24.32 cfsStorm frequency = 10 yrsTime to peak = 720 min Time interval = 2 min Hyd. volume = 63.100 cuftDrainage area Curve number = 5.700 ac= 79 Hydraulic length Basin Slope = 0.0 %= 0 ftTc method Time of conc. (Tc) = 10.00 min = User Total precip. = 5.18 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



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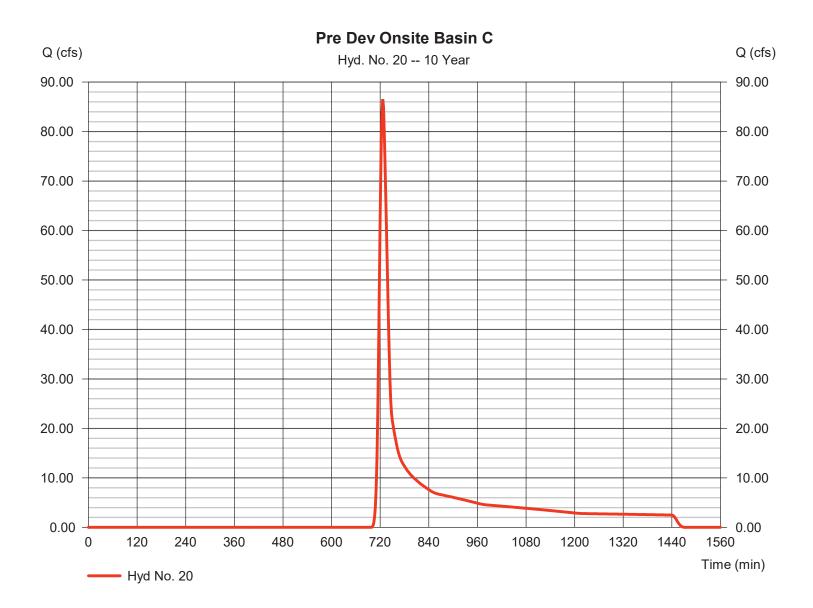
Friday, 01 / 31 / 2025

Hyd. No. 20

Pre Dev Onsite Basin C

Hydrograph type = SCS Runoff Peak discharge = 86.56 cfsStorm frequency Time to peak = 10 yrs= 726 min = 325,098 cuft Time interval = 2 min Hyd. volume Drainage area Curve number = 83.630 ac = 55 Hydraulic length Basin Slope = 0.0 %= 0 ftTc method Time of conc. (Tc) = 17.00 min = User

Total precip. = 5.18 in Distribution = Type II
Storm duration = 24 hrs Shape factor = 484



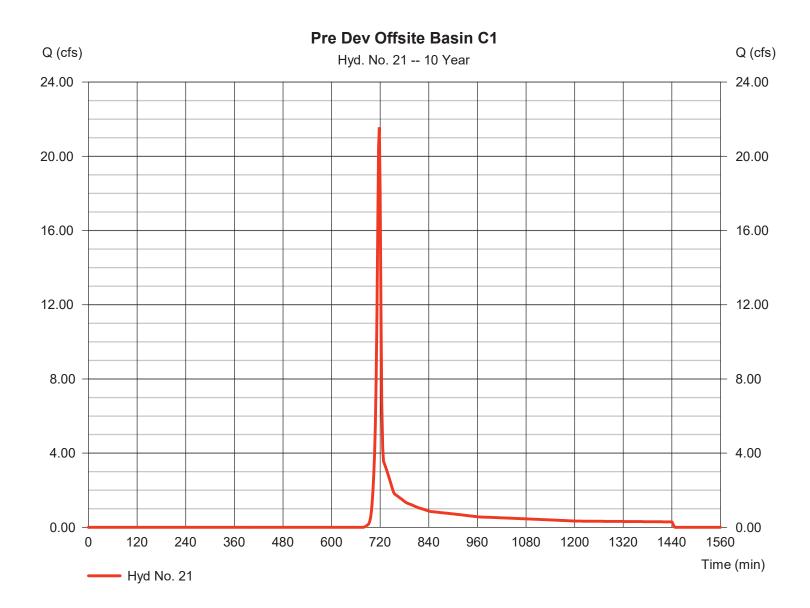
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Friday, 01 / 31 / 2025

Hyd. No. 21

Pre Dev Offsite Basin C1

Hydrograph type = SCS Runoff Peak discharge = 21.56 cfsStorm frequency = 10 yrsTime to peak = 718 min Time interval = 2 min Hyd. volume = 43,730 cuftDrainage area = 9.130 acCurve number = 60 Hydraulic length Basin Slope = 0.0 %= 0 ftTc method Time of conc. (Tc) $= 6.00 \, \text{min}$ = User Total precip. = 5.18 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



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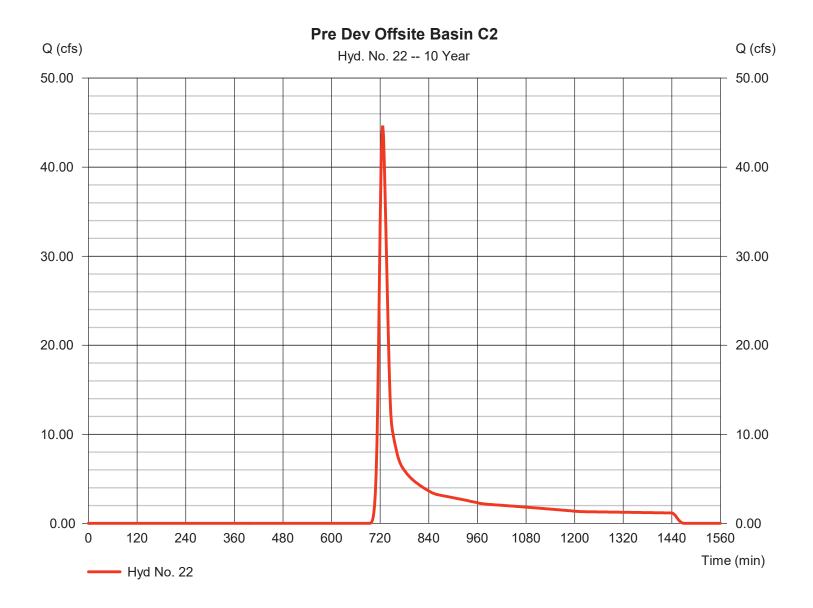
Friday, 01 / 31 / 2025

Hyd. No. 22

Pre Dev Offsite Basin C2

Hydrograph type = SCS Runoff Peak discharge = 44.67 cfsStorm frequency = 10 yrsTime to peak = 726 min Time interval = 2 min Hyd. volume = 160,184 cuft Drainage area = 36.720 acCurve number = 57 Basin Slope = 0.0 % Hydraulic length = 0 ftTc method = 18.00 min = User

Tc method = User Time of conc. (Tc) = 18.00 r
Total precip. = 5.18 in Distribution = Type II
Storm duration = 24 hrs Shape factor = 484



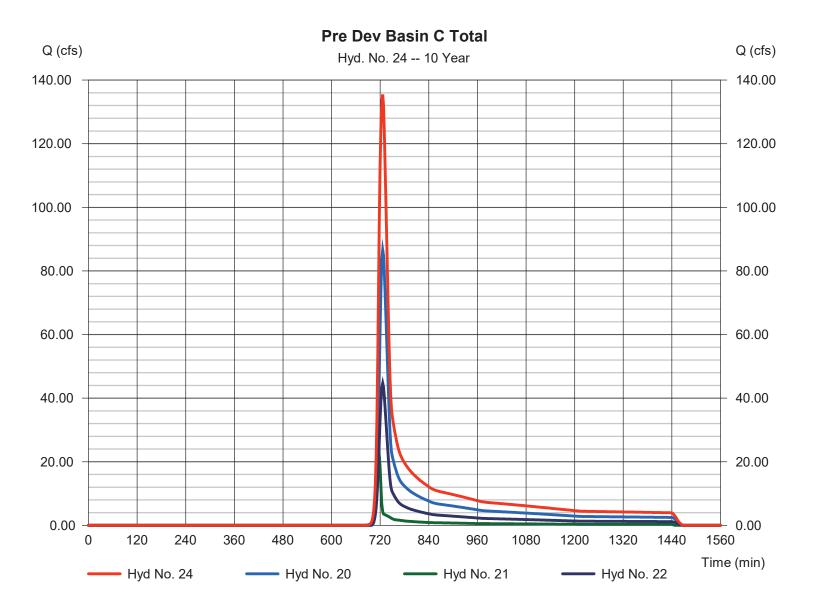
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Friday, 01 / 31 / 2025

Hyd. No. 24

Pre Dev Basin C Total

Hydrograph type = Combine Peak discharge = 135.52 cfsStorm frequency Time to peak = 10 yrs= 726 min Time interval = 2 min Hyd. volume = 529,012 cuft Inflow hyds. = 20, 21, 22 Contrib. drain. area = 129.480 ac



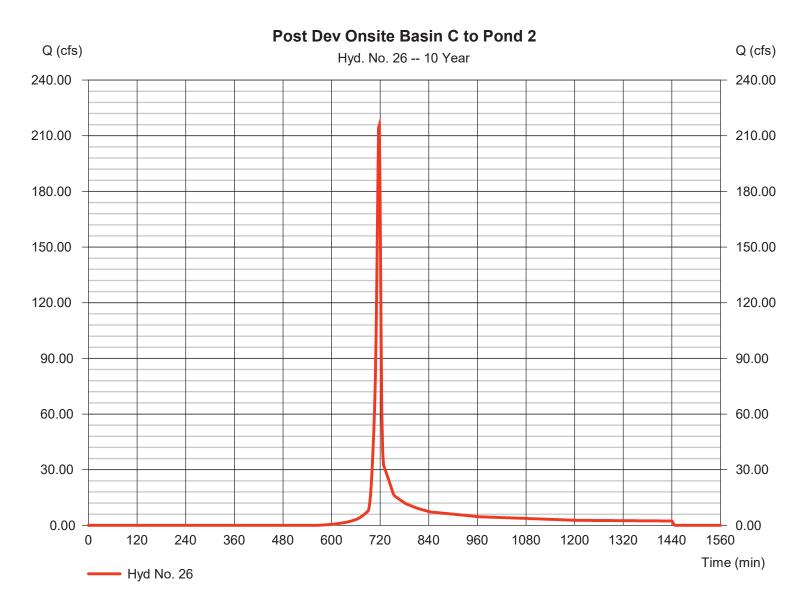
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Friday, 01 / 31 / 2025

Hyd. No. 26

Post Dev Onsite Basin C to Pond 2

Hydrograph type = SCS Runoff Peak discharge = 215.68 cfsStorm frequency = 10 yrsTime to peak = 718 min Time interval = 2 min Hyd. volume = 433.064 cuft Drainage area = 56.470 ac Curve number = 71 Hydraulic length = 0 ftBasin Slope = 0.0 % Tc method Time of conc. (Tc) = 5.00 min = User Total precip. = 5.18 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



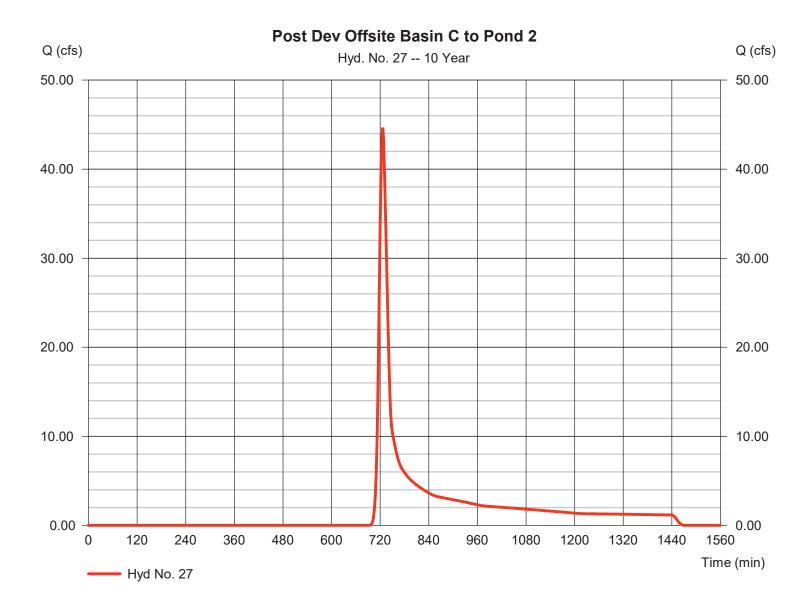
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Friday, 01 / 31 / 2025

Hyd. No. 27

Post Dev Offsite Basin C to Pond 2

= SCS Runoff Hydrograph type Peak discharge = 44.67 cfsStorm frequency = 10 yrsTime to peak = 726 min Time interval = 2 min Hyd. volume = 160.184 cuft Drainage area = 36.720 acCurve number = 57 Hydraulic length Basin Slope = 0.0 % = 0 ftTc method Time of conc. (Tc) = 18.00 min = User Total precip. = 5.18 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



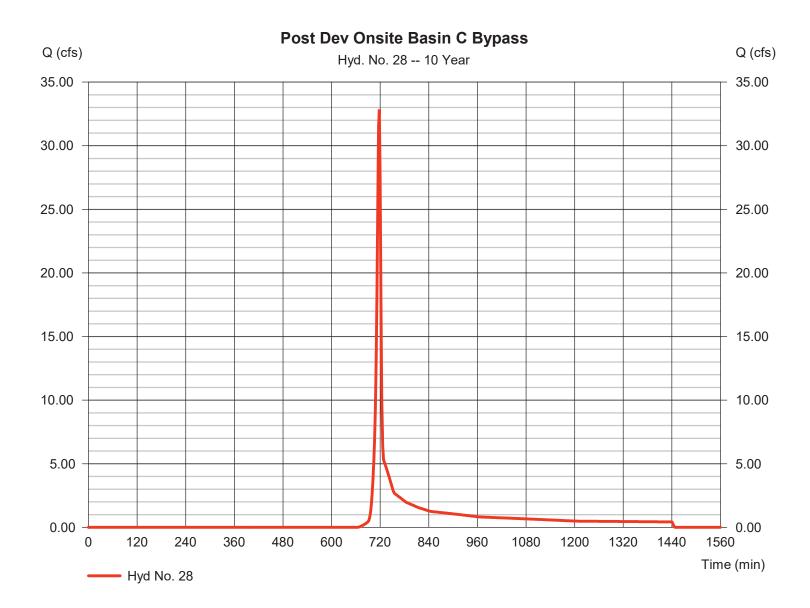
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Friday, 01 / 31 / 2025

Hyd. No. 28

Post Dev Onsite Basin C Bypass

Hydrograph type = SCS Runoff Peak discharge = 32.86 cfsStorm frequency Time to peak = 10 yrs= 718 min Time interval = 2 min Hyd. volume = 66.122 cuft Drainage area = 12.530 acCurve number = 62 Hydraulic length Basin Slope = 0.0 %= 0 ftTc method Time of conc. (Tc) $= 5.00 \, \text{min}$ = User Total precip. = 5.18 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



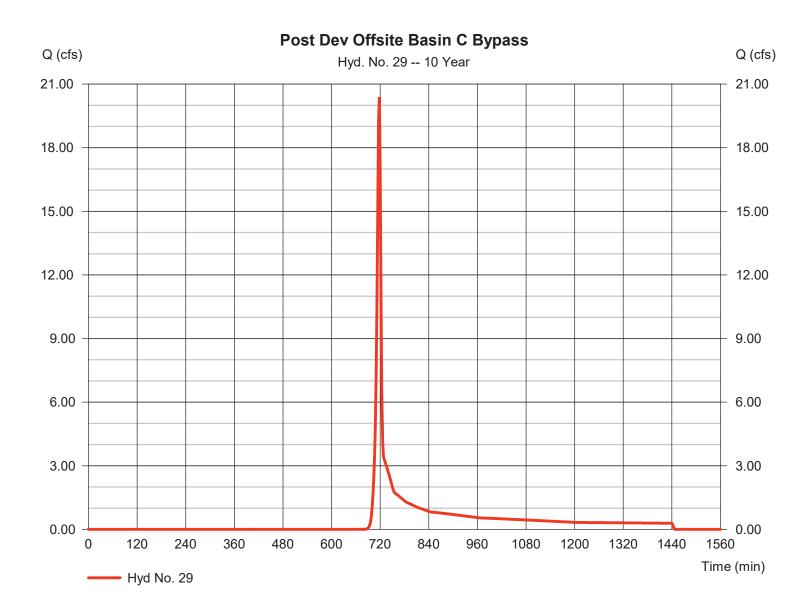
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Friday, 01 / 31 / 2025

Hyd. No. 29

Post Dev Offsite Basin C Bypass

Hydrograph type = SCS Runoff Peak discharge = 20.39 cfsStorm frequency = 10 yrsTime to peak = 718 min Time interval = 2 min Hyd. volume = 41,561 cuft Drainage area = 9.130 acCurve number = 59 Hydraulic length Basin Slope = 0.0 %= 0 ftTc method Time of conc. (Tc) = 6.00 min = User Total precip. = 5.18 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



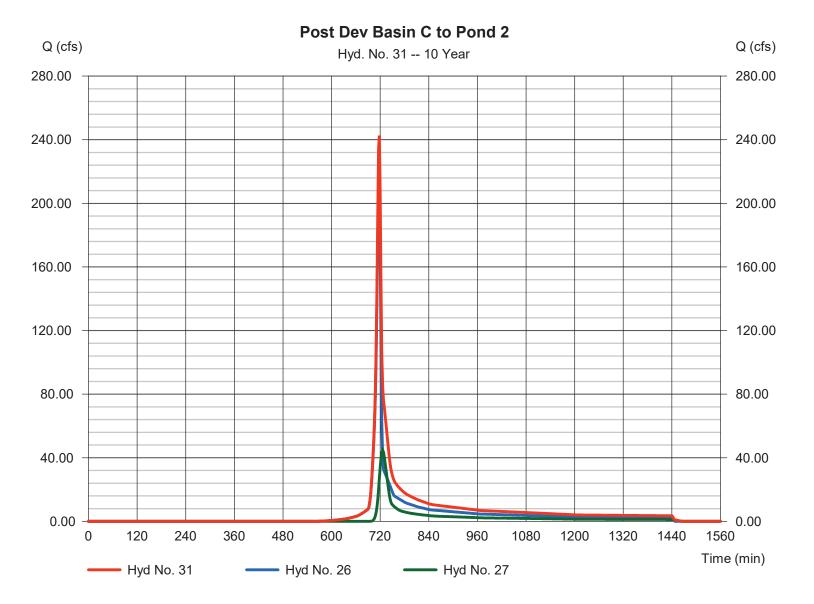
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Friday, 01 / 31 / 2025

Hyd. No. 31

Post Dev Basin C to Pond 2

Hydrograph type = Combine Peak discharge = 242.62 cfsStorm frequency Time to peak = 10 yrs= 718 min Time interval = 2 min Hyd. volume = 593,249 cuft Inflow hyds. = 26, 27Contrib. drain. area = 93.190 ac



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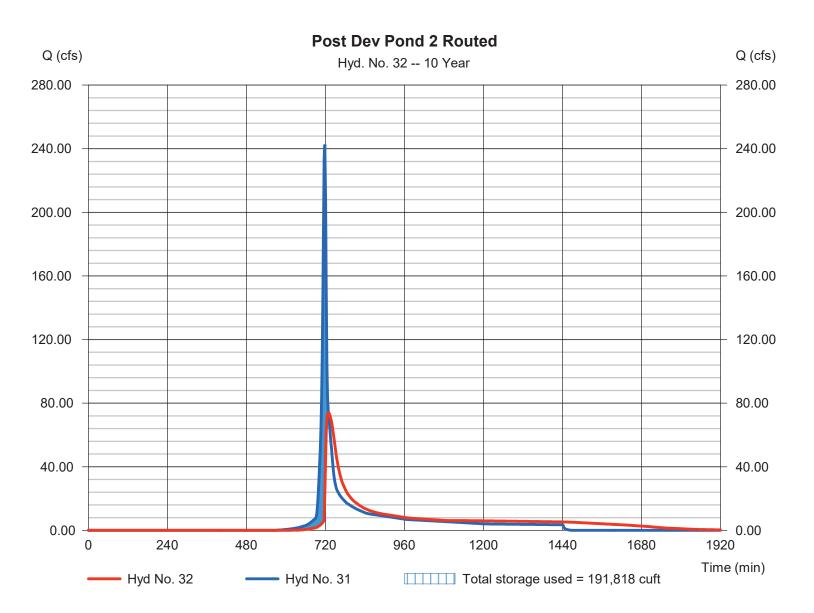
Friday, 01 / 31 / 2025

Hyd. No. 32

Post Dev Pond 2 Routed

Hydrograph type Peak discharge = 73.86 cfs= Reservoir Storm frequency = 10 yrsTime to peak = 730 min Time interval = 2 min Hyd. volume = 593,229 cuft Inflow hyd. No. = 31 - Post Dev Basin C to PondMax. Elevation $= 1109.51 \, \text{ft}$ = Pond 2 Max. Storage Reservoir name = 191,818 cuft

Storage Indication method used.



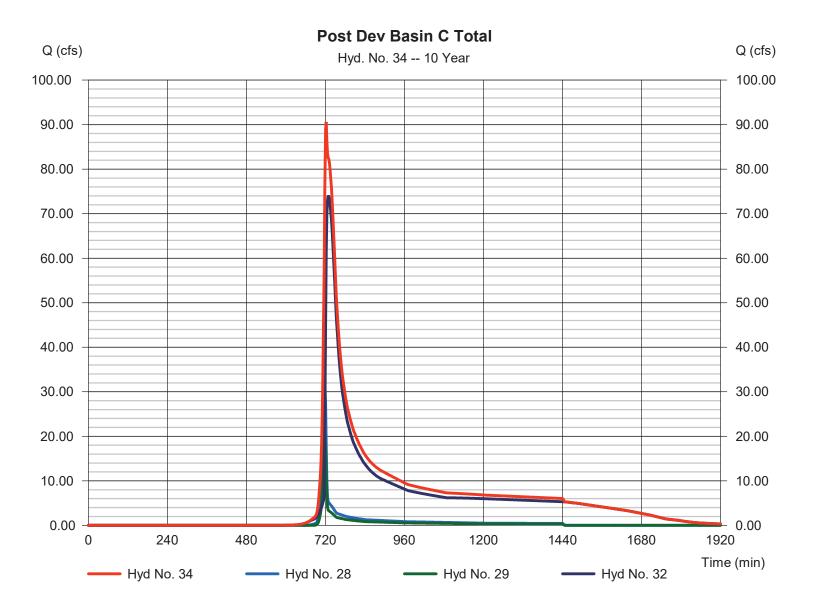
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Friday, 01 / 31 / 2025

Hyd. No. 34

Post Dev Basin C Total

Hydrograph type = Combine Peak discharge = 90.60 cfsStorm frequency Time to peak = 10 yrs= 722 min Time interval = 2 min Hyd. volume = 700,912 cuft Inflow hyds. = 28, 29, 32 Contrib. drain. area = 21.660 ac



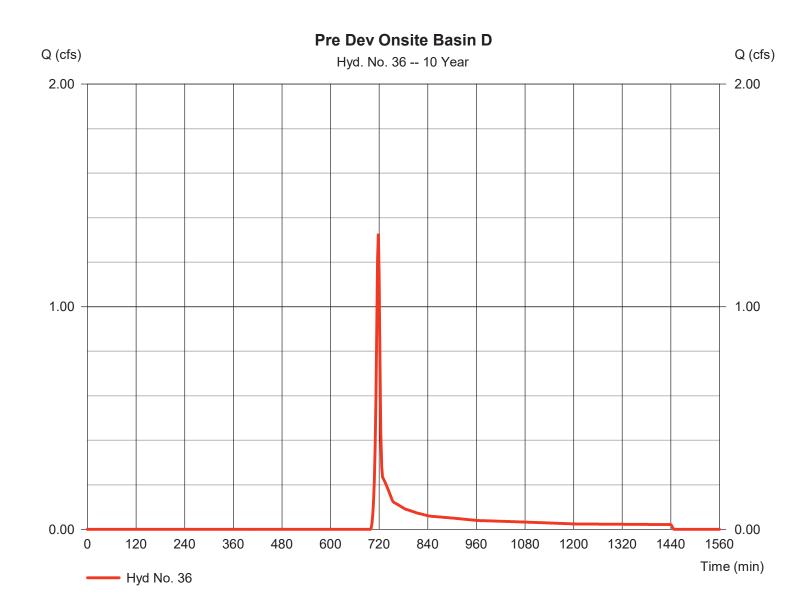
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Friday, 01 / 31 / 2025

Hyd. No. 36

Pre Dev Onsite Basin D

Hydrograph type = SCS Runoff Peak discharge = 1.328 cfsStorm frequency = 10 yrsTime to peak = 718 min Time interval = 2 min Hyd. volume = 2.806 cuft Drainage area Curve number = 0.770 ac= 55 Basin Slope = 0.0 %Hydraulic length = 0 ftTc method Time of conc. (Tc) $= 5.00 \, \text{min}$ = User Total precip. = 5.18 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



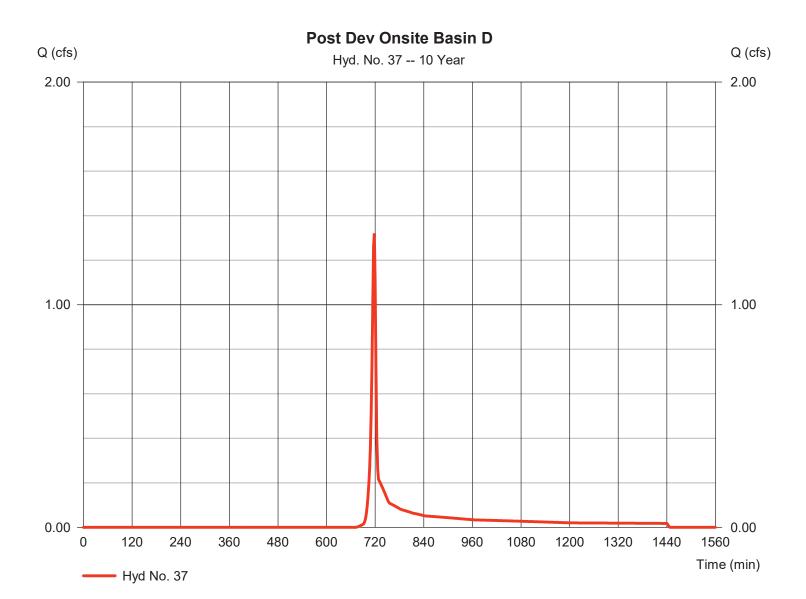
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Friday, 01 / 31 / 2025

Hyd. No. 37

Post Dev Onsite Basin D

Hydrograph type = SCS Runoff Peak discharge = 1.321 cfsStorm frequency = 10 yrsTime to peak = 718 min Time interval = 2 min Hyd. volume = 2.667 cuft Drainage area = 0.530 acCurve number = 61 Basin Slope = 0.0 %Hydraulic length = 0 ftTc method Time of conc. (Tc) $= 5.00 \, \text{min}$ = User Total precip. = 5.18 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



Hydrograph Summary Report

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

Hyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description	
1	SCS Runoff	77.35	2	728	298,731				Pre Dev Onsite Basin A	
2	SCS Runoff	23.56	2	722	62,577				Pre Dev Offite Basin A1	
3	SCS Runoff	1.793	2	720	4,180				Pre Dev Offsite Basin A2	
5	Combine	95.15	2	726	365,488	1, 2, 3,			Pre Dev Basin A Total	
7	SCS Runoff	255.60	2	716	520,618				Post Dev Onsite Basin A to Pond 1	
8	SCS Runoff	5.175	2	722	13,883				Post Dev Offsite Basin A to Pond 1	
9	SCS Runoff	19.10	2	720	44,147				Post Dev Offsite Basin A1 Bypass	
10	SCS Runoff	1.793	2	720	4,180				Post Dev Offsite Basin A2 Bypass	
12	Combine	259.28	2	716	534,501	7, 8,			Post Dev Basin A to Pond 1	
13	Reservoir	48.37	2	726	518,530	12	1061.51	254,883	Post Dev Pond 1 Routed	
15	Combine	60.95	2	724	566,857	9, 10, 13,			Post Dev Basin A Total	
17	SCS Runoff	7.950	2	730	32,804				Pre Dev Onsite Basin B	
18	SCS Runoff	31.30	2	720	81,570				Post Dev Onsite Basin B	
20	SCS Runoff	141.61	2	726	488,935				Pre Dev Onsite Basin C	
21	SCS Runoff	31.44	2	718	62,979				Pre Dev Offsite Basin C1	
22	SCS Runoff	70.18	2	726	236,461				Pre Dev Offsite Basin C2	
24	Combine	217.80	2	726	788,376	20, 21, 22,			Pre Dev Basin C Total	
26	SCS Runoff	288.20	2	716	581,944				Post Dev Onsite Basin C to Pond 2	
27	SCS Runoff	70.18	2	726	236,461				Post Dev Offsite Basin C to Pond 2	
28	SCS Runoff	46.92	2	718	93,838				Post Dev Onsite Basin C Bypass	
29	SCS Runoff	30.06	2	718	60,327				Post Dev Offsite Basin C Bypass	
31	Combine	333.93	2	718	818,404	26, 27,			Post Dev Basin C to Pond 2	
32	Reservoir	143.60	2	724	818,386	31	1111.04	246,093	Post Dev Pond 2 Routed	
34	Combine	180.05	2	720	972,552	28, 29, 32,			Post Dev Basin C Total	
36	SCS Runoff	2.073	2	718	4,220				Pre Dev Onsite Basin D	
37	SCS Runoff	1.905	2	718	3,812				Post Dev Onsite Basin D	
1220263 Hydrology Concept.gpw					Return F	Return Period: 25 Year			Friday, 01 / 31 / 2025	

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Friday, 01 / 31 / 2025

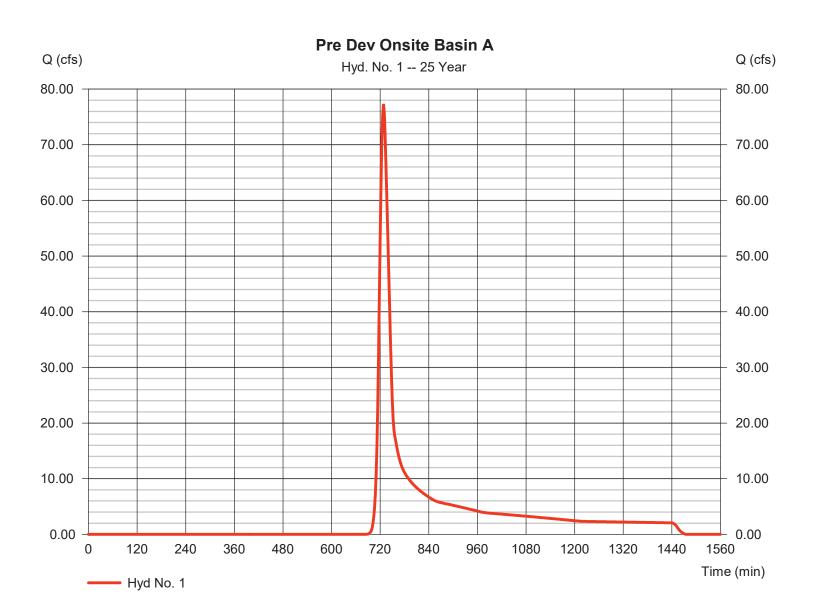
Hyd. No. 1

Pre Dev Onsite Basin A

Hydrograph type= SCS RunoffPeak discharge= 77.35 cfsStorm frequency= 25 yrsTime to peak= 728 minTime interval= 2 minHyd. volume= 298,731 cuftDrainage area= 50,200 acCurve number= 55

Drainage area = 50.200 ac Curve number = 55 Basin Slope = 0.0 % Hydraulic length = 0 ft

Tc method = User Time of conc. (Tc) = 22.00 min
Total precip. = 6.16 in Distribution = Type II
Storm duration = 24 hrs Shape factor = 484



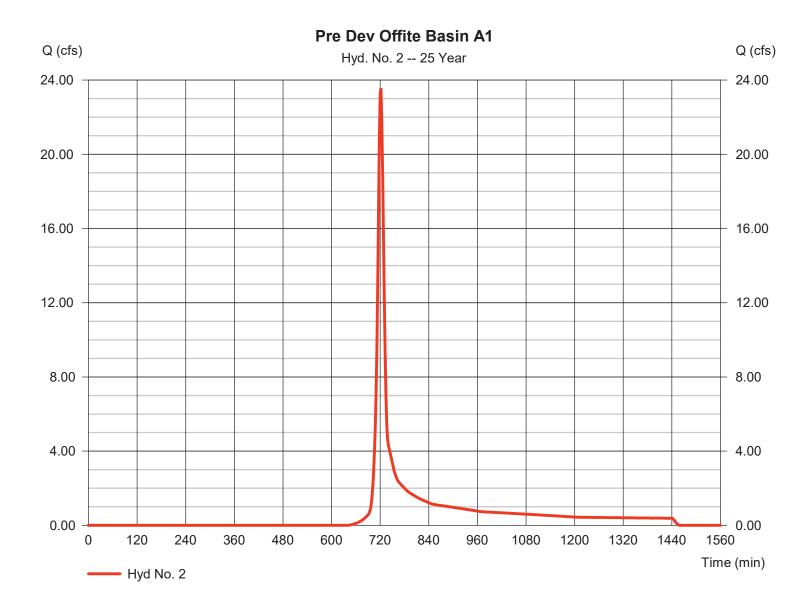
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Friday, 01 / 31 / 2025

Hyd. No. 2

Pre Dev Offite Basin A1

Hydrograph type = SCS Runoff Peak discharge = 23.56 cfsStorm frequency = 25 yrsTime to peak = 722 min Time interval = 2 min Hyd. volume = 62,577 cuftDrainage area = 7.910 acCurve number = 61 Hydraulic length Basin Slope = 0.0 %= 0 ftTc method Time of conc. (Tc) = 12.00 min = User Total precip. = 6.16 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



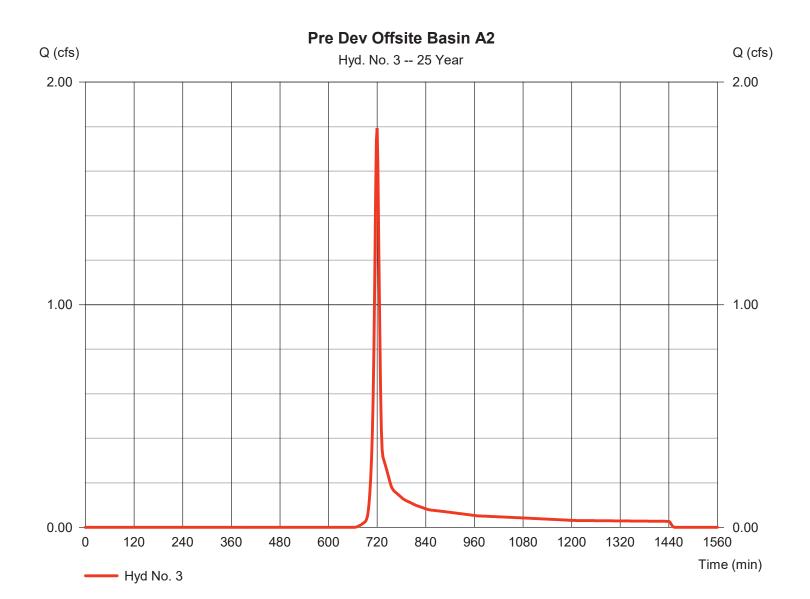
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Friday, 01 / 31 / 2025

Hyd. No. 3

Pre Dev Offsite Basin A2

Hydrograph type = SCS Runoff Peak discharge = 1.793 cfsStorm frequency = 25 yrs Time to peak = 720 min Time interval = 2 min Hyd. volume = 4,180 cuftDrainage area = 0.620 acCurve number = 58 Basin Slope = 0.0 %Hydraulic length = 0 ftTc method Time of conc. (Tc) $= 8.00 \, \text{min}$ = User Total precip. = 6.16 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



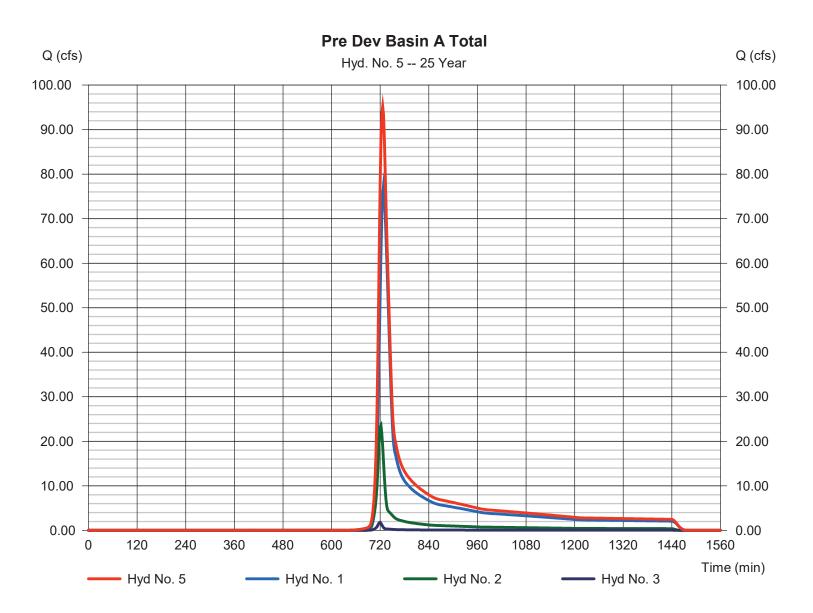
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Friday, 01 / 31 / 2025

Hyd. No. 5

Pre Dev Basin A Total

Hydrograph type = Combine Peak discharge = 95.15 cfsStorm frequency Time to peak = 25 yrs= 726 min Time interval = 2 min Hyd. volume = 365,488 cuft Inflow hyds. = 1, 2, 3Contrib. drain. area = 58.730 ac



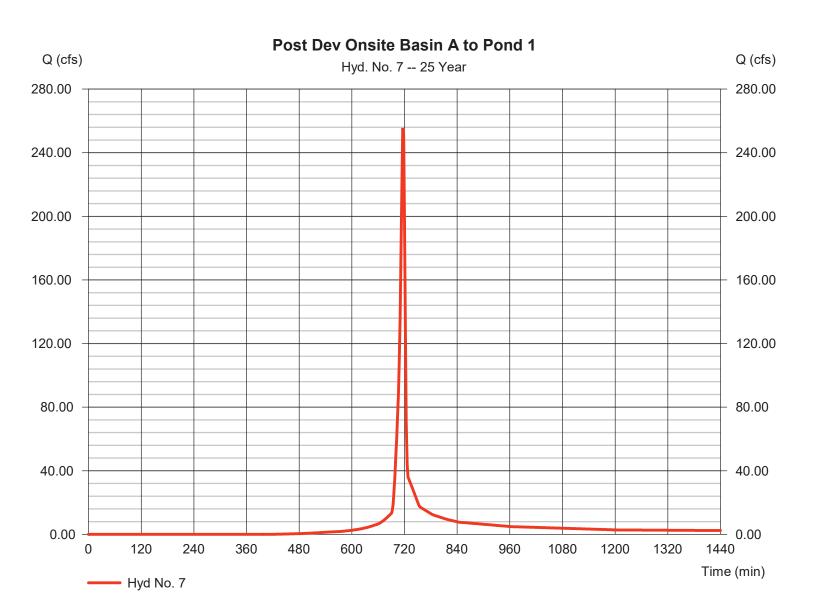
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Friday, 01 / 31 / 2025

Hyd. No. 7

Post Dev Onsite Basin A to Pond 1

Hydrograph type = SCS Runoff Peak discharge = 255.60 cfsStorm frequency Time to peak = 25 yrs = 716 min Time interval = 2 min Hyd. volume = 520,618 cuft Drainage area = 42.270 ac Curve number = 77 = 0 ftHydraulic length Basin Slope = 0.0 %Tc method Time of conc. (Tc) $= 5.00 \, \text{min}$ = User Total precip. = 6.16 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



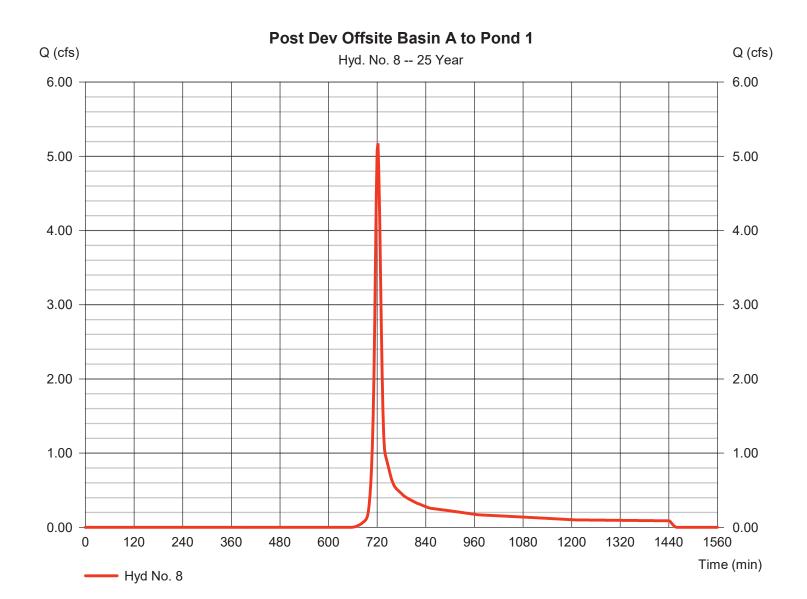
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Friday, 01 / 31 / 2025

Hyd. No. 8

Post Dev Offsite Basin A to Pond 1

Hydrograph type = SCS Runoff Peak discharge = 5.175 cfsStorm frequency = 25 yrs Time to peak = 722 min Time interval = 2 min Hyd. volume = 13.883 cuft Drainage area Curve number = 1.910 ac= 59 Basin Slope = 0.0 %Hydraulic length = 0 ftTc method Time of conc. (Tc) = 12.00 min = User Total precip. = 6.16 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



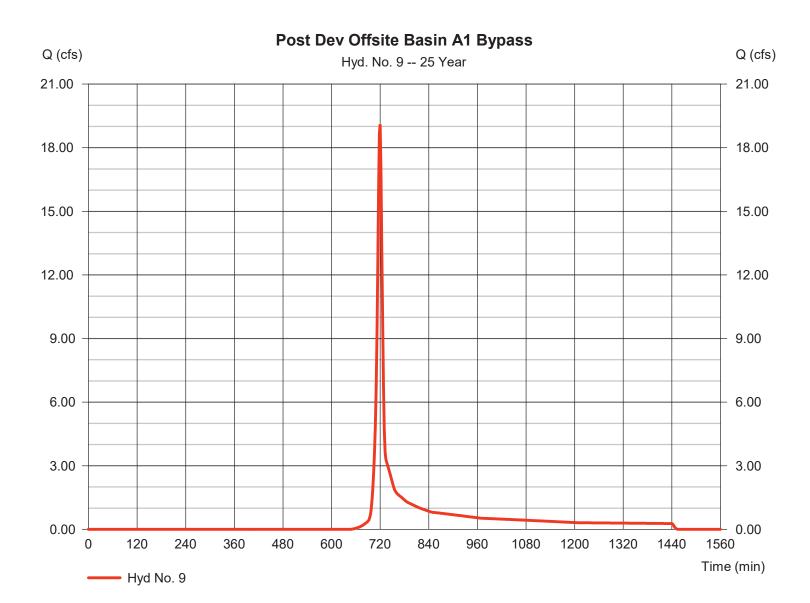
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Friday, 01 / 31 / 2025

Hyd. No. 9

Post Dev Offsite Basin A1 Bypass

Hydrograph type = SCS Runoff Peak discharge = 19.10 cfsStorm frequency = 25 yrsTime to peak = 720 min Time interval = 2 min Hyd. volume = 44,147 cuft Drainage area = 6.000 acCurve number = 60 Hydraulic length Basin Slope = 0.0 %= 0 ftTc method Time of conc. (Tc) = 8.00 min = User Total precip. = 6.16 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



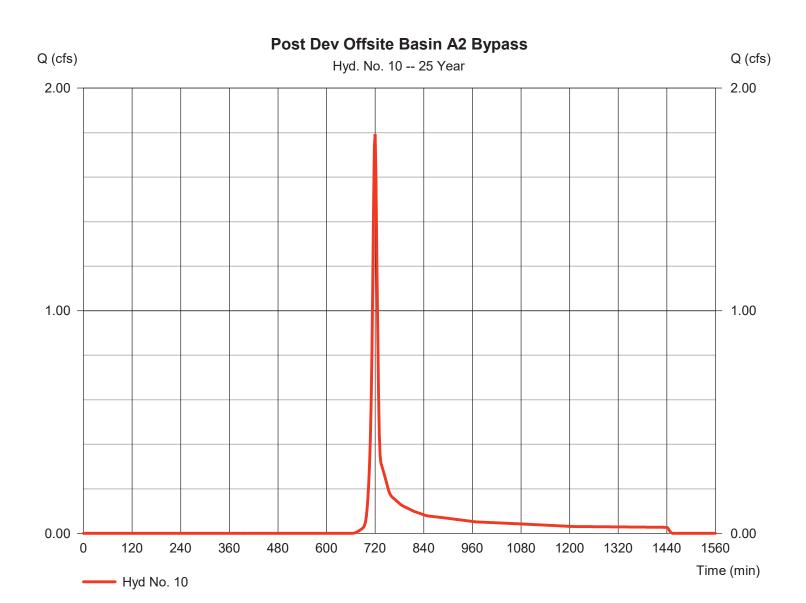
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Friday, 01 / 31 / 2025

Hyd. No. 10

Post Dev Offsite Basin A2 Bypass

Hydrograph type = SCS Runoff Peak discharge = 1.793 cfsStorm frequency = 25 yrsTime to peak = 720 min Time interval = 2 min Hyd. volume = 4,180 cuftDrainage area = 0.620 acCurve number = 58 Basin Slope = 0.0 %Hydraulic length = 0 ftTc method Time of conc. (Tc) = 8.00 min = User Total precip. = 6.16 inDistribution = Type II Storm duration Shape factor = 24 hrs = 484



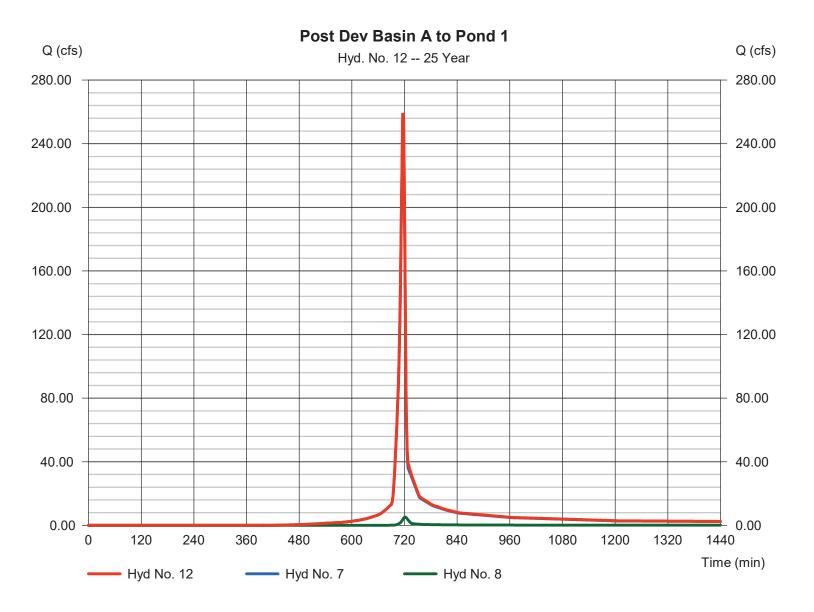
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Friday, 01 / 31 / 2025

Hyd. No. 12

Post Dev Basin A to Pond 1

Hydrograph type = Combine Peak discharge = 259.28 cfsStorm frequency = 25 yrsTime to peak = 716 min Time interval = 2 min Hyd. volume = 534,501 cuft Inflow hyds. = 7,8 Contrib. drain. area = 44.180 ac



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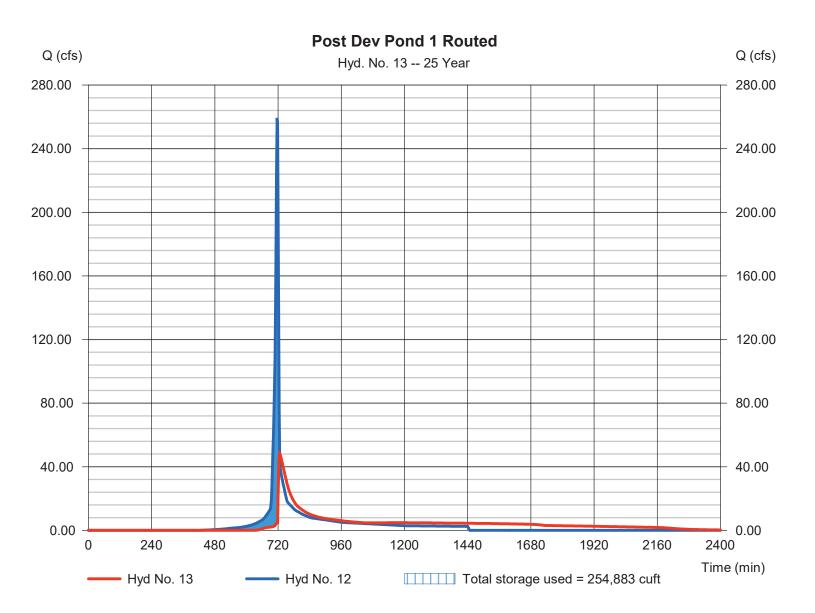
Friday, 01 / 31 / 2025

Hyd. No. 13

Post Dev Pond 1 Routed

Hydrograph type Peak discharge = 48.37 cfs= Reservoir Storm frequency Time to peak = 25 yrs= 726 min Time interval = 2 min Hyd. volume = 518,530 cuft Inflow hyd. No. = 12 - Post Dev Basin A to PondMax. Elevation $= 1061.51 \, \text{ft}$ = Pond 1 Max. Storage Reservoir name = 254,883 cuft

Storage Indication method used.



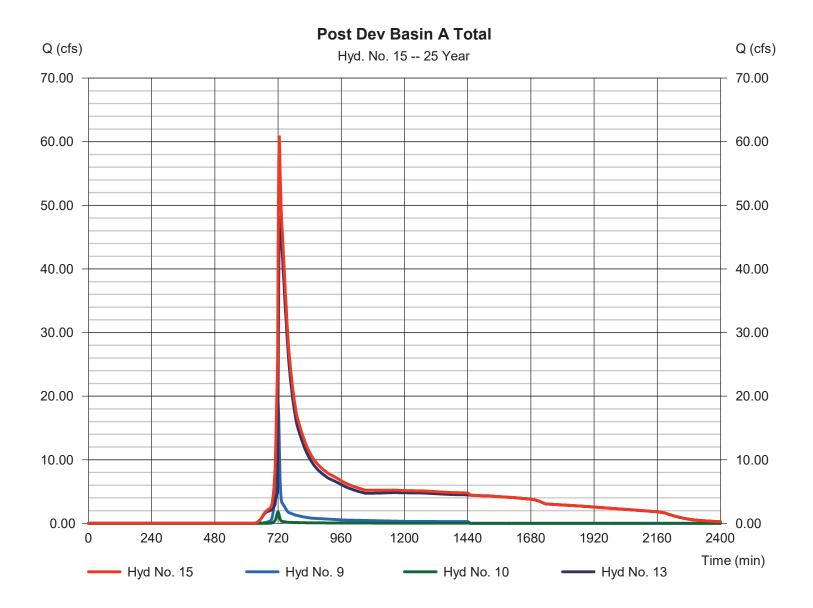
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Friday, 01 / 31 / 2025

Hyd. No. 15

Post Dev Basin A Total

Hydrograph type = Combine Peak discharge = 60.95 cfsStorm frequency Time to peak = 25 yrs= 724 min Time interval = 2 min Hyd. volume = 566,857 cuft Inflow hyds. = 9, 10, 13Contrib. drain. area = 6.620 ac



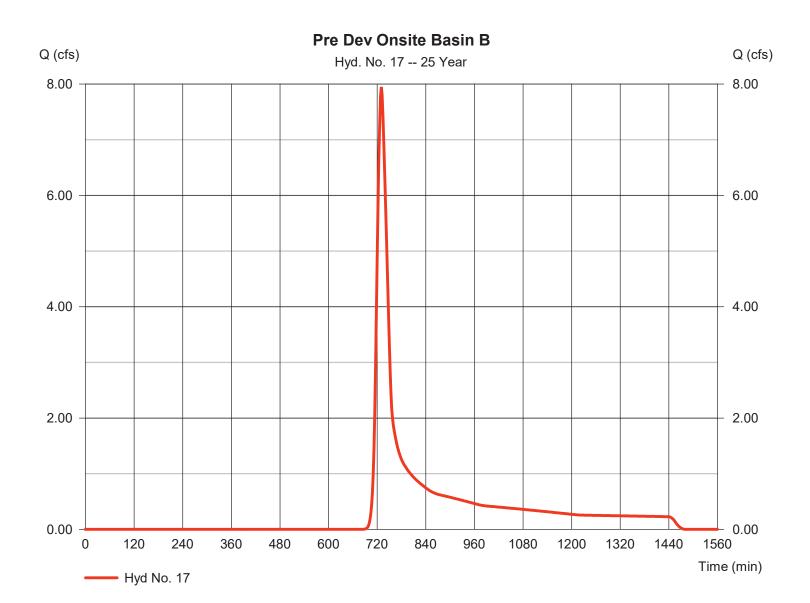
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Friday, 01 / 31 / 2025

Hyd. No. 17

Pre Dev Onsite Basin B

Hydrograph type = SCS Runoff Peak discharge = 7.950 cfsStorm frequency = 25 yrs Time to peak = 730 min Time interval = 2 min Hyd. volume = 32.804 cuft Drainage area = 5.700 acCurve number = 55 Basin Slope = 0.0 %Hydraulic length = 0 ftTc method Time of conc. (Tc) = 25.00 min = User Total precip. = 6.16 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



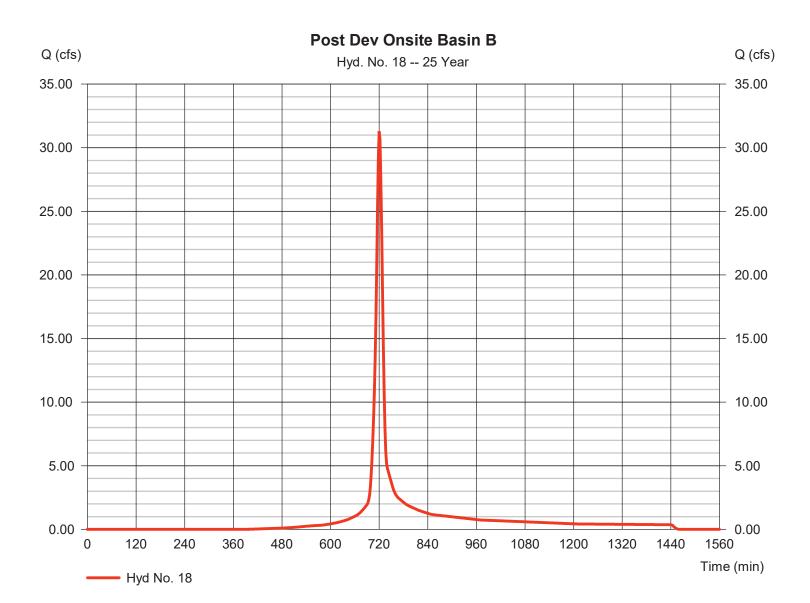
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Friday, 01 / 31 / 2025

Hyd. No. 18

Post Dev Onsite Basin B

Hydrograph type = SCS Runoff Peak discharge = 31.30 cfsStorm frequency Time to peak = 25 yrs= 720 min Time interval = 2 min Hyd. volume = 81,570 cuft Drainage area = 5.700 acCurve number = 79 Hydraulic length Basin Slope = 0.0 %= 0 ftTc method Time of conc. (Tc) = 10.00 min = User Total precip. = 6.16 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



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Friday, 01 / 31 / 2025

Hyd. No. 20

Pre Dev Onsite Basin C

Hydrograph type = SCS Runoff Peak discharge = 141.61 cfsStorm frequency Time to peak = 25 yrs = 726 min Time interval = 2 min Hyd. volume = 488.935 cuft Drainage area Curve number = 83.630 ac = 55 Hydraulic length Basin Slope = 0.0 % = 0 ftTc method Time of conc. (Tc) = 17.00 min = User Total precip. = 6.16 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484

Pre Dev Onsite Basin C Q (cfs) Q (cfs) Hyd. No. 20 -- 25 Year 160.00 160.00 140.00 140.00 120.00 120.00 100.00 100.00 80.00 80.00 60.00 60.00 40.00 40.00 20.00 20.00 0.00 0.00 120 240 360 480 600 720 840 960 1080 1200 1320 1440 1560 Time (min) Hyd No. 20

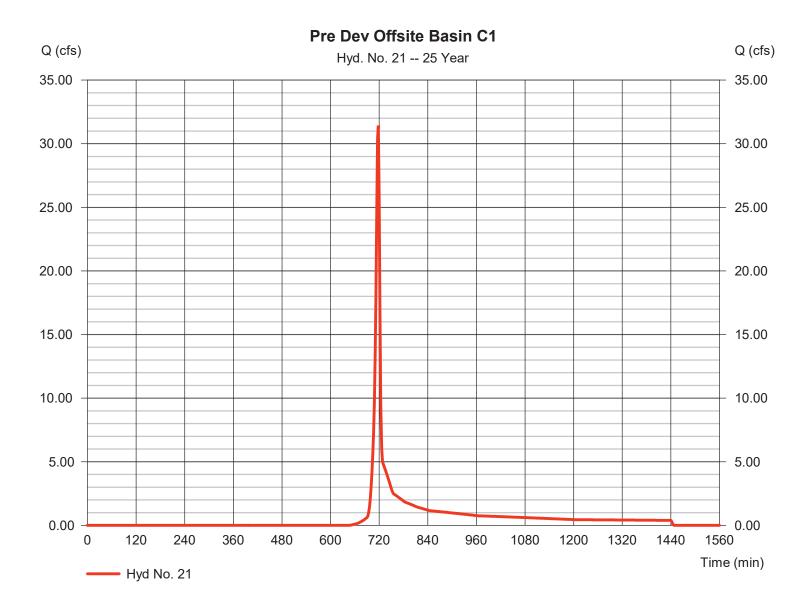
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Friday, 01 / 31 / 2025

Hyd. No. 21

Pre Dev Offsite Basin C1

Hydrograph type = SCS Runoff Peak discharge = 31.44 cfsStorm frequency Time to peak = 25 yrs= 718 min Time interval = 2 min Hyd. volume = 62.979 cuft Drainage area = 9.130 acCurve number = 60 Hydraulic length Basin Slope = 0.0 %= 0 ftTc method Time of conc. (Tc) $= 6.00 \, \text{min}$ = User Total precip. = 6.16 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



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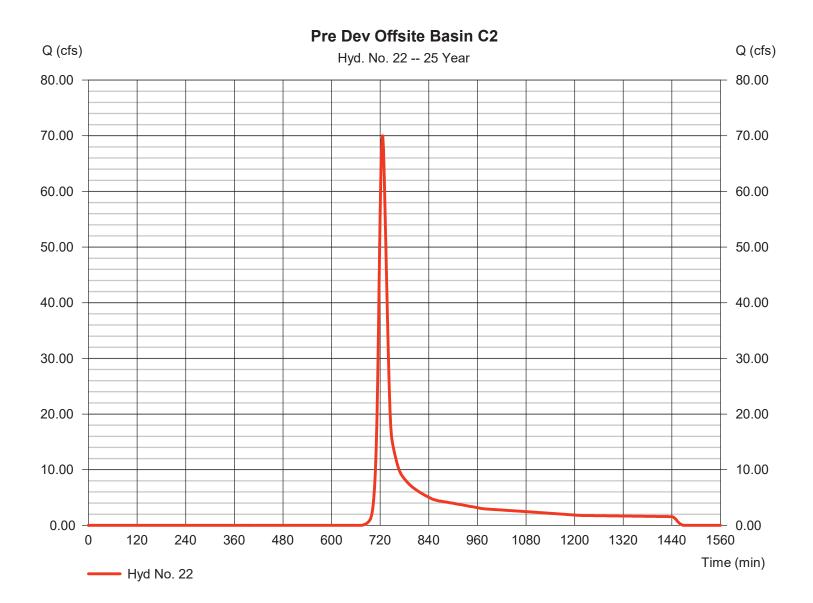
Friday, 01 / 31 / 2025

Hyd. No. 22

Pre Dev Offsite Basin C2

Hydrograph type = SCS Runoff Peak discharge = 70.18 cfsStorm frequency Time to peak = 25 yrs = 726 min Time interval = 2 min Hyd. volume = 236.461 cuft Drainage area = 36.720 ac Curve number = 57 Hydraulic length Basin Slope = 0.0 % = 0 ftTc method Time of conc. (Tc) = 18.00 min = User

Total precip. = 6.16 in Distribution = Type II
Storm duration = 24 hrs Shape factor = 484



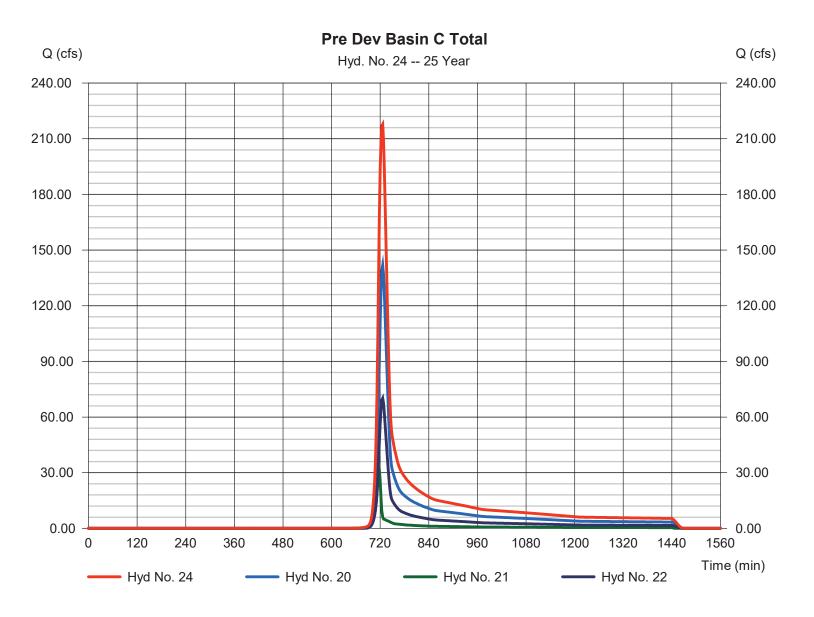
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Friday, 01 / 31 / 2025

Hyd. No. 24

Pre Dev Basin C Total

Hydrograph type = Combine Peak discharge = 217.80 cfsStorm frequency Time to peak = 25 yrs= 726 min Time interval = 2 min Hyd. volume = 788,376 cuft Inflow hyds. = 20, 21, 22 Contrib. drain. area = 129.480 ac



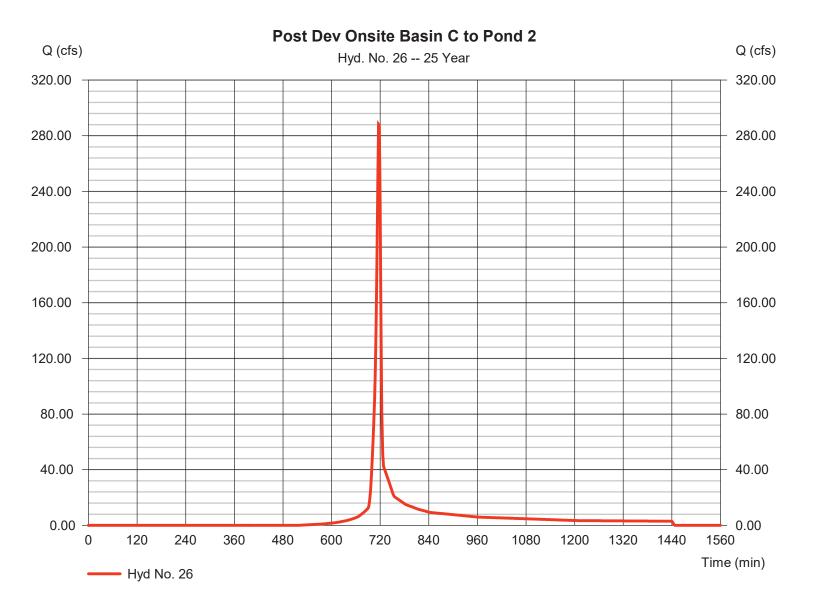
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Friday, 01 / 31 / 2025

Hyd. No. 26

Post Dev Onsite Basin C to Pond 2

= SCS Runoff Hydrograph type Peak discharge = 288.20 cfsStorm frequency = 25 yrsTime to peak = 716 min Time interval = 2 min Hyd. volume = 581.944 cuft Drainage area = 56.470 ac Curve number = 71 Hydraulic length = 0 ftBasin Slope = 0.0 % Tc method Time of conc. (Tc) = 5.00 min = User Total precip. = 6.16 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

= 24 hrs

Friday, 01 / 31 / 2025

= 484

Hyd. No. 27

Storm duration

Post Dev Offsite Basin C to Pond 2

Hyd No. 27

= SCS Runoff Hydrograph type Peak discharge = 70.18 cfsStorm frequency Time to peak = 25 yrs= 726 min Time interval = 2 min Hyd. volume = 236.461 cuft Drainage area = 36.720 acCurve number = 57 Hydraulic length Basin Slope = 0.0 % = 0 ftTc method Time of conc. (Tc) = 18.00 min = User Total precip. = 6.16 inDistribution = Type II

Shape factor

Post Dev Offsite Basin C to Pond 2 Q (cfs) Q (cfs) Hyd. No. 27 -- 25 Year 80.00 00.08 70.00 70.00 60.00 60.00 50.00 50.00 40.00 40.00 30.00 30.00 20.00 20.00 10.00 10.00 0.00 0.00 120 240 360 480 600 720 840 960 1080 1200 1320 1440 1560 Time (min)

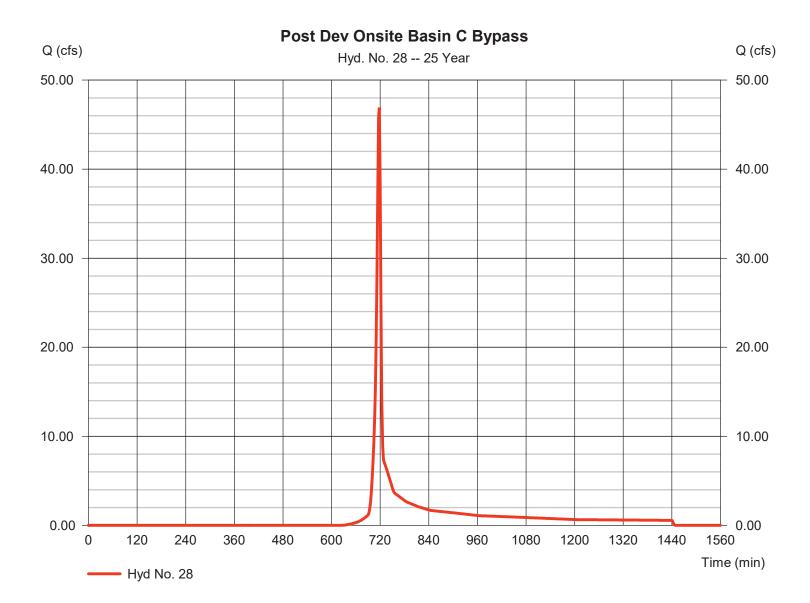
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Friday, 01 / 31 / 2025

Hyd. No. 28

Post Dev Onsite Basin C Bypass

Hydrograph type = SCS Runoff Peak discharge = 46.92 cfsStorm frequency = 25 yrs Time to peak = 718 min Time interval = 2 min Hyd. volume = 93.838 cuft Drainage area = 12.530 acCurve number = 62 Basin Slope = 0.0 %Hydraulic length = 0 ftTc method Time of conc. (Tc) = 5.00 min = User Total precip. = 6.16 inDistribution = Type II Shape factor Storm duration = 24 hrs = 484



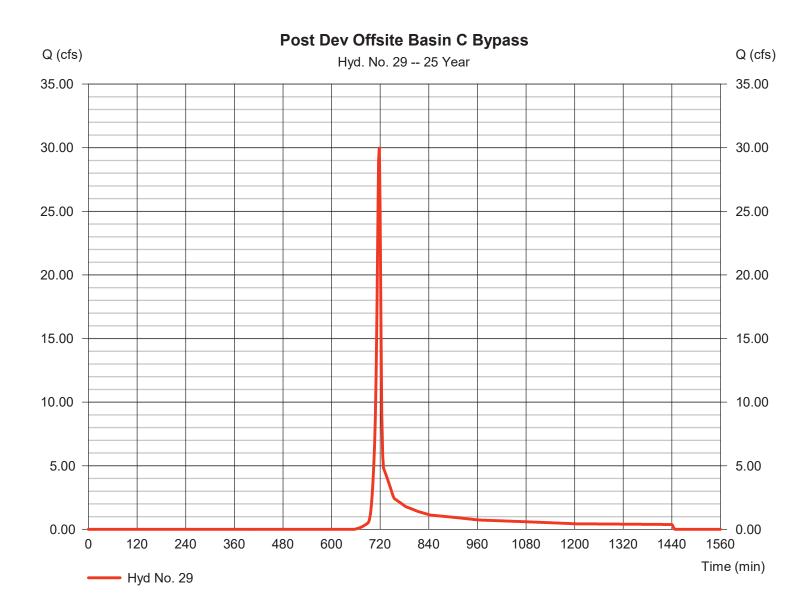
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Friday, 01 / 31 / 2025

Hyd. No. 29

Post Dev Offsite Basin C Bypass

Hydrograph type = SCS Runoff Peak discharge = 30.06 cfsStorm frequency Time to peak = 25 yrs = 718 min Time interval = 2 min Hyd. volume = 60.327 cuftDrainage area = 9.130 acCurve number = 59 Hydraulic length Basin Slope = 0.0 %= 0 ftTc method Time of conc. (Tc) = 6.00 min = User Total precip. = 6.16 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



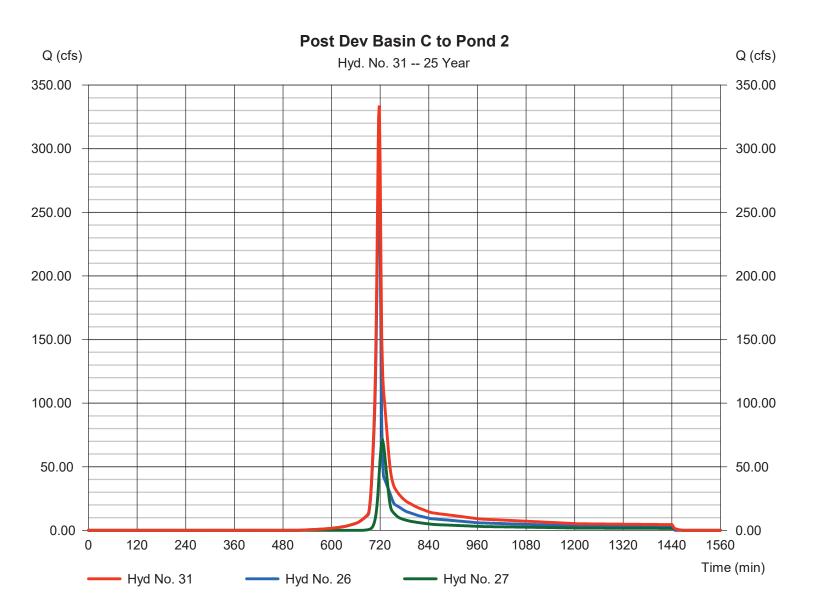
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Friday, 01 / 31 / 2025

Hyd. No. 31

Post Dev Basin C to Pond 2

Hydrograph type = Combine Peak discharge = 333.93 cfsStorm frequency Time to peak = 25 yrs= 718 min = 818,404 cuft Time interval = 2 min Hyd. volume Inflow hyds. = 26, 27Contrib. drain. area = 93.190 ac



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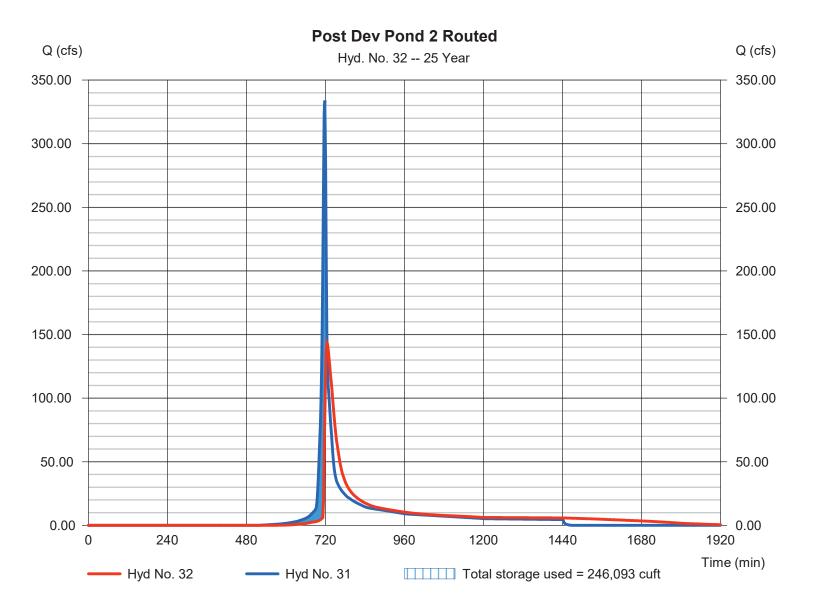
Friday, 01 / 31 / 2025

Hyd. No. 32

Post Dev Pond 2 Routed

Hydrograph type Peak discharge = 143.60 cfs= Reservoir Storm frequency = 25 yrsTime to peak = 724 min Time interval = 2 min Hyd. volume = 818,386 cuft Inflow hyd. No. = 31 - Post Dev Basin C to PondMax. Elevation = 1111.04 ft = Pond 2 Max. Storage Reservoir name = 246,093 cuft

Storage Indication method used.



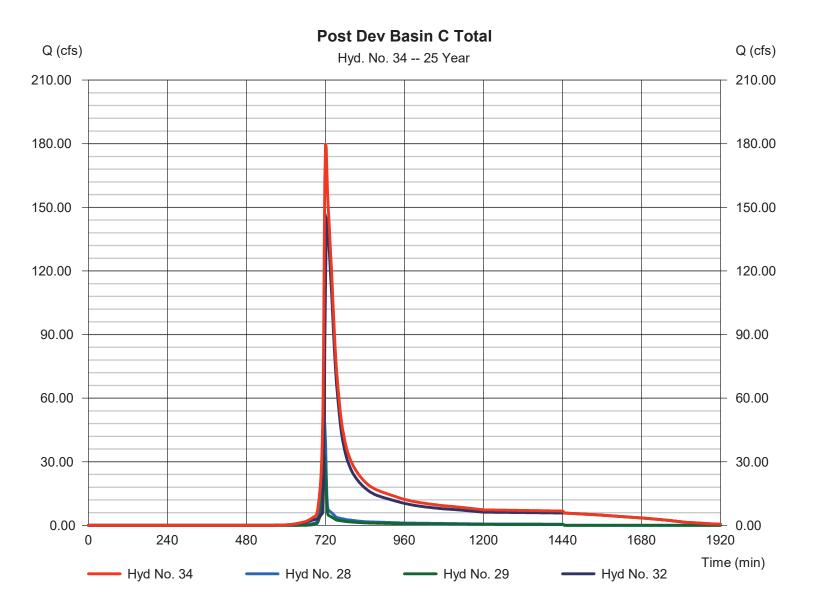
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Friday, 01 / 31 / 2025

Hyd. No. 34

Post Dev Basin C Total

Hydrograph type = Combine Peak discharge = 180.05 cfsStorm frequency Time to peak = 25 yrs= 720 min Time interval = 2 min Hyd. volume = 972,552 cuft Inflow hyds. = 28, 29, 32 Contrib. drain. area = 21.660 ac



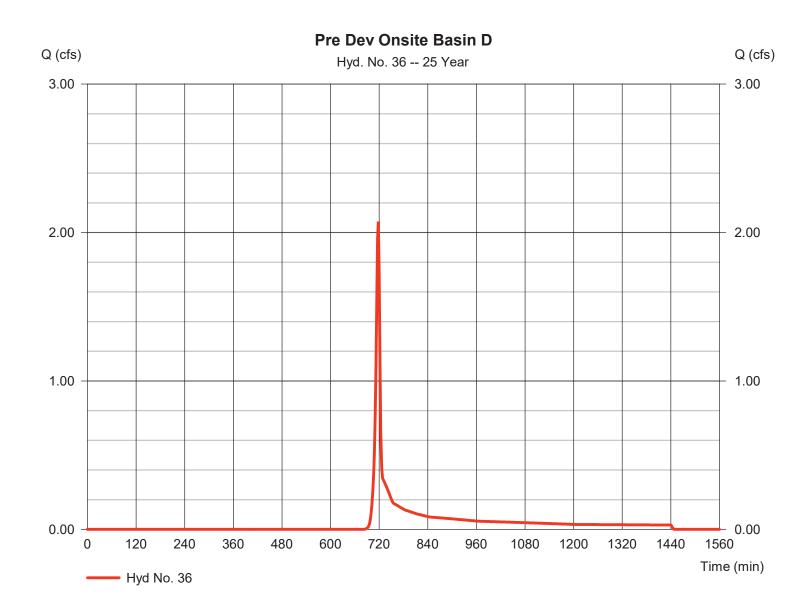
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Friday, 01 / 31 / 2025

Hyd. No. 36

Pre Dev Onsite Basin D

Hydrograph type = SCS Runoff Peak discharge = 2.073 cfsStorm frequency = 25 yrs Time to peak = 718 min Time interval = 2 min Hyd. volume = 4,220 cuftDrainage area = 0.770 acCurve number = 55 Basin Slope = 0.0 %Hydraulic length = 0 ftTc method Time of conc. (Tc) $= 5.00 \, \text{min}$ = User Total precip. = 6.16 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



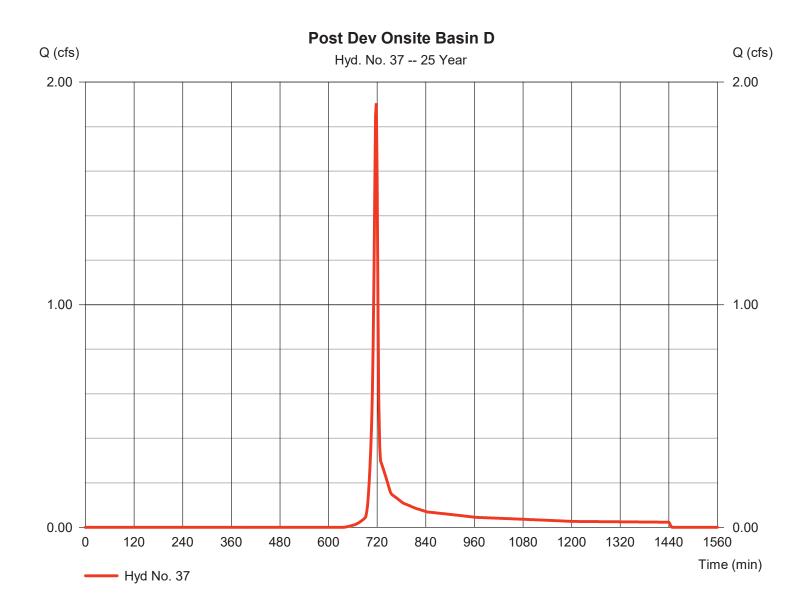
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Friday, 01 / 31 / 2025

Hyd. No. 37

Post Dev Onsite Basin D

Hydrograph type = SCS Runoff Peak discharge = 1.905 cfsStorm frequency = 25 yrsTime to peak = 718 min Time interval = 2 min Hyd. volume = 3.812 cuft Drainage area = 0.530 acCurve number = 61 Basin Slope = 0.0 %Hydraulic length = 0 ftTc method Time of conc. (Tc) $= 5.00 \, \text{min}$ = User Total precip. = 6.16 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



Hydrograph Summary Report

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

Hyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description
1	SCS Runoff	104.40	2	728	388,055				Pre Dev Onsite Basin A
2	SCS Runoff	30.01	2	722	78,940				Pre Dev Offite Basin A1
3	SCS Runoff	2.319	2	720	5,346				Pre Dev Offsite Basin A2
5	Combine	127.51	2	726	472,340	1, 2, 3,			Pre Dev Basin A Total
7	SCS Runoff	303.35	2	716	621,561				Post Dev Onsite Basin A to Pond 1
8	SCS Runoff	6.678	2	722	17,670				Post Dev Offsite Basin A to Pond 1
9	SCS Runoff	24.37	2	720	55,937				Post Dev Offsite Basin A1 Bypass
10	SCS Runoff	2.319	2	720	5,346				Post Dev Offsite Basin A2 Bypass
12	Combine	308.25	2	716	639,231	7, 8,			Post Dev Basin A to Pond 1
13	Reservoir	96.62	2	724	623,259	12	1062.49	290,706	Post Dev Pond 1 Routed
15	Combine	115.76	2	722	684,543	9, 10, 13,			Post Dev Basin A Total
17	SCS Runoff	10.74	2	730	42,613				Pre Dev Onsite Basin B
18	SCS Runoff	36.98	2	720	96,844				Post Dev Onsite Basin B
20	SCS Runoff	190.30	2	726	635,133				Pre Dev Onsite Basin C
21	SCS Runoff	39.91	2	718	79,797				Pre Dev Offsite Basin C1
22	SCS Runoff	92.53	2	726	303,920				Pre Dev Offsite Basin C2
24	Combine	291.06	2	724	1,018,850	20, 21, 22,			Pre Dev Basin C Total
26	SCS Runoff	349.68	2	716	707,468				Post Dev Onsite Basin C to Pond 2
27	SCS Runoff	92.53	2	726	303,920				Post Dev Offsite Basin C to Pond 2
28	SCS Runoff	58.89	2	718	117,875				Post Dev Onsite Basin C Bypass
29	SCS Runoff	38.39	2	718	76,788				Post Dev Offsite Basin C Bypass
31	Combine	410.75	2	718	1,011,387	26, 27,			Post Dev Basin C to Pond 2
32	Reservoir	201.97	2	724	1,011,368	31	1112.13	291,046	Post Dev Pond 2 Routed
34	Combine	257.73	2	720	1,206,030	28, 29, 32,			Post Dev Basin C Total
36	SCS Runoff	2.725	2	718	5,482				Pre Dev Onsite Basin D
37	SCS Runoff	2.404	2	718	4,808				Post Dev Onsite Basin D
1220263 Hydrology Concept.gpw					Return P	Return Period: 50 Year			/ 31 / 2025

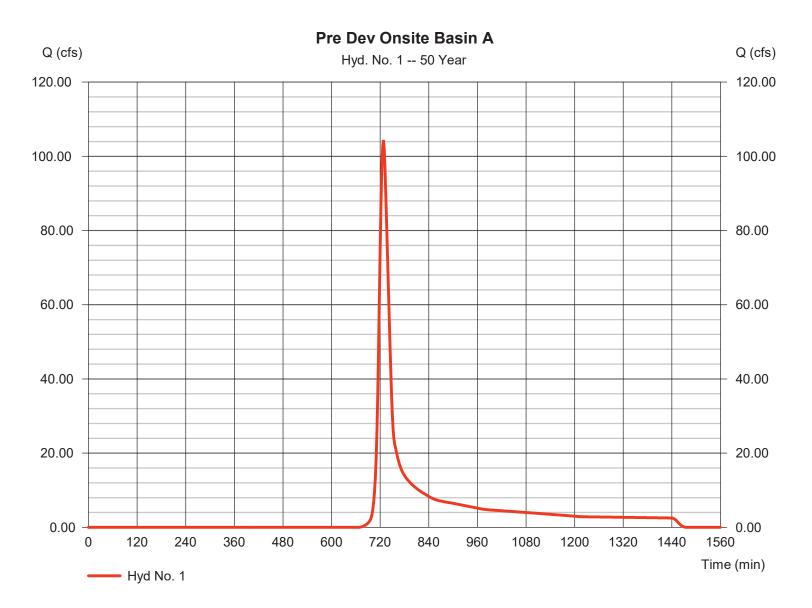
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Friday, 01 / 31 / 2025

Hyd. No. 1

Pre Dev Onsite Basin A

Hydrograph type = SCS Runoff Peak discharge = 104.40 cfsStorm frequency = 50 yrsTime to peak = 728 min Time interval = 2 min Hyd. volume = 388.055 cuft Drainage area Curve number = 50.200 ac= 55 Basin Slope = 0.0 %Hydraulic length = 0 ftTc method Time of conc. (Tc) = 22.00 min = User Total precip. = 6.95 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



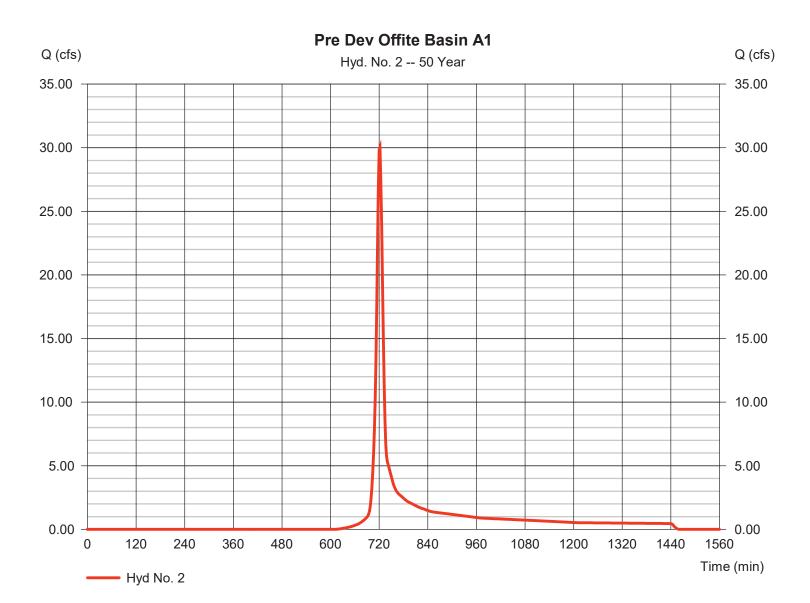
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Friday, 01 / 31 / 2025

Hyd. No. 2

Pre Dev Offite Basin A1

Hydrograph type = SCS Runoff Peak discharge = 30.01 cfsStorm frequency = 50 yrsTime to peak = 722 min Time interval = 2 min Hyd. volume = 78.940 cuftDrainage area = 7.910 acCurve number = 61 Hydraulic length Basin Slope = 0.0 %= 0 ftTc method Time of conc. (Tc) = 12.00 min = User Total precip. = 6.95 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



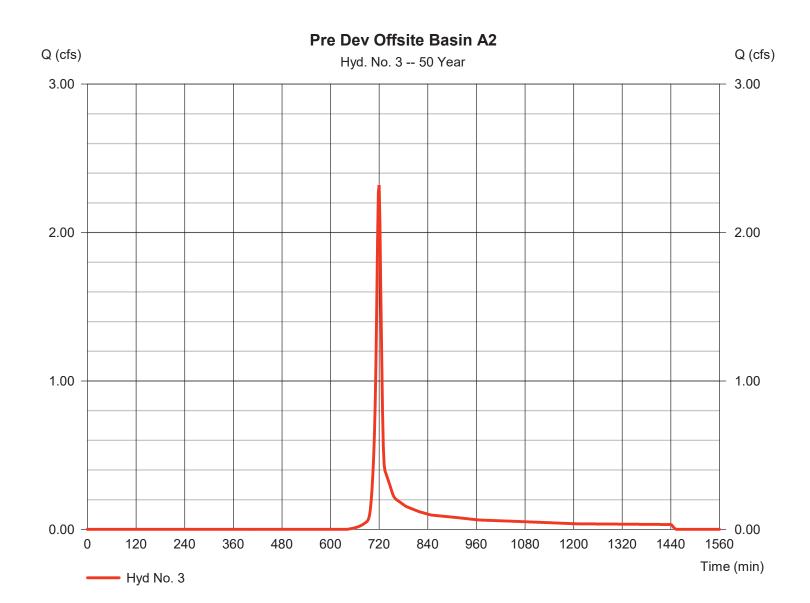
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Friday, 01 / 31 / 2025

Hyd. No. 3

Pre Dev Offsite Basin A2

Hydrograph type = SCS Runoff Peak discharge = 2.319 cfsStorm frequency = 50 yrsTime to peak = 720 min Time interval = 2 min Hyd. volume = 5,346 cuftDrainage area Curve number = 0.620 ac= 58 Basin Slope = 0.0 %Hydraulic length = 0 ftTc method Time of conc. (Tc) $= 8.00 \, \text{min}$ = User Total precip. = 6.95 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



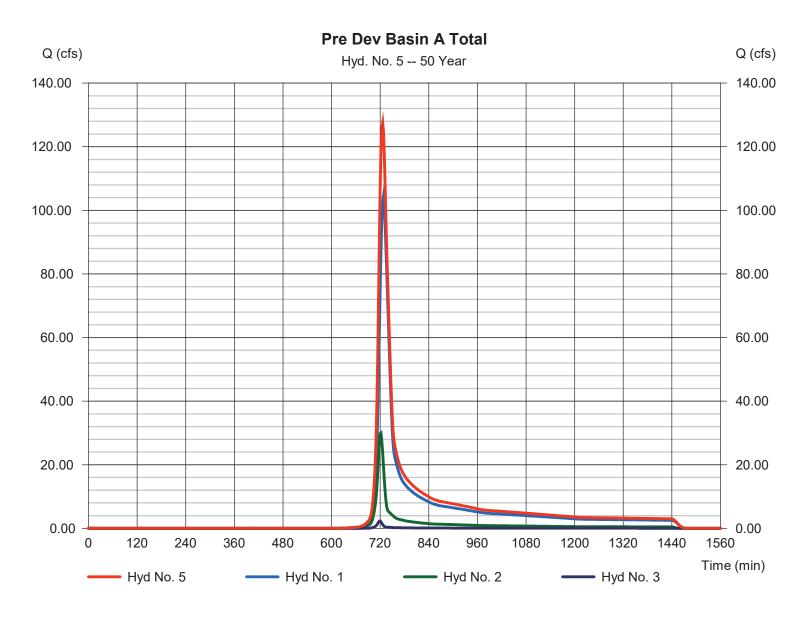
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Friday, 01 / 31 / 2025

Hyd. No. 5

Pre Dev Basin A Total

Hydrograph type = Combine Peak discharge = 127.51 cfsStorm frequency Time to peak = 50 yrs= 726 min Time interval = 2 min Hyd. volume = 472,340 cuft Inflow hyds. = 1, 2, 3Contrib. drain. area = 58.730 ac



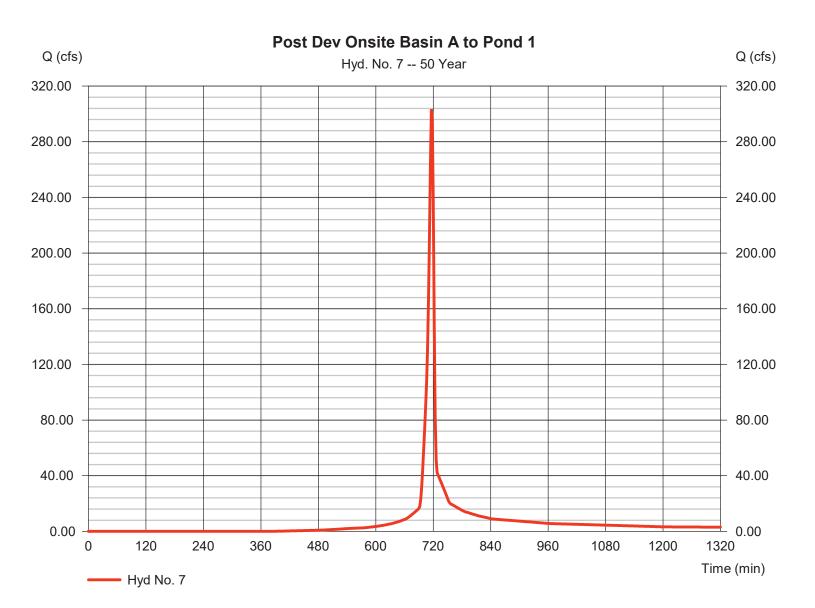
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Friday, 01 / 31 / 2025

Hyd. No. 7

Post Dev Onsite Basin A to Pond 1

Hydrograph type = SCS Runoff Peak discharge = 303.35 cfsStorm frequency Time to peak = 50 yrs= 716 min Time interval = 2 min Hyd. volume = 621,561 cuft Drainage area = 42.270 ac Curve number = 77 Hydraulic length = 0 ftBasin Slope = 0.0 % Tc method Time of conc. (Tc) $= 5.00 \, \text{min}$ = User Total precip. = 6.95 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



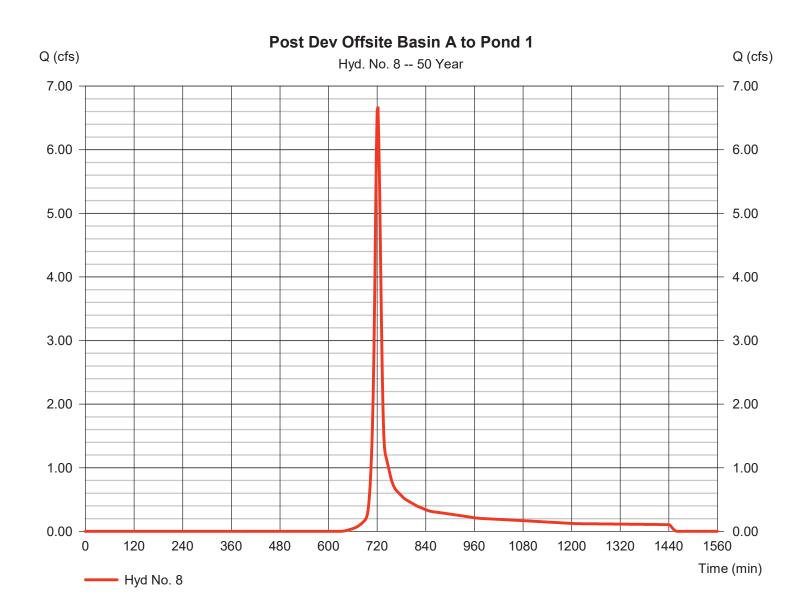
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Friday, 01 / 31 / 2025

Hyd. No. 8

Post Dev Offsite Basin A to Pond 1

Hydrograph type = SCS Runoff Peak discharge = 6.678 cfsStorm frequency = 50 yrsTime to peak = 722 min Time interval = 2 min Hyd. volume = 17,670 cuftDrainage area Curve number = 1.910 ac= 59 Hydraulic length Basin Slope = 0.0 %= 0 ftTc method Time of conc. (Tc) = 12.00 min = User Total precip. = 6.95 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



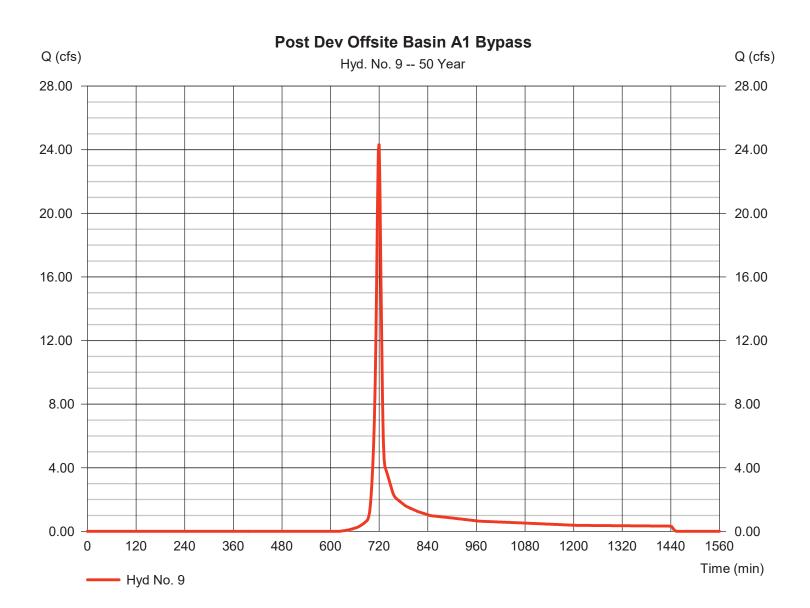
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Friday, 01 / 31 / 2025

Hyd. No. 9

Post Dev Offsite Basin A1 Bypass

Hydrograph type = SCS Runoff Peak discharge = 24.37 cfsStorm frequency = 50 yrsTime to peak = 720 min Time interval = 2 min Hyd. volume = 55,937 cuftDrainage area Curve number = 6.000 ac= 60 Hydraulic length Basin Slope = 0.0 %= 0 ftTc method Time of conc. (Tc) = 8.00 min = User Total precip. = 6.95 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



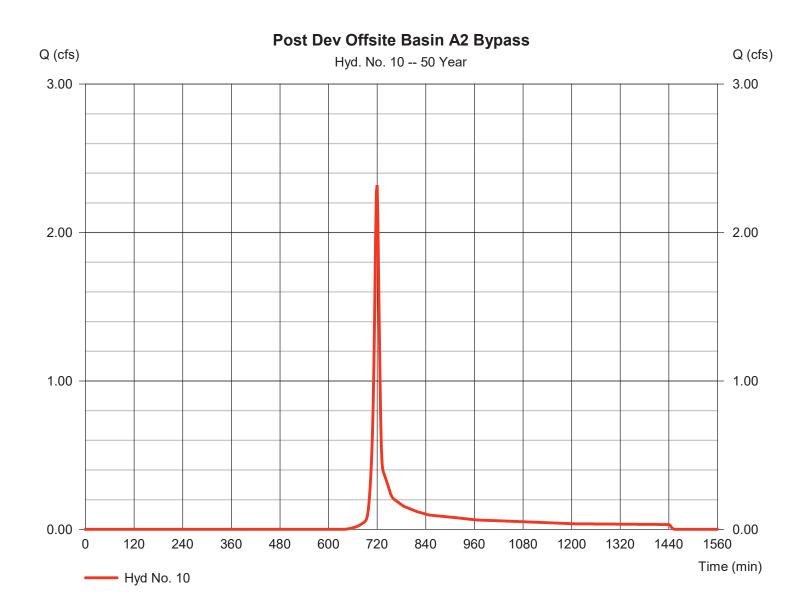
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Friday, 01 / 31 / 2025

Hyd. No. 10

Post Dev Offsite Basin A2 Bypass

Hydrograph type = SCS Runoff Peak discharge = 2.319 cfsStorm frequency = 50 yrsTime to peak = 720 min Time interval = 2 min Hyd. volume = 5,346 cuftDrainage area = 0.620 acCurve number = 58 Basin Slope = 0.0 %Hydraulic length = 0 ftTc method Time of conc. (Tc) = 8.00 min = User Total precip. = 6.95 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



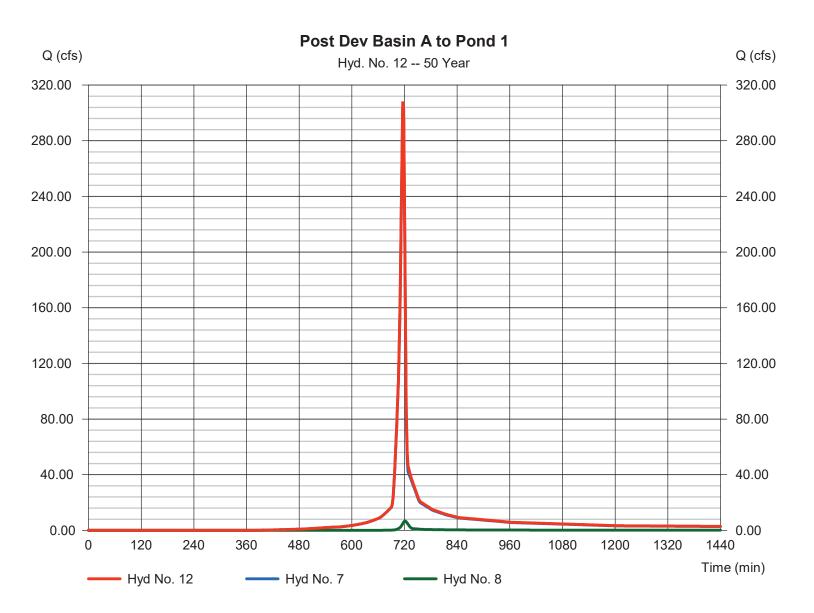
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Friday, 01 / 31 / 2025

Hyd. No. 12

Post Dev Basin A to Pond 1

Hydrograph type = Combine Peak discharge = 308.25 cfsStorm frequency Time to peak = 50 yrs= 716 min Time interval = 2 min Hyd. volume = 639,231 cuft Inflow hyds. = 7,8 Contrib. drain. area = 44.180 ac



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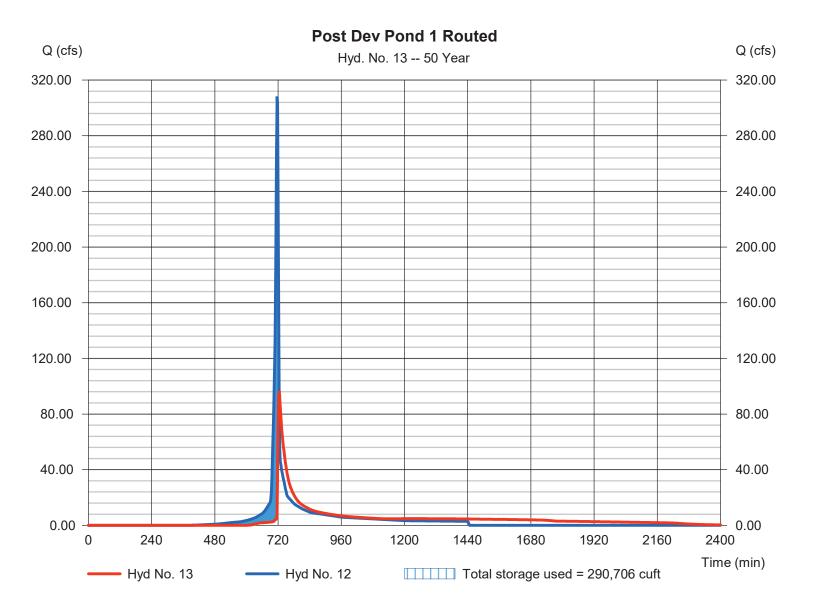
Friday, 01 / 31 / 2025

Hyd. No. 13

Post Dev Pond 1 Routed

Hydrograph type Peak discharge = 96.62 cfs= Reservoir Storm frequency Time to peak = 50 yrs= 724 min Time interval = 2 min Hyd. volume = 623,259 cuft Inflow hyd. No. = 12 - Post Dev Basin A to PondMax. Elevation = 1062.49 ftReservoir name = Pond 1 Max. Storage = 290,706 cuft

Storage Indication method used.



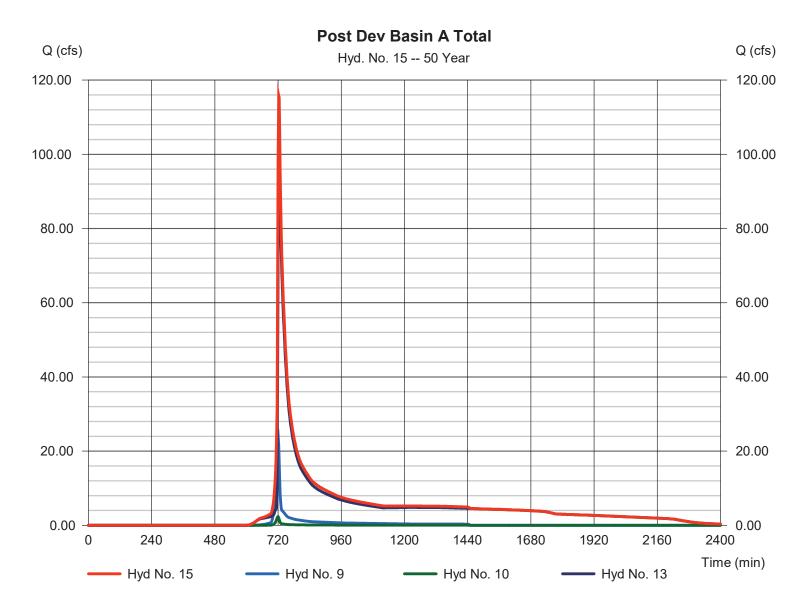
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Friday, 01 / 31 / 2025

Hyd. No. 15

Post Dev Basin A Total

Hydrograph type = Combine Peak discharge = 115.76 cfsStorm frequency Time to peak = 50 yrs= 722 min Time interval = 2 min Hyd. volume = 684,543 cuft Inflow hyds. = 9, 10, 13Contrib. drain. area = 6.620 ac



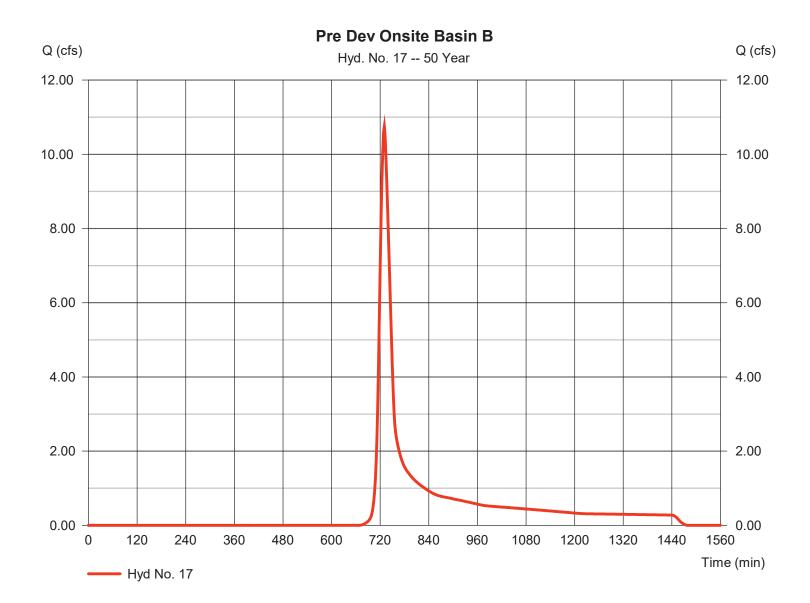
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Friday, 01 / 31 / 2025

Hyd. No. 17

Pre Dev Onsite Basin B

Hydrograph type = SCS Runoff Peak discharge = 10.74 cfsStorm frequency = 50 yrsTime to peak = 730 min Time interval = 2 min Hyd. volume = 42,613 cuftDrainage area Curve number = 5.700 ac= 55 Hydraulic length Basin Slope = 0.0 %= 0 ftTc method Time of conc. (Tc) = 25.00 min = User Total precip. = 6.95 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



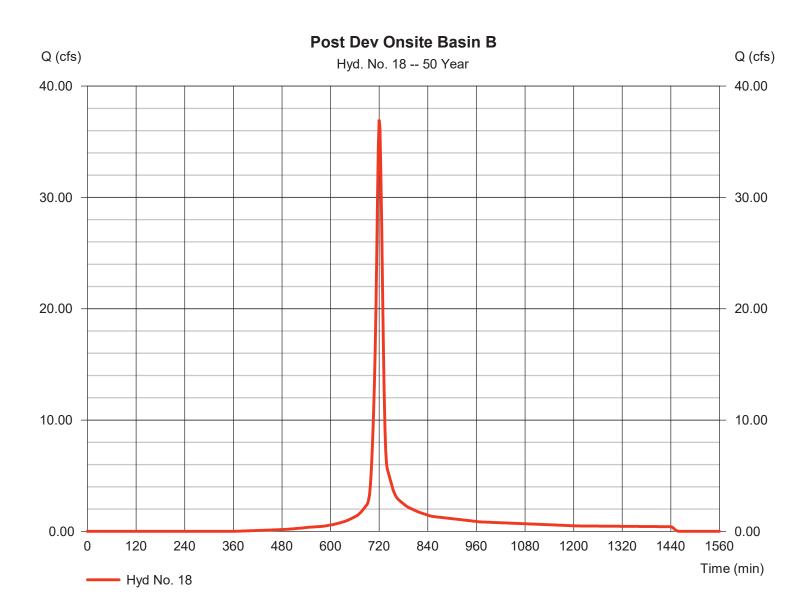
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Friday, 01 / 31 / 2025

Hyd. No. 18

Post Dev Onsite Basin B

Hydrograph type = SCS Runoff Peak discharge = 36.98 cfsStorm frequency = 50 yrsTime to peak = 720 min Time interval = 2 min Hyd. volume = 96.844 cuft Drainage area = 5.700 acCurve number = 79 Hydraulic length Basin Slope = 0.0 %= 0 ftTc method Time of conc. (Tc) = 10.00 min = User Total precip. = 6.95 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



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Friday, 01 / 31 / 2025

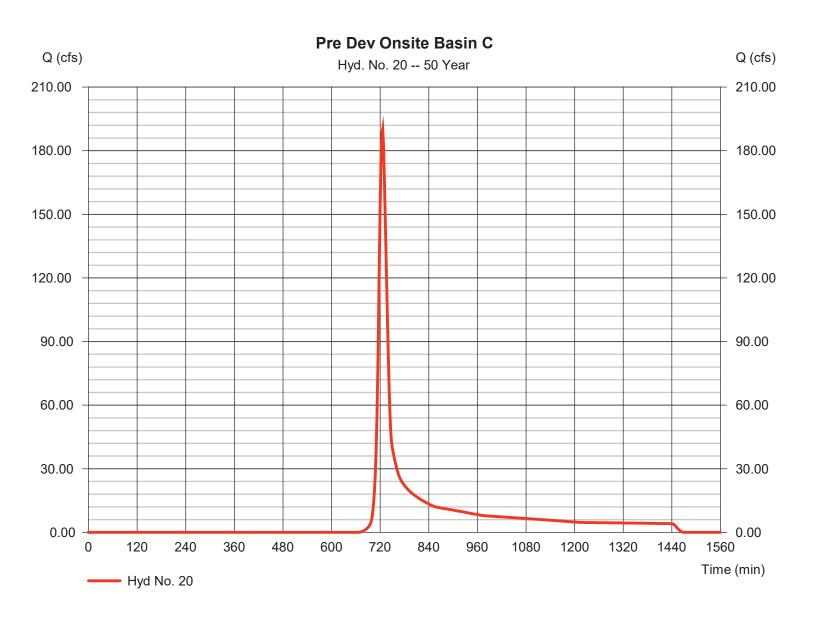
Hyd. No. 20

Pre Dev Onsite Basin C

Hydrograph type = SCS Runoff Peak discharge = 190.30 cfsStorm frequency = 50 yrsTime to peak = 726 min Time interval = 2 min Hyd. volume = 635,133 cuft Drainage area Curve number = 83.630 ac = 55

Basin Slope = 0.0 % Hydraulic length = 0 ft
Tc method = User Time of conc. (Tc) = 17.00 min
Total precip. = 6.95 in Distribution = Type II

Storm duration = 24 hrs Shape factor = 484



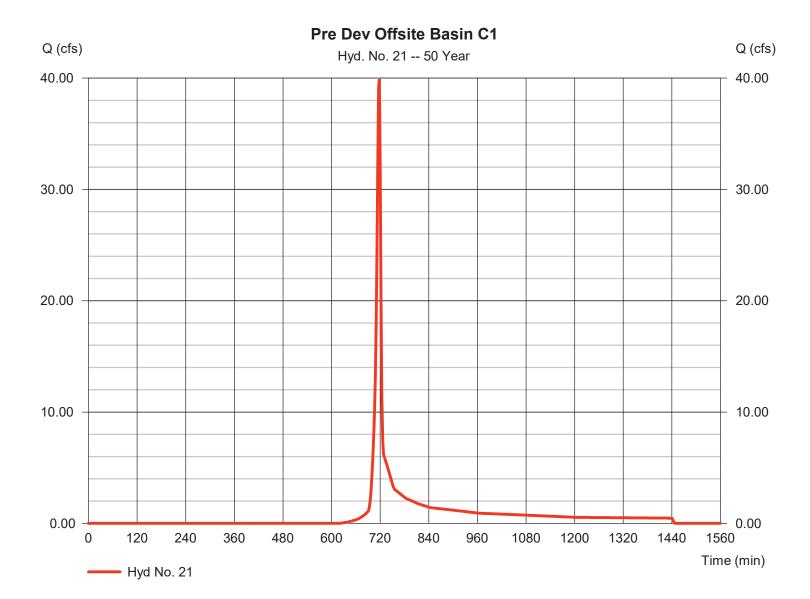
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Friday, 01 / 31 / 2025

Hyd. No. 21

Pre Dev Offsite Basin C1

Hydrograph type = SCS Runoff Peak discharge = 39.91 cfsStorm frequency = 50 yrsTime to peak = 718 min Time interval = 2 min Hyd. volume = 79,797 cuft Drainage area = 9.130 acCurve number = 60 Hydraulic length Basin Slope = 0.0 %= 0 ftTc method Time of conc. (Tc) $= 6.00 \, \text{min}$ = User Total precip. = 6.95 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



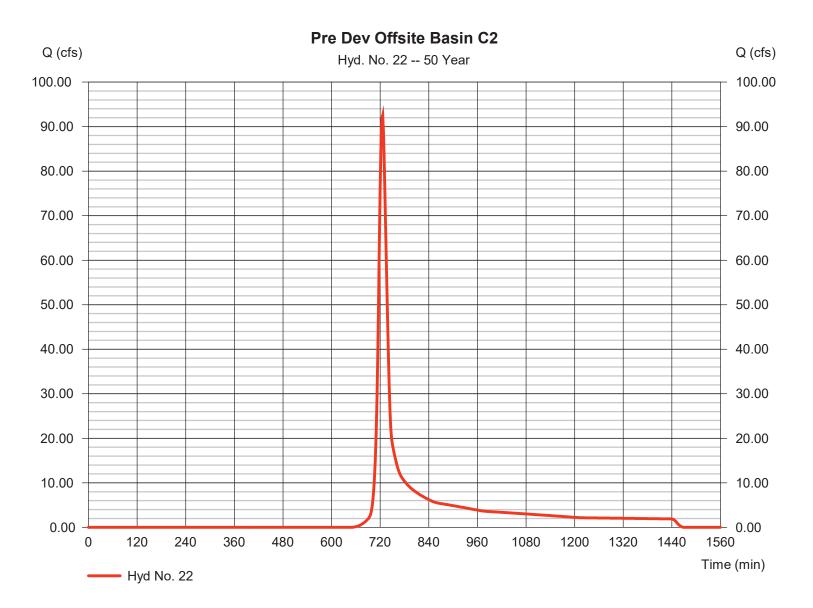
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Friday, 01 / 31 / 2025

Hyd. No. 22

Pre Dev Offsite Basin C2

Hydrograph type = SCS Runoff Peak discharge = 92.53 cfsStorm frequency Time to peak = 50 yrs= 726 min Time interval = 2 min Hyd. volume = 303,920 cuft = 36.720 ac Drainage area Curve number = 57 Hydraulic length Basin Slope = 0.0 %= 0 ftTc method Time of conc. (Tc) = 18.00 min = User Total precip. = 6.95 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



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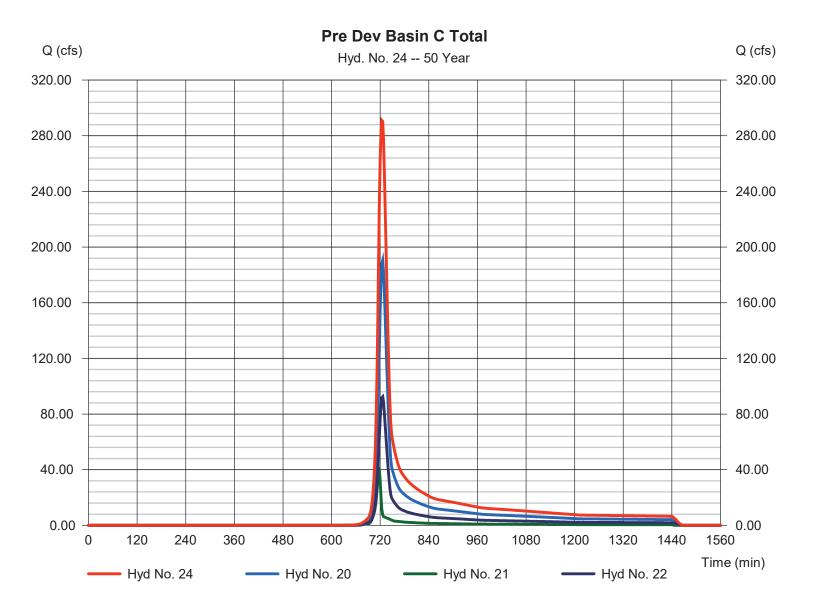
Friday, 01 / 31 / 2025

Hyd. No. 24

Pre Dev Basin C Total

Hydrograph type= CombinePeak discharge= 291.06 cfsStorm frequency= 50 yrsTime to peak= 724 minTime interval= 2 minHyd. volume= 1,018,850 cuft

Inflow hyds. = 2 min Hyd. volume = 1,018,850 c Contrib. drain. area = 129.480 ac



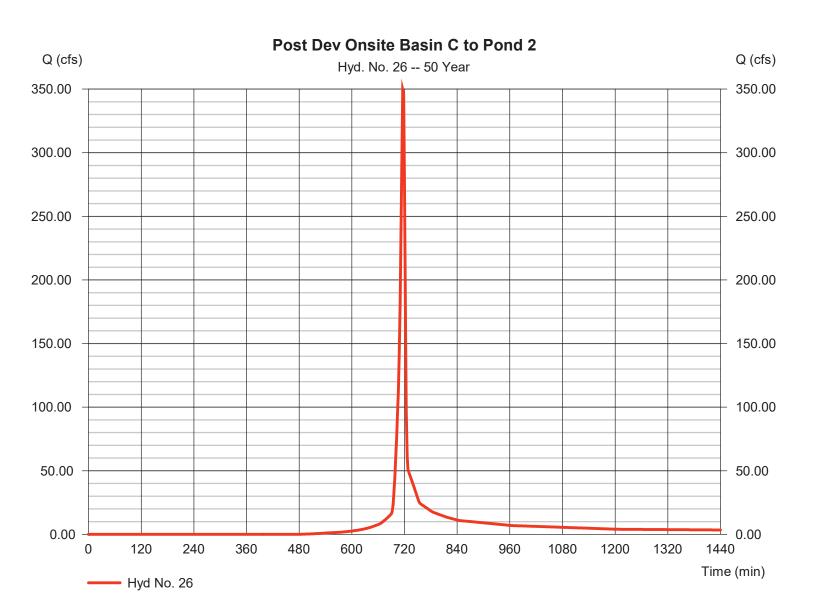
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Friday, 01 / 31 / 2025

Hyd. No. 26

Post Dev Onsite Basin C to Pond 2

Hydrograph type = SCS Runoff Peak discharge = 349.68 cfsStorm frequency Time to peak = 50 yrs= 716 min Time interval = 2 min Hyd. volume = 707.468 cuft Drainage area = 56.470 ac Curve number = 71 Basin Slope Hydraulic length = 0 ft= 0.0 %Tc method Time of conc. (Tc) $= 5.00 \, \text{min}$ = User Total precip. = 6.95 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



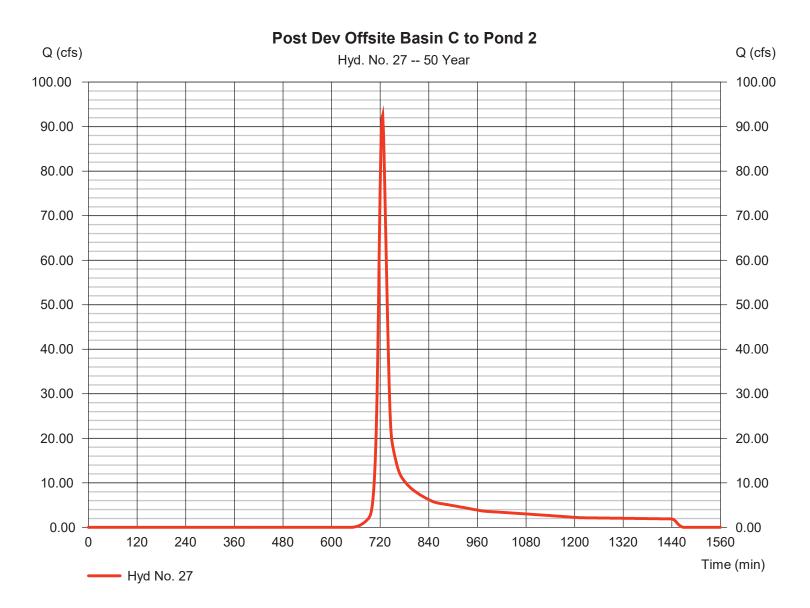
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Friday, 01 / 31 / 2025

Hyd. No. 27

Post Dev Offsite Basin C to Pond 2

Hydrograph type = SCS Runoff Peak discharge = 92.53 cfsStorm frequency Time to peak = 50 yrs= 726 min = 303,920 cuft Time interval = 2 min Hyd. volume Drainage area = 36.720 ac Curve number = 57 Hydraulic length Basin Slope = 0.0 %= 0 ftTc method Time of conc. (Tc) = 18.00 min = User Total precip. = 6.95 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



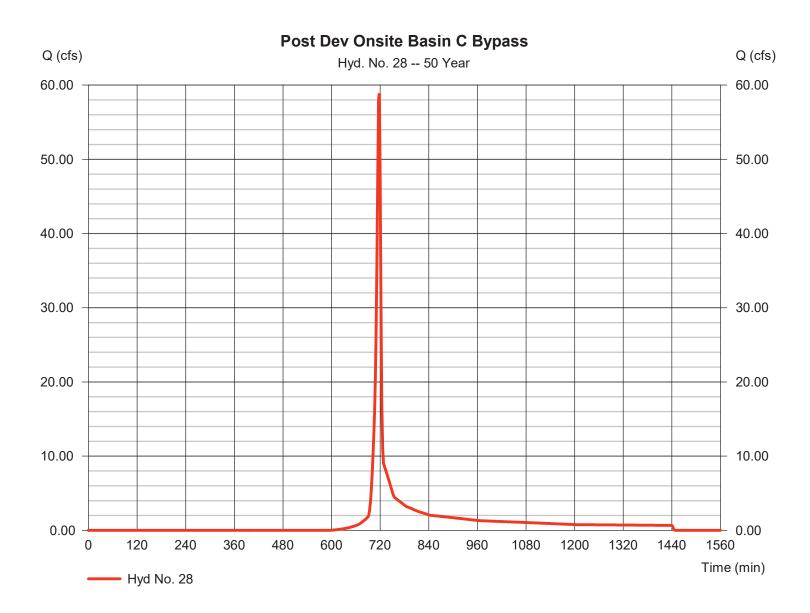
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Friday, 01 / 31 / 2025

Hyd. No. 28

Post Dev Onsite Basin C Bypass

Hydrograph type = SCS Runoff Peak discharge = 58.89 cfsStorm frequency = 50 yrsTime to peak = 718 min Time interval = 2 min Hyd. volume = 117,875 cuft Drainage area = 12.530 acCurve number = 62 = 0 ftBasin Slope = 0.0 %Hydraulic length Tc method Time of conc. (Tc) $= 5.00 \, \text{min}$ = User Total precip. = 6.95 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



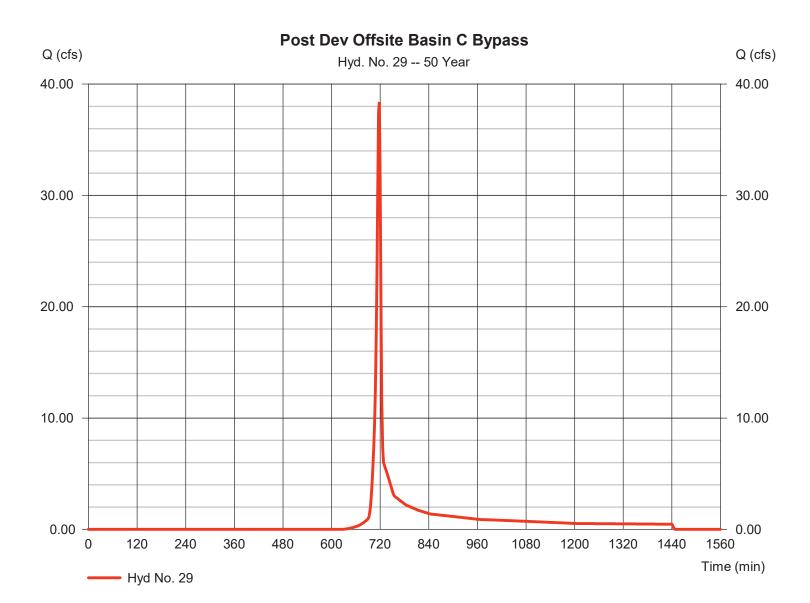
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Friday, 01 / 31 / 2025

Hyd. No. 29

Post Dev Offsite Basin C Bypass

Hydrograph type = SCS Runoff Peak discharge = 38.39 cfsStorm frequency = 50 yrsTime to peak = 718 min Time interval = 2 min Hyd. volume = 76.788 cuft Drainage area = 9.130 acCurve number = 59 Hydraulic length Basin Slope = 0.0 %= 0 ftTc method Time of conc. (Tc) = 6.00 min = User Total precip. = 6.95 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



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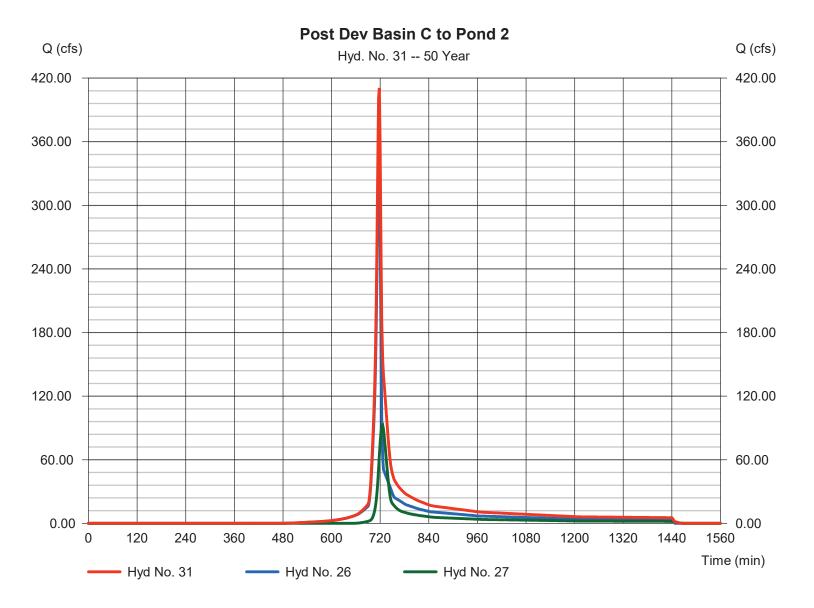
Friday, 01 / 31 / 2025

Hyd. No. 31

Post Dev Basin C to Pond 2

Hydrograph type= CombinePeak discharge= 410.75 cfsStorm frequency= 50 yrsTime to peak= 718 minTime interval= 2 minHyd. volume= 1,011,387 cuft

Inflow hyds. = 26, 27 Contrib. drain. area = 93.190 ac



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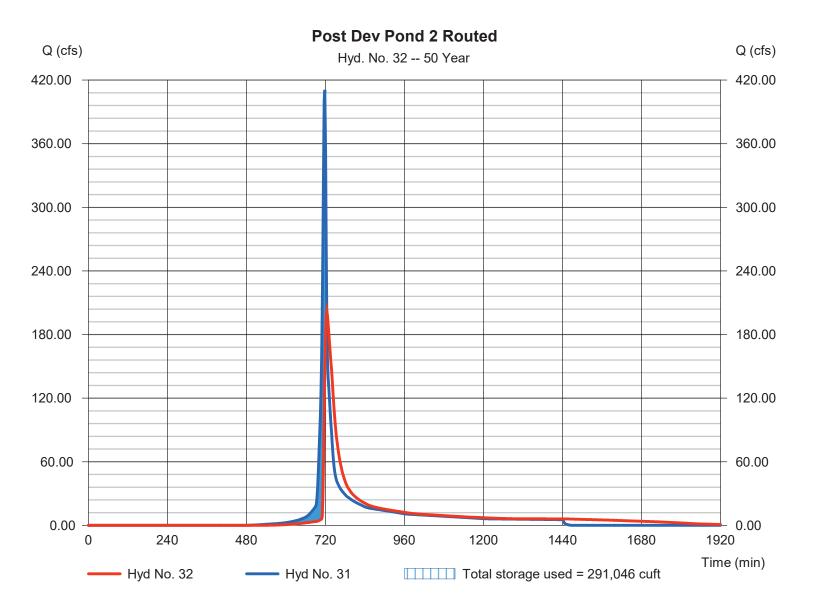
Hyd. No. 32

Post Dev Pond 2 Routed

Hydrograph type = Reservoir Peak discharge = 201.97 cfs Storm frequency = 50 yrs Time to peak = 724 min

Time interval = 2 min Hyd. volume = 1,011,368 cuft | 1,011,368 cuft | 1,011,368 cuft | 2 min Hyd. volume = 1,011,368 cuft | 1,011,368 cuft | 2 min Hyd. volume = 1,011,368 cuft | 1,011,368 cuft | 2 min Hyd. volume = 2,011,368 cuft | 2

Storage Indication method used.



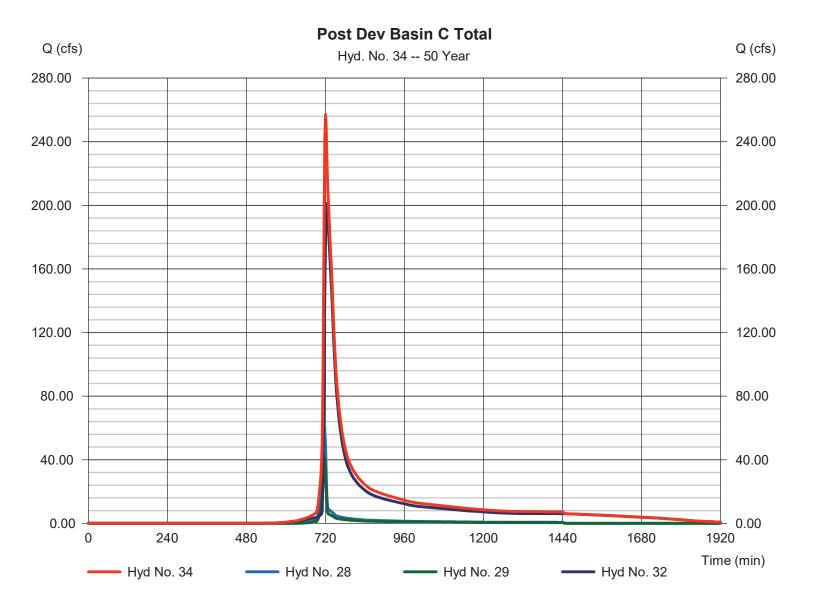
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Friday, 01 / 31 / 2025

Hyd. No. 34

Post Dev Basin C Total

Hydrograph type= CombinePeak discharge= 257.73 cfsStorm frequency= 50 yrsTime to peak= 720 minTime interval= 2 minHyd. volume= 1,206,030 cuft



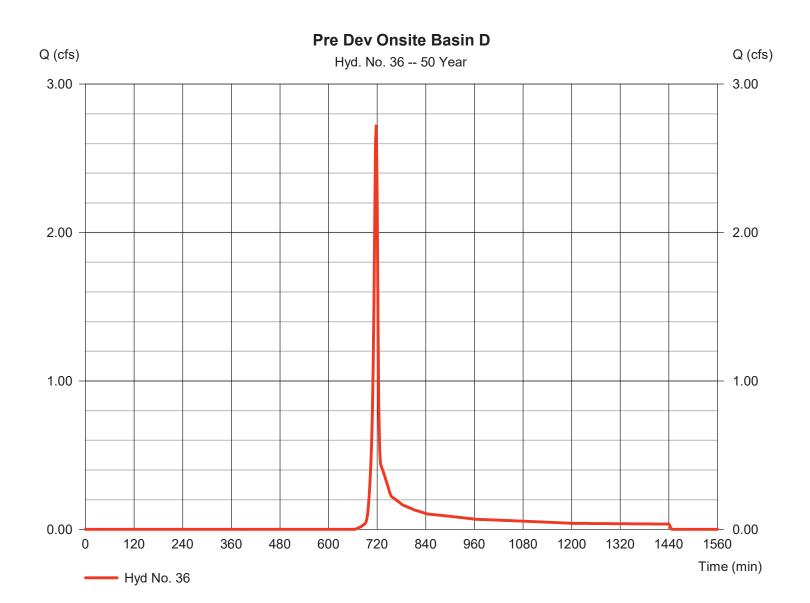
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Friday, 01 / 31 / 2025

Hyd. No. 36

Pre Dev Onsite Basin D

Hydrograph type = SCS Runoff Peak discharge = 2.725 cfsStorm frequency = 50 yrsTime to peak = 718 min Time interval = 2 min Hyd. volume = 5,482 cuftDrainage area Curve number = 0.770 ac= 55 Basin Slope = 0.0 %Hydraulic length = 0 ftTc method Time of conc. (Tc) $= 5.00 \, \text{min}$ = User Total precip. = 6.95 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



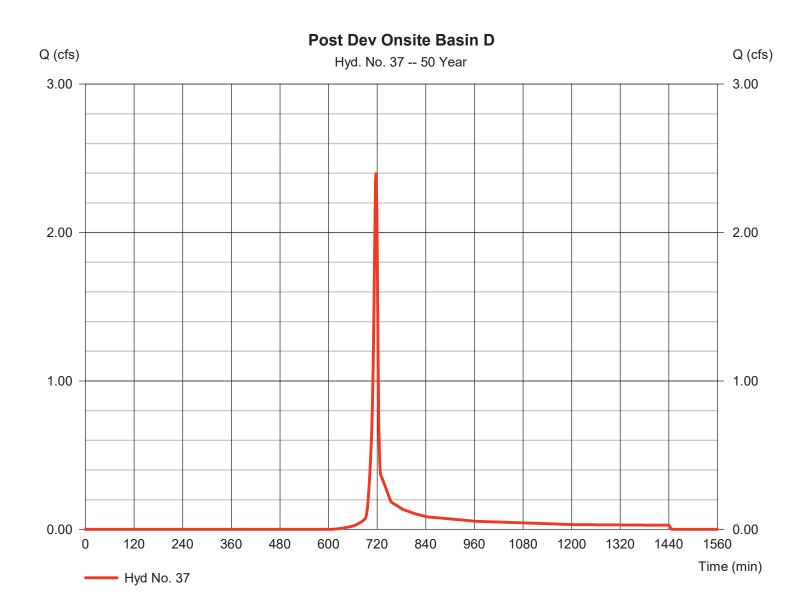
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Friday, 01 / 31 / 2025

Hyd. No. 37

Post Dev Onsite Basin D

Hydrograph type = SCS Runoff Peak discharge = 2.404 cfsStorm frequency = 50 yrsTime to peak = 718 min Time interval = 2 min Hyd. volume = 4,808 cuft Drainage area Curve number = 0.530 ac= 61 Basin Slope = 0.0 %Hydraulic length = 0 ftTc method Time of conc. (Tc) $= 5.00 \, \text{min}$ = User Total precip. = 6.95 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



Hydrograph Summary Report

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

Hyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description	
1	SCS Runoff	134.70	2	728	488,696				Pre Dev Onsite Basin A	
2	SCS Runoff	37.08	2	722	97,059				Pre Dev Offite Basin A1	
3	SCS Runoff	2.899	2	720	6,648				Pre Dev Offsite Basin A2	
5	Combine	163.69	2	726	592,403	1, 2, 3,			Pre Dev Basin A Total	
7	SCS Runoff	353.83	2	716	729,741				Post Dev Onsite Basin A to Pond 1	
8	SCS Runoff	8.334	2	722	21,887				Post Dev Offsite Basin A to Pond 1	
9	SCS Runoff	30.15	2	720	69,026				Post Dev Offsite Basin A1 Bypass	
10	SCS Runoff	2.899	2	720	6,648				Post Dev Offsite Basin A2 Bypass	
12	Combine	360.10	2	716	751,629	7, 8,			Post Dev Basin A to Pond 1	
13	Reservoir	148.59	2	722	735,656	12	1063.38	324,674	Post Dev Pond 1 Routed	
15	Combine	177.62	2	722	811,331	9, 10, 13,			Post Dev Basin A Total	
17	SCS Runoff	13.87	2	730	53,664				Pre Dev Onsite Basin B	
18	SCS Runoff	42.99	2	720	113,162				Post Dev Onsite Basin B	
20	SCS Runoff	244.76	2	726	799,853				Pre Dev Onsite Basin C	
21	SCS Runoff	49.17	2	718	98,470				Pre Dev Offsite Basin C1	
22	SCS Runoff	117.37	2	726	379,456				Pre Dev Offsite Basin C2	
24	Combine	374.08	2	724	1,277,780	20, 21, 22,			Pre Dev Basin C Total	
26	SCS Runoff	415.42	2	716	843,398				Post Dev Onsite Basin C to Pond 2	
27	SCS Runoff	117.37	2	726	379,456				Post Dev Offsite Basin C to Pond 2	
28	SCS Runoff	71.92	2	718	144,423				Post Dev Onsite Basin C Bypass	
29	SCS Runoff	47.53	2	718	95,113				Post Dev Offsite Basin C Bypass	
31	Combine	493.70	2	718	1,222,854	26, 27,			Post Dev Basin C to Pond 2	
32	Reservoir	265.72	2	724	1,222,834	31	1113.19	336,562	Post Dev Pond 2 Routed	
34	Combine	341.86	2	720	1,462,372	28, 29, 32,			Post Dev Basin C Total	
36	SCS Runoff	3.449	2	718	6,904				Pre Dev Onsite Basin D	
37	SCS Runoff	2.949	2	718	5,912				Post Dev Onsite Basin D	
122	L 20263 Hydrold	ogy Conce	ept.gpw		Return P	Return Period: 100 Year			Friday, 01 / 31 / 2025	

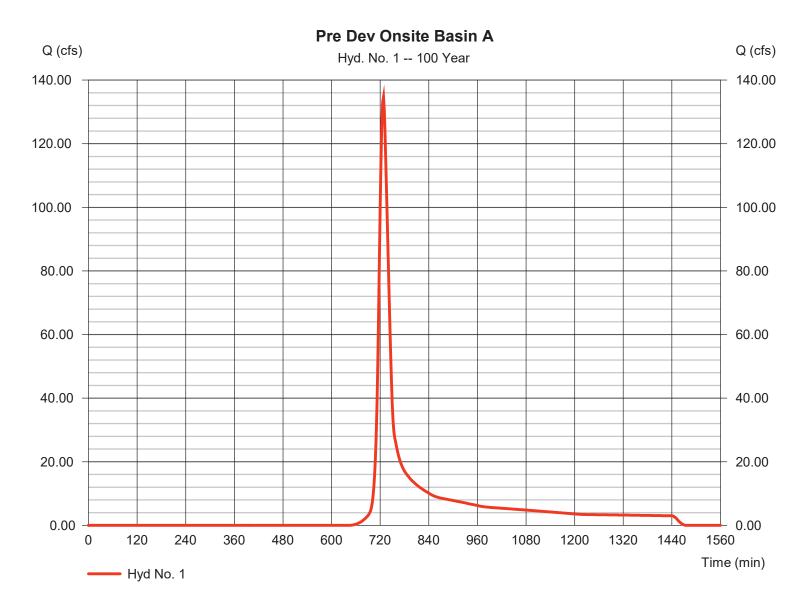
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Friday, 01 / 31 / 2025

Hyd. No. 1

Pre Dev Onsite Basin A

Hydrograph type = SCS Runoff Peak discharge = 134.70 cfsStorm frequency = 100 yrsTime to peak = 728 min Time interval = 2 min Hyd. volume = 488,696 cuft Drainage area Curve number = 50.200 ac= 55 Hydraulic length Basin Slope = 0.0 %= 0 ftTc method Time of conc. (Tc) = 22.00 min = User Total precip. = 7.78 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



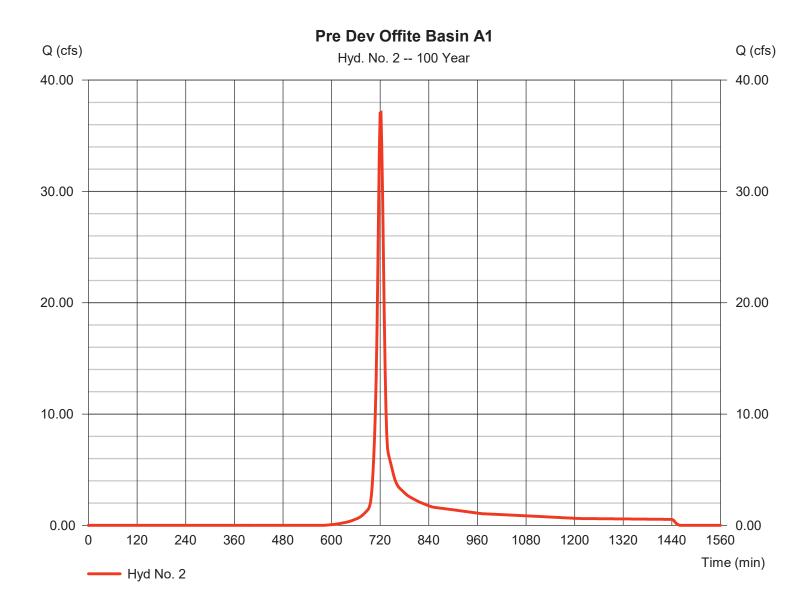
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Friday, 01 / 31 / 2025

Hyd. No. 2

Pre Dev Offite Basin A1

Hydrograph type = SCS Runoff Peak discharge = 37.08 cfsStorm frequency = 100 yrsTime to peak = 722 min Time interval = 2 min Hyd. volume = 97,059 cuftDrainage area = 7.910 acCurve number = 61 = 0.0 % Hydraulic length Basin Slope = 0 ftTc method Time of conc. (Tc) = 12.00 min = User Total precip. = 7.78 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



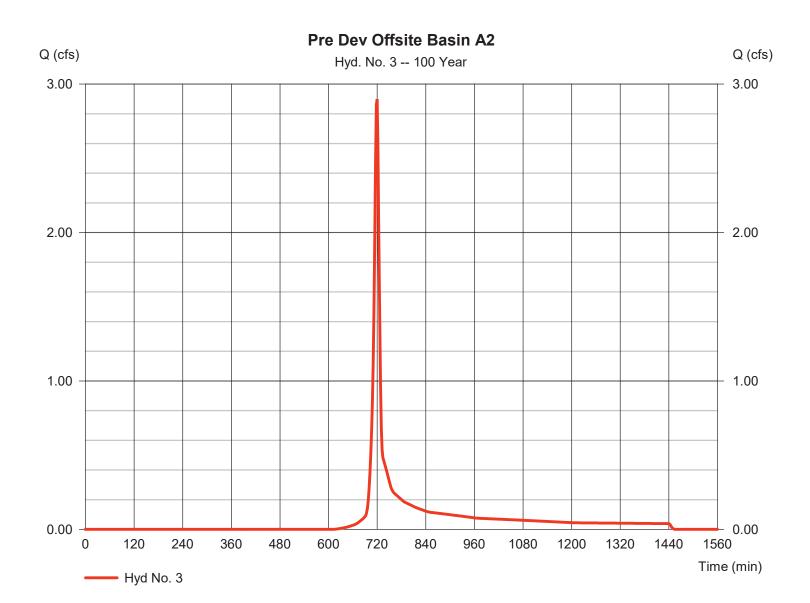
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Friday, 01 / 31 / 2025

Hyd. No. 3

Pre Dev Offsite Basin A2

Hydrograph type = SCS Runoff Peak discharge = 2.899 cfsStorm frequency = 100 yrsTime to peak = 720 min Time interval = 2 min Hyd. volume = 6,648 cuftDrainage area Curve number = 0.620 ac= 58 Basin Slope = 0.0 %Hydraulic length = 0 ftTc method Time of conc. (Tc) = 8.00 min = User Total precip. = 7.78 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



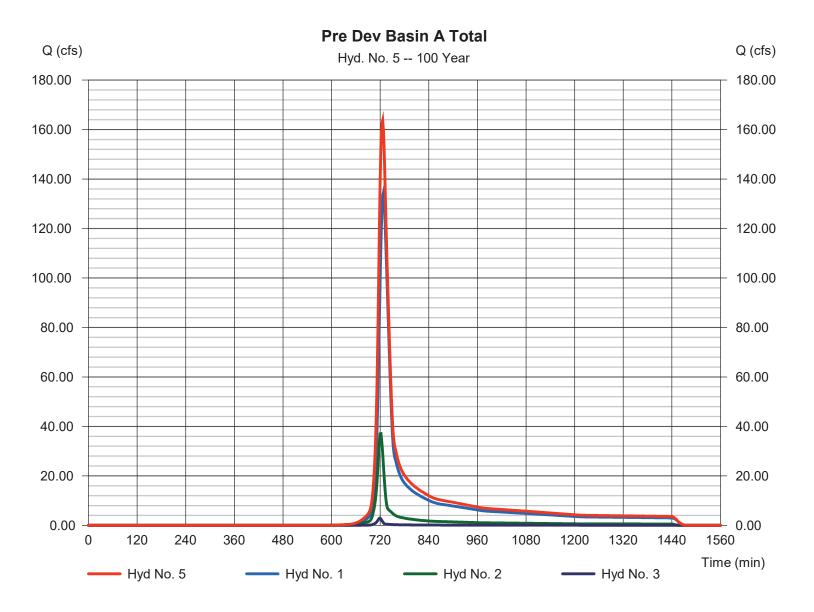
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Friday, 01 / 31 / 2025

Hyd. No. 5

Pre Dev Basin A Total

Hydrograph type = Combine Peak discharge = 163.69 cfsStorm frequency Time to peak = 100 yrs= 726 min Time interval = 2 min Hyd. volume = 592,403 cuft Inflow hyds. = 1, 2, 3Contrib. drain. area = 58.730 ac



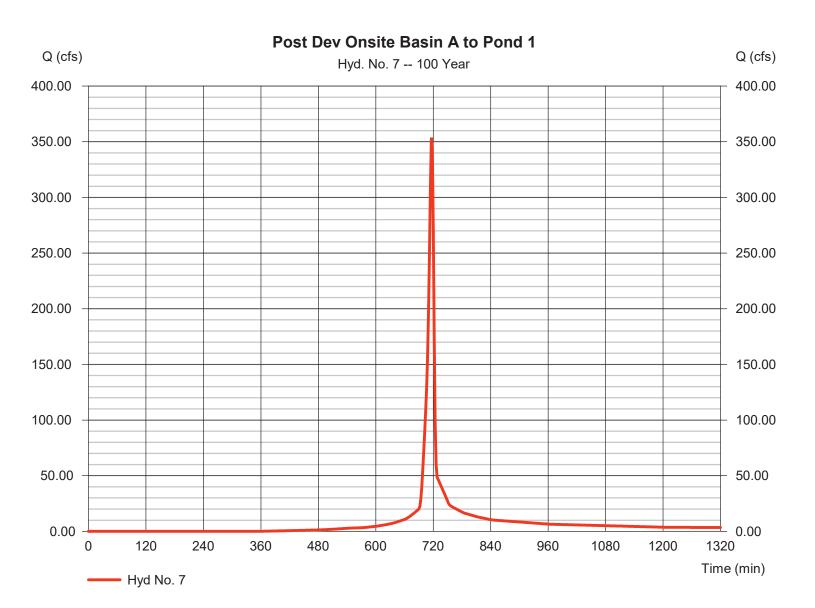
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Friday, 01 / 31 / 2025

Hyd. No. 7

Post Dev Onsite Basin A to Pond 1

Hydrograph type = SCS Runoff Peak discharge = 353.83 cfsStorm frequency Time to peak = 100 yrs= 716 min Time interval = 2 min Hyd. volume = 729,741 cuft Drainage area = 42.270 ac Curve number = 77 Hydraulic length = 0 ftBasin Slope = 0.0 % Tc method Time of conc. (Tc) $= 5.00 \, \text{min}$ = User Total precip. = 7.78 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



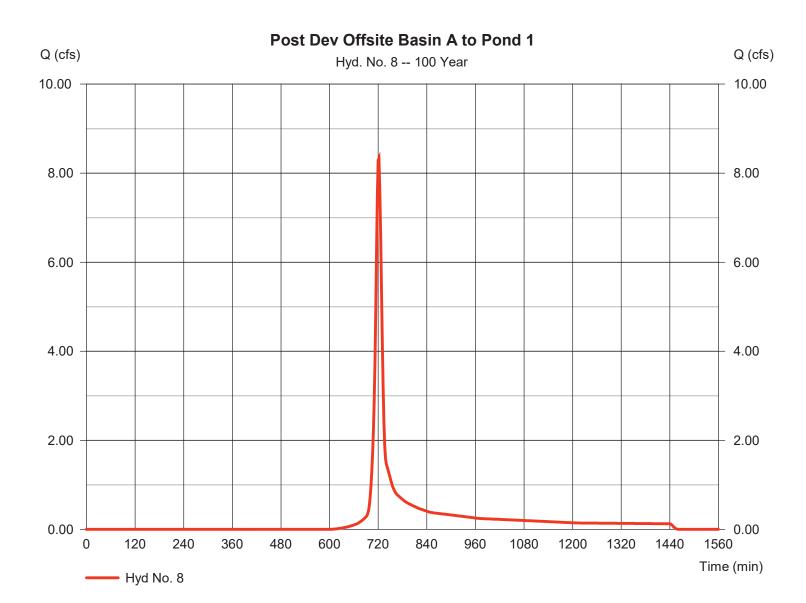
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Friday, 01 / 31 / 2025

Hyd. No. 8

Post Dev Offsite Basin A to Pond 1

Hydrograph type = SCS Runoff Peak discharge = 8.334 cfsStorm frequency = 100 yrsTime to peak = 722 min Time interval = 2 min Hyd. volume = 21.887 cuft Drainage area = 1.910 acCurve number = 59 Hydraulic length Basin Slope = 0.0 %= 0 ftTc method Time of conc. (Tc) = 12.00 min = User Total precip. = 7.78 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



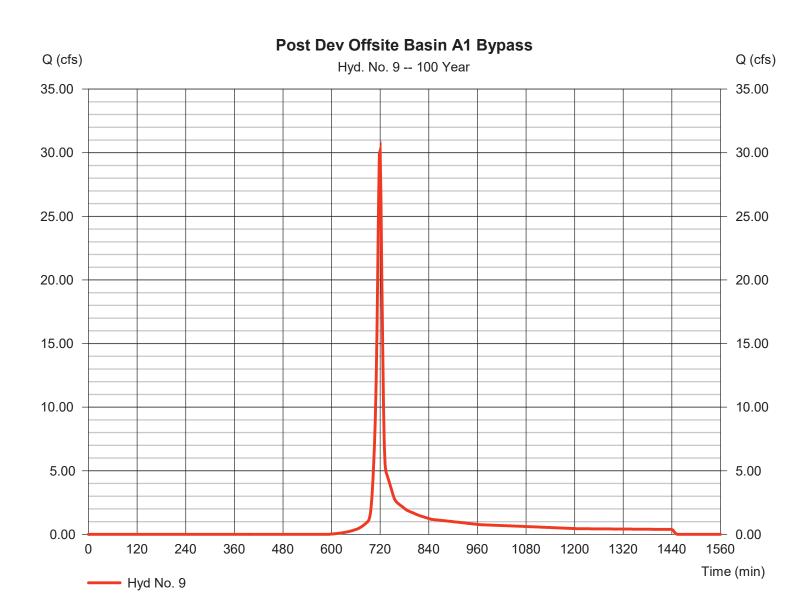
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Friday, 01 / 31 / 2025

Hyd. No. 9

Post Dev Offsite Basin A1 Bypass

Hydrograph type = SCS Runoff Peak discharge = 30.15 cfsStorm frequency Time to peak = 100 yrs= 720 min Time interval = 2 min Hyd. volume = 69.026 cuft Drainage area = 6.000 acCurve number = 60 Hydraulic length Basin Slope = 0.0 %= 0 ftTc method Time of conc. (Tc) = 8.00 min = User Total precip. = 7.78 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



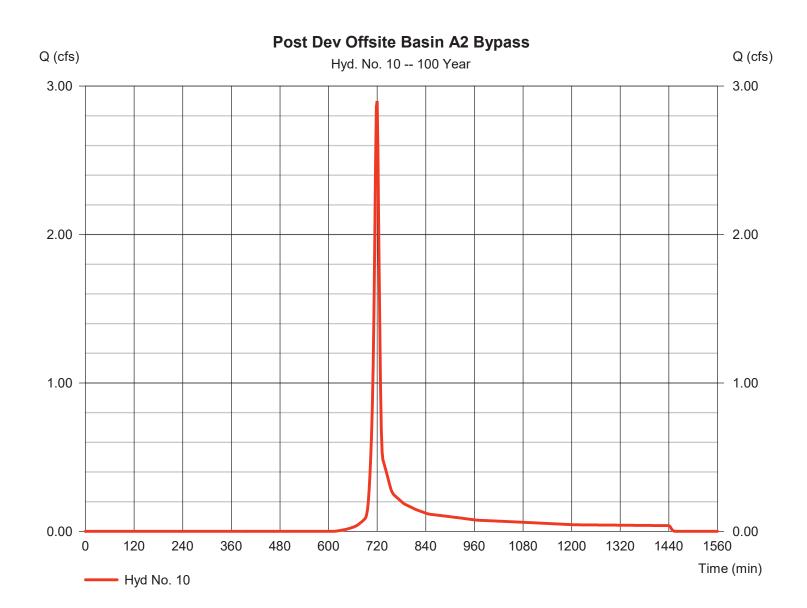
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Friday, 01 / 31 / 2025

Hyd. No. 10

Post Dev Offsite Basin A2 Bypass

Hydrograph type = SCS Runoff Peak discharge = 2.899 cfsStorm frequency = 100 yrsTime to peak = 720 min Time interval = 2 min Hyd. volume = 6,648 cuftDrainage area = 0.620 acCurve number = 58 Basin Slope = 0.0 %Hydraulic length = 0 ftTc method Time of conc. (Tc) = 8.00 min = User Total precip. = 7.78 inDistribution = Type II Shape factor Storm duration = 24 hrs = 484



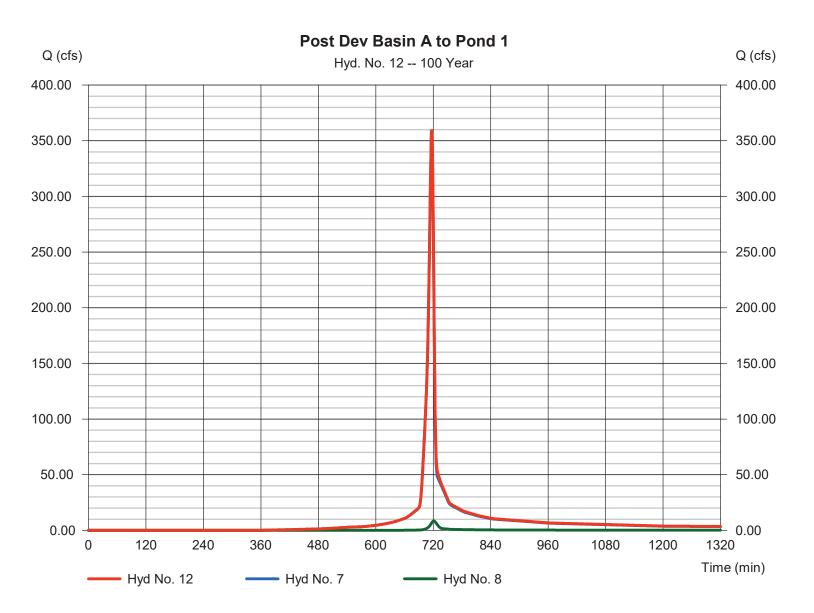
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Friday, 01 / 31 / 2025

Hyd. No. 12

Post Dev Basin A to Pond 1

Hydrograph type = Combine Peak discharge = 360.10 cfsStorm frequency Time to peak = 100 yrs= 716 min Time interval = 2 min Hyd. volume = 751,629 cuft Inflow hyds. = 7,8 Contrib. drain. area = 44.180 ac



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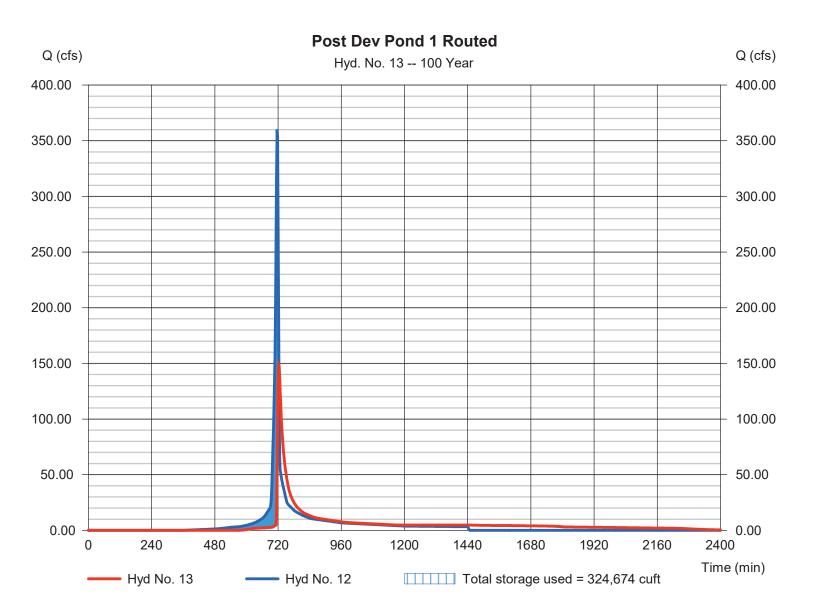
Friday, 01 / 31 / 2025

Hyd. No. 13

Post Dev Pond 1 Routed

Hydrograph type Peak discharge = 148.59 cfs= Reservoir Storm frequency Time to peak = 100 yrs= 722 min Time interval = 2 min Hyd. volume = 735,656 cuft Inflow hyd. No. = 12 - Post Dev Basin A to PondMax. Elevation = 1063.38 ft= Pond 1 Max. Storage Reservoir name = 324,674 cuft

Storage Indication method used.



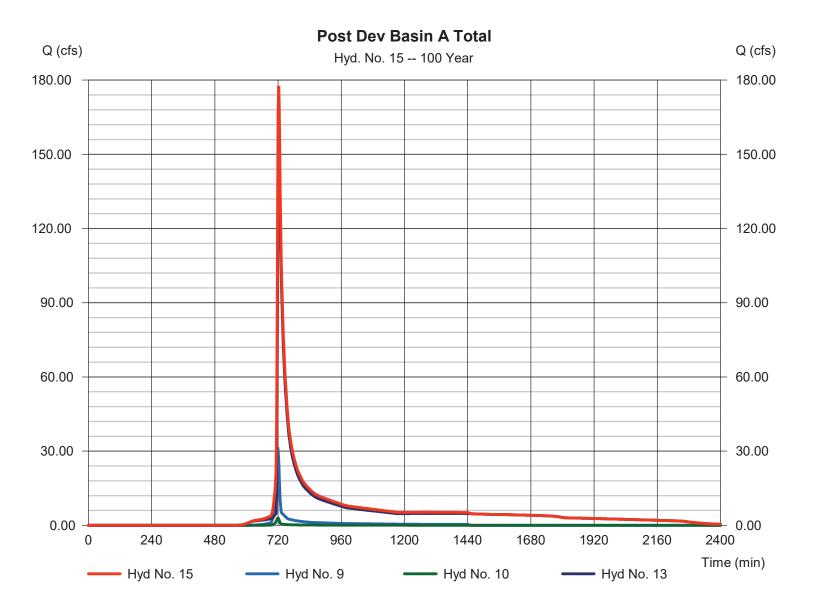
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Friday, 01 / 31 / 2025

Hyd. No. 15

Post Dev Basin A Total

Hydrograph type = Combine Peak discharge = 177.62 cfsStorm frequency Time to peak = 100 yrs= 722 min Time interval = 2 min Hyd. volume = 811,331 cuft Inflow hyds. = 9, 10, 13 Contrib. drain. area = 6.620 ac



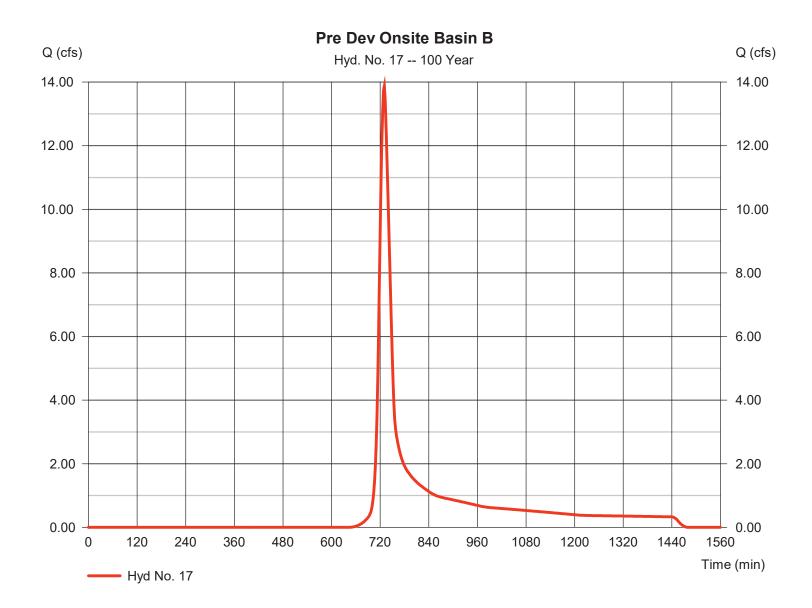
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

Friday, 01 / 31 / 2025

Hyd. No. 17

Pre Dev Onsite Basin B

Hydrograph type = SCS Runoff Peak discharge = 13.87 cfsStorm frequency = 100 yrsTime to peak = 730 min Time interval = 2 min Hyd. volume = 53.664 cuft Drainage area = 5.700 acCurve number = 55 Hydraulic length Basin Slope = 0.0 %= 0 ftTc method Time of conc. (Tc) = 25.00 min = User Total precip. = 7.78 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



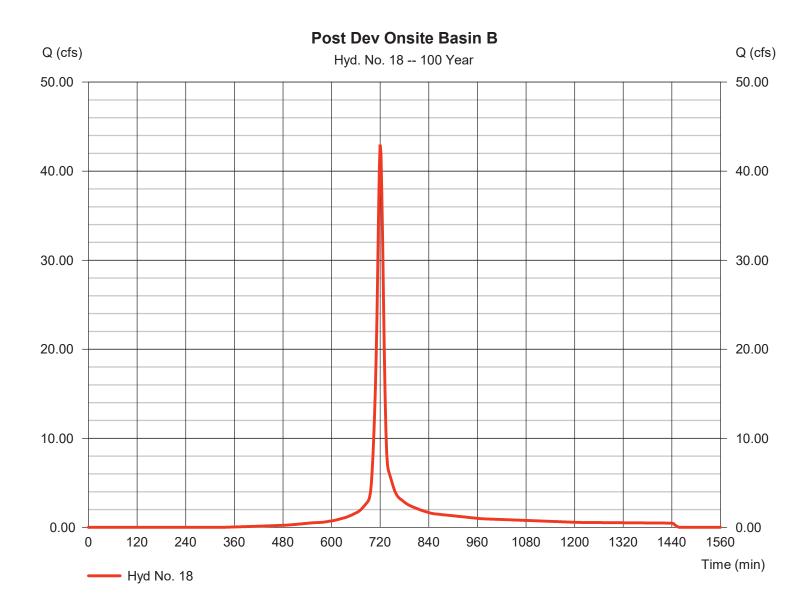
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

Friday, 01 / 31 / 2025

Hyd. No. 18

Post Dev Onsite Basin B

Hydrograph type = SCS Runoff Peak discharge = 42.99 cfsStorm frequency = 100 yrsTime to peak = 720 min Time interval = 2 min Hyd. volume = 113,162 cuft Drainage area = 5.700 acCurve number = 79 Basin Slope = 0.0 %Hydraulic length = 0 ftTc method Time of conc. (Tc) = 10.00 min = User Total precip. = 7.78 inDistribution = Type II Shape factor Storm duration = 24 hrs = 484



Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

= 24 hrs

Friday, 01 / 31 / 2025

= 484

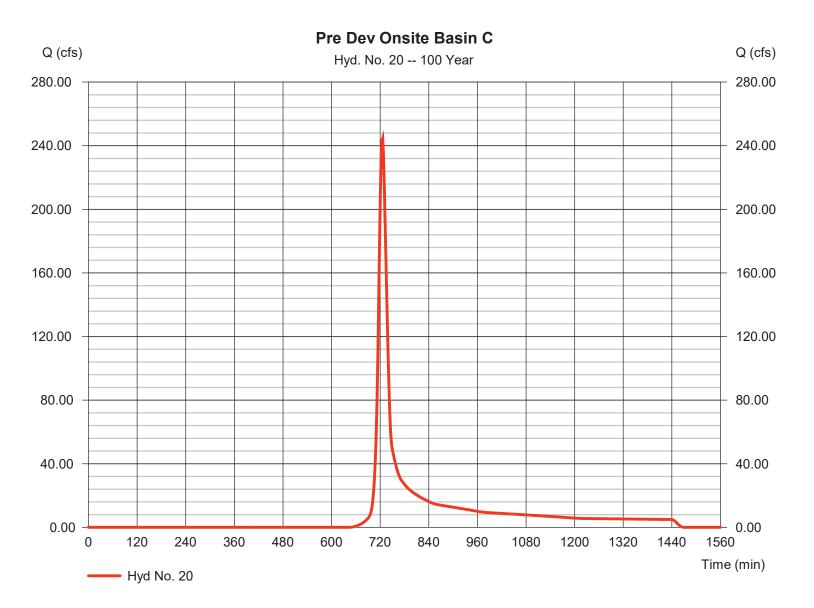
Hyd. No. 20

Storm duration

Pre Dev Onsite Basin C

Hydrograph type = SCS Runoff Peak discharge = 244.76 cfsStorm frequency = 100 yrsTime to peak = 726 min Time interval = 2 min Hyd. volume = 799,853 cuft Drainage area Curve number = 83.630 ac = 55 Hydraulic length Basin Slope = 0.0 %= 0 ftTc method Time of conc. (Tc) = 17.00 min = User Total precip. = 7.78 inDistribution = Type II

Shape factor



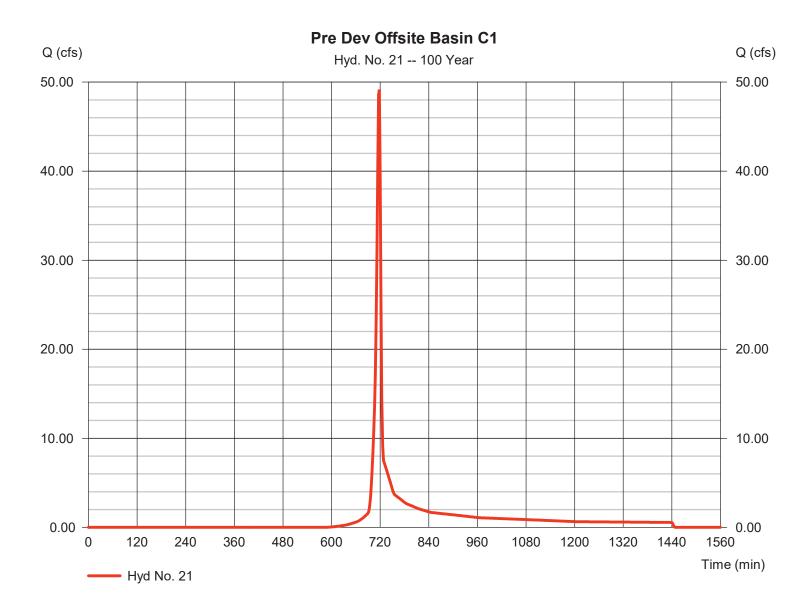
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

Friday, 01 / 31 / 2025

Hyd. No. 21

Pre Dev Offsite Basin C1

Hydrograph type = SCS Runoff Peak discharge = 49.17 cfsStorm frequency = 100 yrsTime to peak = 718 min Time interval = 2 min Hyd. volume = 98,470 cuftDrainage area = 9.130 ac Curve number = 60 Basin Slope = 0.0 %Hydraulic length = 0 ftTc method Time of conc. (Tc) = 6.00 min = User Total precip. = 7.78 inDistribution = Type II Shape factor Storm duration = 24 hrs = 484



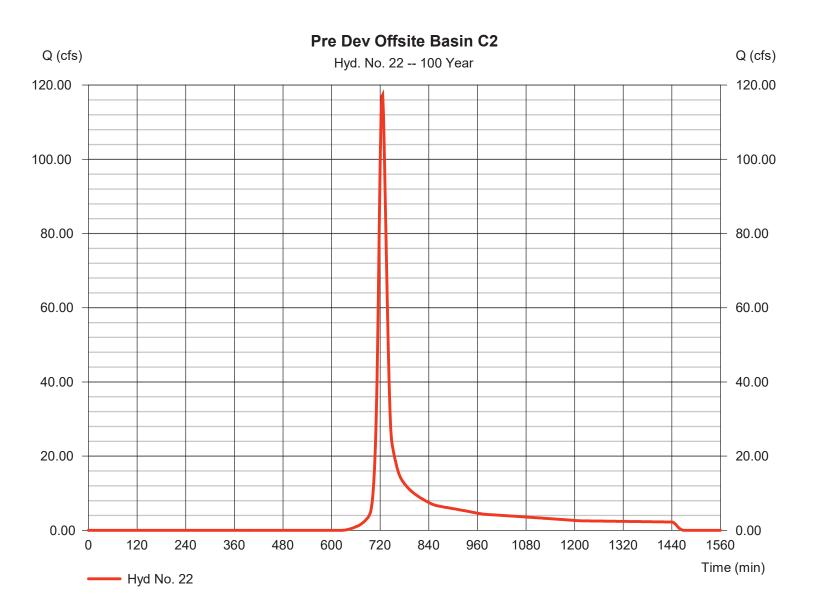
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

Friday, 01 / 31 / 2025

Hyd. No. 22

Pre Dev Offsite Basin C2

Hydrograph type = SCS Runoff Peak discharge = 117.37 cfsStorm frequency = 100 yrsTime to peak = 726 min Time interval = 2 min Hyd. volume = 379,456 cuft Drainage area = 36.720 acCurve number = 57 Basin Slope = 0.0 %Hydraulic length = 0 ftTc method Time of conc. (Tc) = 18.00 min = User Total precip. = 7.78 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

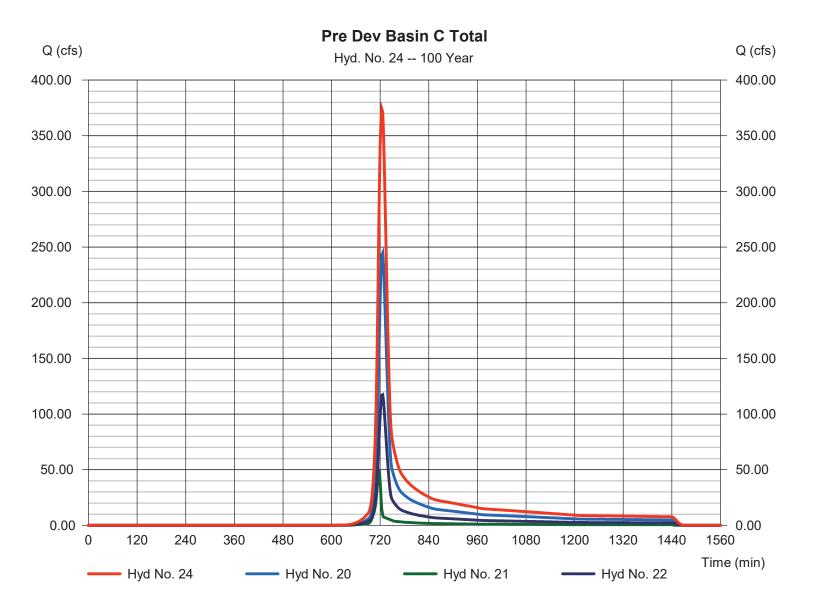
Friday, 01 / 31 / 2025

Hyd. No. 24

Pre Dev Basin C Total

Hydrograph type= CombinePeak discharge= 374.08 cfsStorm frequency= 100 yrsTime to peak= 724 minTime interval= 2 minHyd. volume= 1,277,780

Time interval = 2 min Hyd. volume = 1,277,780 cuft Contrib. drain. area = 129.480 ac



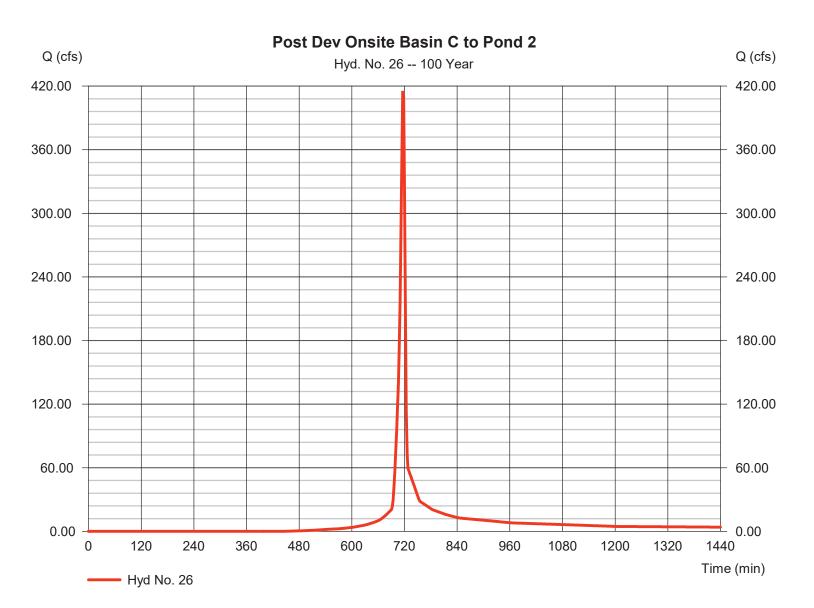
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

Friday, 01 / 31 / 2025

Hyd. No. 26

Post Dev Onsite Basin C to Pond 2

Hydrograph type = SCS Runoff Peak discharge = 415.42 cfsStorm frequency = 100 yrsTime to peak = 716 min Time interval = 2 min Hyd. volume = 843,398 cuft Drainage area = 56.470 ac Curve number = 71 Hydraulic length Basin Slope = 0.0 %= 0 ftTc method Time of conc. (Tc) $= 5.00 \, \text{min}$ = User Total precip. = 7.78 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



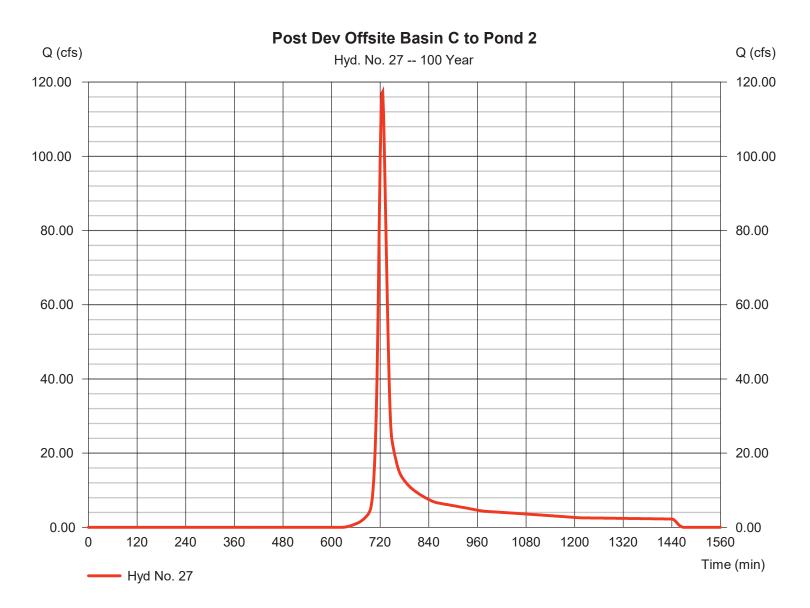
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

Friday, 01 / 31 / 2025

Hyd. No. 27

Post Dev Offsite Basin C to Pond 2

= SCS Runoff Hydrograph type Peak discharge = 117.37 cfsStorm frequency = 100 yrsTime to peak = 726 min Time interval = 2 min Hyd. volume = 379,456 cuft Drainage area = 36.720 acCurve number = 57 Hydraulic length Basin Slope = 0.0 %= 0 ftTc method Time of conc. (Tc) = 18.00 min = User Total precip. = 7.78 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



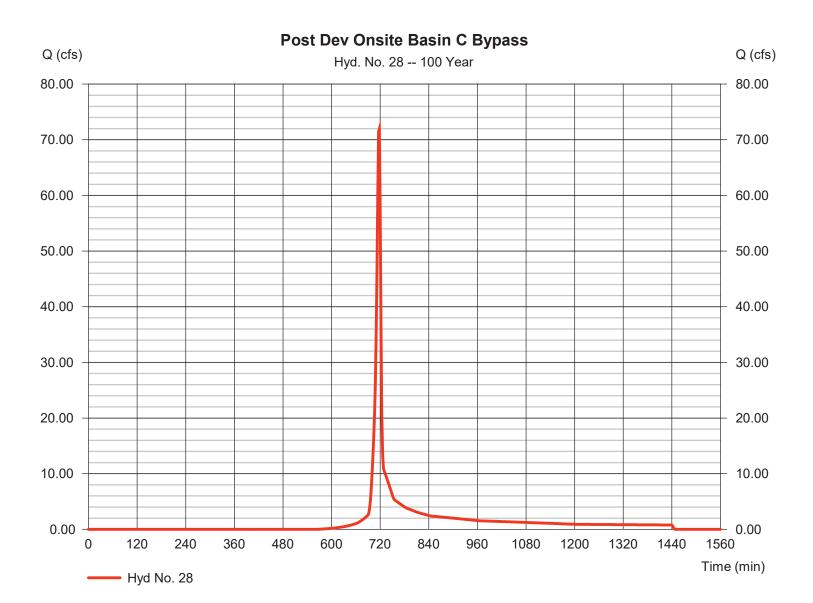
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

Friday, 01 / 31 / 2025

Hyd. No. 28

Post Dev Onsite Basin C Bypass

Hydrograph type = SCS Runoff Peak discharge = 71.92 cfsStorm frequency Time to peak = 100 yrs= 718 min Time interval = 2 min Hyd. volume = 144,423 cuft Drainage area = 12.530 ac Curve number = 62 Hydraulic length = 0 ftBasin Slope = 0.0 %Tc method Time of conc. (Tc) $= 5.00 \, \text{min}$ = User Total precip. = 7.78 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



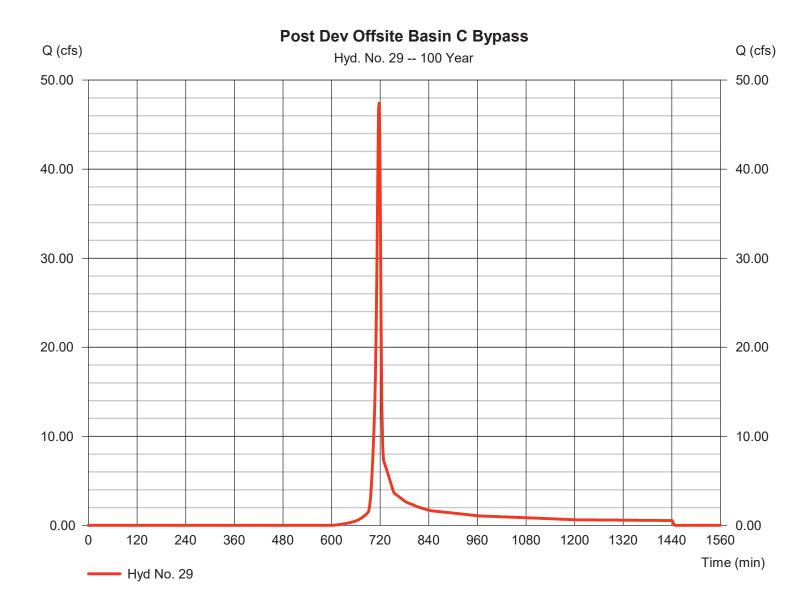
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Friday, 01 / 31 / 2025

Hyd. No. 29

Post Dev Offsite Basin C Bypass

Hydrograph type = SCS Runoff Peak discharge = 47.53 cfsStorm frequency = 100 yrsTime to peak = 718 min Time interval = 2 min Hyd. volume = 95.113 cuft Drainage area = 9.130 acCurve number = 59 Basin Slope = 0.0 %Hydraulic length = 0 ftTc method Time of conc. (Tc) = 6.00 min = User Total precip. = 7.78 inDistribution = Type II Shape factor Storm duration = 24 hrs = 484



Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

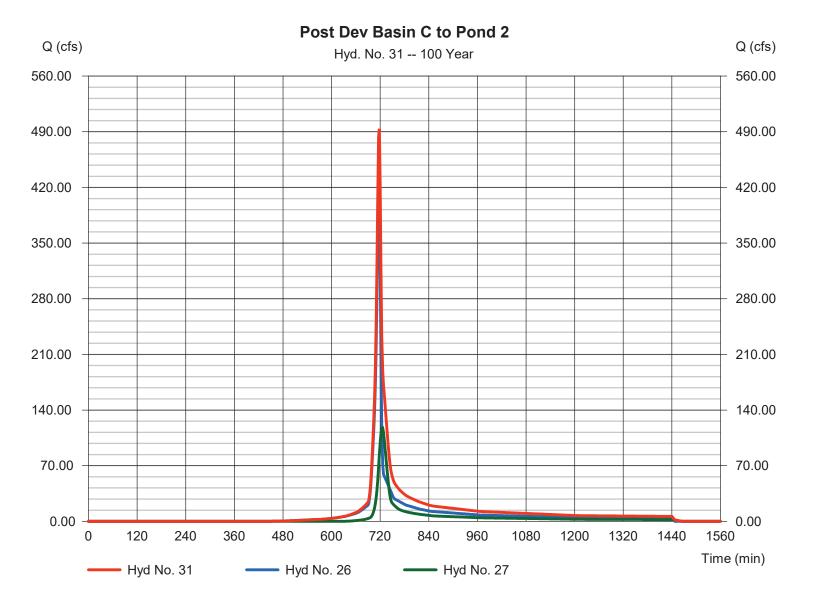
Friday, 01 / 31 / 2025

Hyd. No. 31

Post Dev Basin C to Pond 2

Hydrograph type = Combine
Storm frequency = 100 yrs
Time interval = 2 min
Inflow hyds. = 26, 27

Peak discharge = 493.70 cfs Time to peak = 718 min Hyd. volume = 1,222,854 cuft Contrib. drain. area = 93.190 ac



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Friday, 01 / 31 / 2025

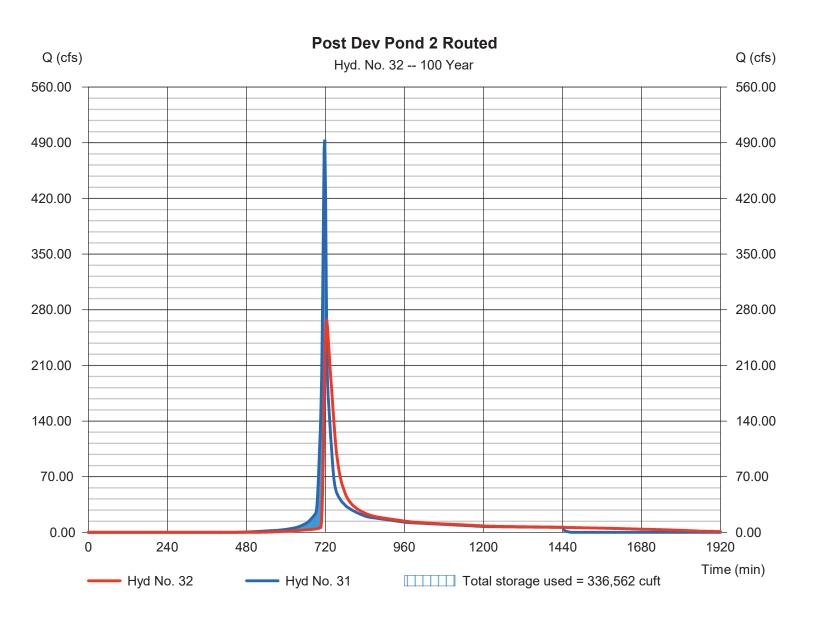
Hyd. No. 32

Post Dev Pond 2 Routed

Hydrograph type = Reservoir Peak discharge = 265.72 cfs Storm frequency = 100 yrs Time to peak = 724 min

Time interval = 2 min Hyd. volume = 1,222,834 cuft Inflow hyd. No. = 31 - Post Dev Basin C to PondMax. Elevation = 1113.19 ft Reservoir name = Pond 2 Max. Storage = 336,562 cuft

Storage Indication method used.



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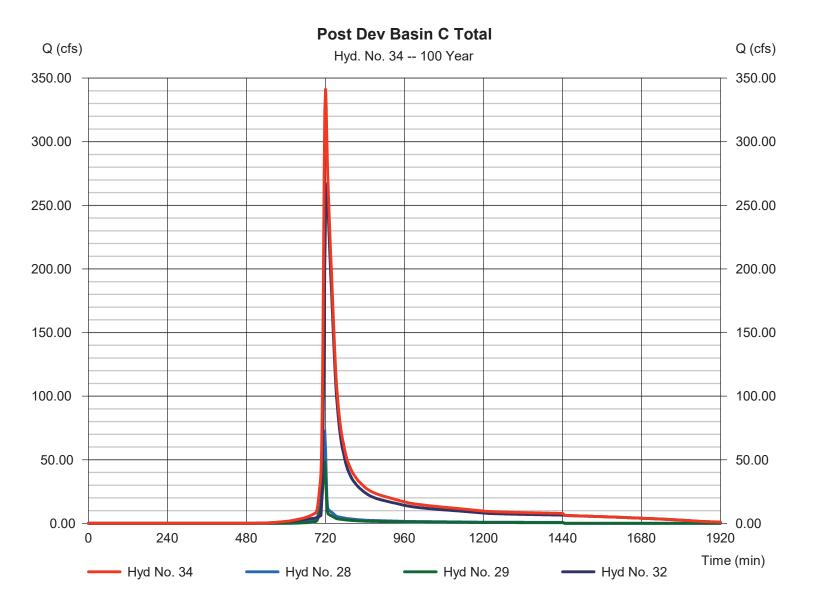
Friday, 01 / 31 / 2025

Hyd. No. 34

Post Dev Basin C Total

Hydrograph type= CombinePeak discharge= 341.86 cfsStorm frequency= 100 yrsTime to peak= 720 minTime interval= 2 minHyd. volume= 1,462,372 cuft

Inflow hyds. = 28, 29, 32 Contrib. drain. area = 21.660 ac



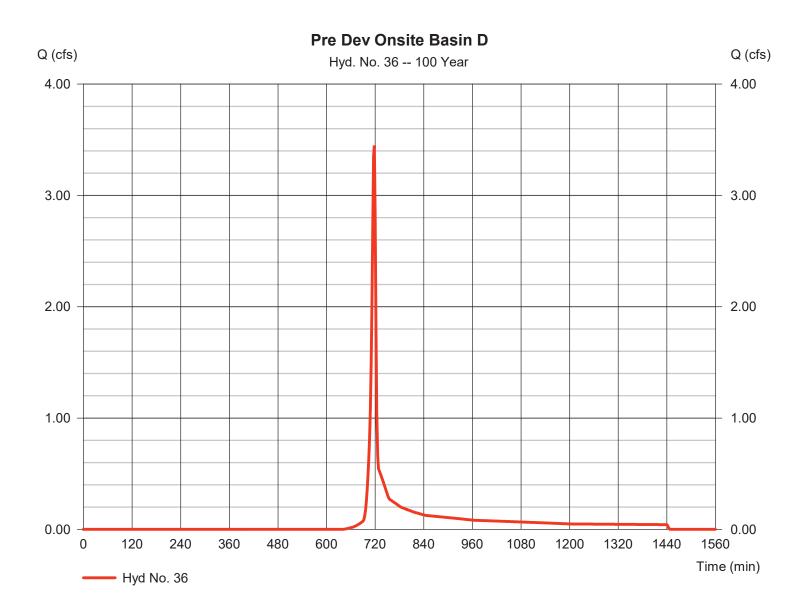
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

Friday, 01 / 31 / 2025

Hyd. No. 36

Pre Dev Onsite Basin D

Hydrograph type = SCS Runoff Peak discharge = 3.449 cfsStorm frequency = 100 yrsTime to peak = 718 min Time interval = 2 min Hyd. volume = 6.904 cuft Drainage area = 0.770 acCurve number = 55 Basin Slope = 0.0 %Hydraulic length = 0 ftTc method Time of conc. (Tc) = 5.00 min = User Total precip. = 7.78 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



Hydrograph Report

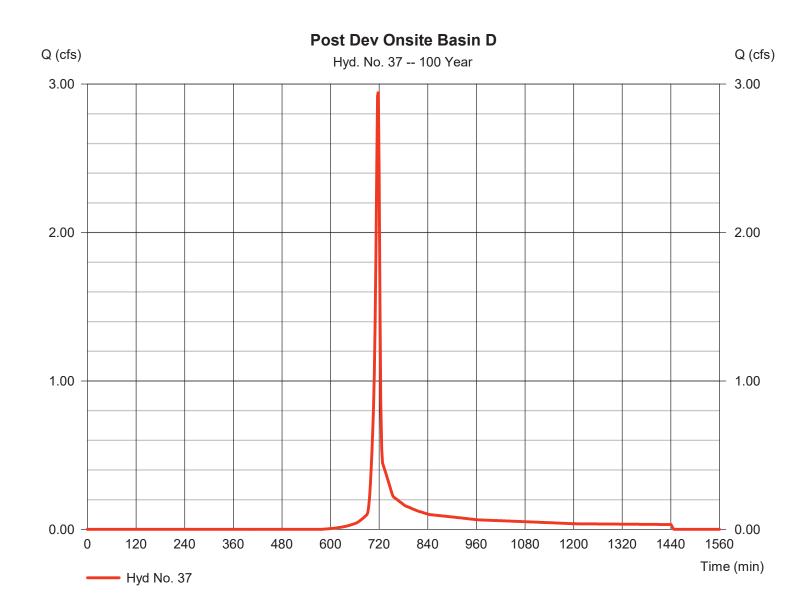
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2022

Friday, 01 / 31 / 2025

Hyd. No. 37

Post Dev Onsite Basin D

Hydrograph type = SCS Runoff Peak discharge = 2.949 cfsStorm frequency = 100 yrsTime to peak = 718 min Time interval = 2 min Hyd. volume = 5,912 cuftDrainage area Curve number = 0.530 ac= 61 Basin Slope = 0.0 %Hydraulic length = 0 ftTc method Time of conc. (Tc) $= 5.00 \, \text{min}$ = User Total precip. = 7.78 inDistribution = Type II Shape factor Storm duration = 24 hrs = 484



Appendix E

	General Information										
Name of Developer:	Audubon Development	Date Submitted:									
Development Name:	Cagle Mill	Permit Number:									
Site Location / Address:	4745 Cagle Mill Rd	Developer Contact:									
	Lula , GA 30554	Phone Number:									
		Name of Engineer(s):									
Development Type:		Maintenance Responsibility:									

Site Summary

Total Pre-Development Area (ac): 129.38
Total Post-Development Area (ac): 140.62
Total Treated Area (ac): 137.31
Total Untreated Area (ac): 3.31

		I (ac)	P (ac)	CA (ac)
Α	DB 1	31.06	39.30	0.00
В	DB 2	38.23	29.21	0.00
С	DB 3	1.52	1.30	0.00
Drainage Basin 4	DB 4	0.00	0.00	0.00
Drainage Basin 5	DB 5	0.00	0.00	0.00
Drainage Basin 6	DB 6	0.00	0.00	0.00
Drainage Basin 7	DB 7	0.00	0.00	0.00
Drainage Basin 8	DB 8	0.00	0.00	0.00
Drainage Basin 9	DB 9	0.00	0.00	0.00
Drainage Basin 10	DB 10	0.00	0.00	0.00
	TOTAL	70.81	69.81	0.00

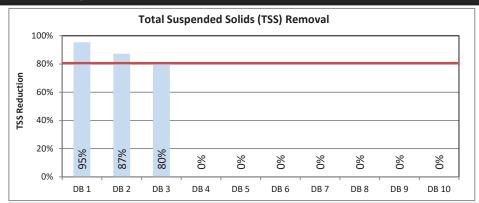
I = Impervious Area, P = Pervious Area, CA = Conservation Area

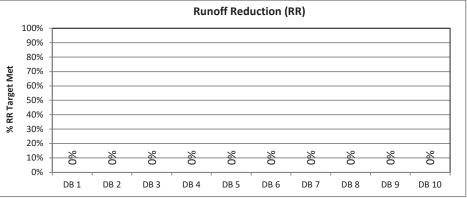
Target Runoff Reduction Volume Achieved?

Target TSS Removal Achieved?

No Yes

Total Target Runoff Reduction Volume (cf) 256,859
Runoff Reduction Volume Achieved (cf) 0
Total Target Water Quality Volume (cf) 308,231
% TSS Removal Achieved 90%





Tracking #: Conditions of Approval: Reviewed By: Date Approved: Conditions of Approval:

equivalent form of protection. above is checked easement or equivalent form of protection. 3 box above is checked easement or equivalent form of protection. 3 box above is checked Scenario 2: Site Reforestation/Revegetation *See the GSMM Volume 2, Section 4.22 and 4.23 for more information. Check the box if a portion of the post-developed area employs site reforestation/revegetation and is protected by a conservation easement or equivalent form of protection. Check the box if the same portion of the post-developed area employs site reforestation/revegetation and soil restoration, and is protected by a conservation easement or equivalent form of protection.		Storing	vater	guain	ly Oile	Devel	opine	III IVE	IGW IC	OI, V2			
Convertings Since		Dovolonment Names Cagle Mill						1			data innut calls		
Site Data Cover Type								-					
Site Data Secretary Fig. Cover Type Fig. CN Fig. Fig.		Drainage Basin Name: A											
Cover Type											constant values		
MSG Cover Type						Site Dat	a						
MSG Cover Type	ndica	te Pre-Development Land Cover and Runoff Curve Numbers in the S	ite's Disturbe	d Area									
Cover type		·					I HSG C		I HSG D		ı		1
Construction Cons		Cover Type		CN	HSG B (acres)	CN		CN		CN	Total	% Cover	
Cover Type Second condition (gass cover > 776), 99 800 91 74 90 90 90 90 90 90 90 9		Woods - Good Condition	(,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,	30	53.31	55	(,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,	70	(******)	77	53.31	100%	1
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HSG = hydrologic soil group Total 0 00 53.31 0.00 0.00 10.00				0		0		0		0			-
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Higher Hydrologic soil group Potential Max So Retention, Syn. (n) 0.00 0.55 0.00 0			0.00		53 31		0.00		0.00				1
Potential Max Soil Retention, S _{pre} (in) 53.18 Indicate Post-Development Land Cover and Runoff Curve Numbers in the Site's Disturbed Area Cover Type	HSC -		0.00		33.31		0.00			0.00	33.31	10070	1
Potential Max Soil Retention, S _{ave} (in) 1836 Cover Type	1100 -	Trydrologic soli group									1		
Cover Type RISG A CN HSG C CN HSG D CN Cover Type RISG A CN HSG D CN Cover Type RISG A CN HSG D CN Cover Type RISG A CN HSG D CN Cover Type CN RISG D CN Cover Type CN CN CN CN CN CN CN C							P	otential Max Soil			1		
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Cover type Cover type Cove	ndica	te Post-Development Land Cover and Runoff Curve Numbers in the	Site's Disturt	ed Area									_
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Company page - Good Condition (grass cover > 75%) 39 20.00 61 74 80 20.00 28%		Cover Type	(acres)		` ′		(acres)		(acres)				
Mode - Good Condition 30 19.30 55 70 77 19.30 27%													
Impervious 98 6.50 98 98 98 98 98 6.50 99													
Conservation Area Credits Conservation Area Credits Scenario 1: Natural Conservation of the post-developed area is protected by a conservation easement or equivalent form of protection. Area (ac) of development reforested/revegetation aseement or equivalent form of protection. Scenario 2: Site Reforestation/Revegetation aseement or equivalent form of protection. Area (ac) of development reforested/revegetated and protected by a conservation easement or equivalent form of protection. Area (ac) of development reforested/revegetated and protected by a conservation easement or equivalent form of protection. Area (ac) of development reforested/revegetated and protected by a conservation easement or equivalent form of protection. Area (ac) of development reforested/revegetated and protected by a conservation easement or equivalent form of protection. Area (ac) of development reforested/revegetation aseement or equivalent form of protection. Area (ac) of development reforested/revegetation aseement or equivalent form of protection. Area (ac) of development reforested/revegetation aseement or equivalent form of protection. Area (ac) of development reforested/revegetation and is protected by a conservation easement or equivalent form of protection. Area (ac) of development reforested/revegetation aseement or equivalent form of protection. Area (ac) of development reforested/revegetation and is protected by a conservation easement or equivalent form of protection. Area (ac) of development reforested/revegetation and protected by a conservation easement or equivalent form of protection. Area (ac) of development reforested/revegetated and protected by a conservation easement or equivalent form of protection. Area (ac) of development reforested/revegetated and protected by a conservation easement or equivalent form of protection. Area (ac) of development reforested/revegetated and protected by a conservation easement or equivalent form of protection. Area (ac) of development reforested													4
Local Jurisdiction Input Other													-
Conservation Area Credits Conservation Area Credits Conservation Area Conservation Conservat				39	0.00	01		74		60			1
Conservation Area Credits Conservation Area See the GSMM Volume 2, Section 2.3.3.3 for more information.													1
Conservation Area Credits Conservation Area Credits			0.00		70.36		0.00		0.00				1
Conservation Area Credits Conservation Area 'See the GSMM Volume 2, Section 2.3.3.3 for more information. Check the box if a portion of the post-developed area is protected by a conservation easement or equivalent form of protection. Area (ac) of development protected by a conservation easement or equivalent form of protection. Check the box if a portion of the post-developed area employs soil restoration and is protected by a conservation easement or equivalent form of protection. Note: The green cell will unlock if the Scenario 1 box above is checked Scenario 2: Site Reforestation/Revegetation 'See the GSMM Volume 2, Section 4.23 for more information. Check the box if a portion of the post-developed area employs soil restoration and is protected by a conservation easement or equivalent form of protection. Note: The green cell will unlock if the Scenario 1 box above is checked Scenario 2: Site Reforestation/Revegetation 'See the GSMM Volume 2, Section 4.22 for more information. Check the box if a portion of the post-developed area employs soil restoration and protected by a conservation easement or equivalent form of protection. Scenario 4: Site Reforestation/Revegetation & Soil Restoration more information. Check the box if the same portion of the post-developed area employs site reforestation/revegetation and soil restoration, and is protected by a conservation easement or equivalent form of protection. Note: The green cell will unlock if the Scenario 2 box above is checked Note: The green cell will unlock if the Scenario 2 box above is checked Note: The green cell will unlock if the Scenario 2 box above is checked Note: The green cell will unlock if the Scenario 2 box above is checked Note: The green cell will unlock if the Scenario 2 box above is checked Note: The green cell will unlock if the Scenario 2 box above is checked Note: The green cell will unlock if the Scenario 2 box above is checked					•		•		Impervious (ac)	31.06			-
Conservation Area Credits Conservation Area See the GSMM Volume 2, Section 2.3.3.3 for more information. Scenario 3: Soil Restoration *See the GSMM Volume 2, Section 4.23 for more information. Check the box if a portion of the post-developed area is protected by a conservation easement or equivalent form of protection. Area (ac) of development protected by a conservation easement or equivalent form of protection. Area (ac) of development with restored soils and protected by a conservation Note: The green cell will unlock if the Scenario 2: Site Reforestation/Revegetation *See the GSMM Volume 2, Section 4.22 for more information. Scenario 2: Site Reforestation/Revegetation *See the GSMM Volume 2, Section 4.22 for more information. Scenario 4: Site Reforestation/Revegetation & Soil Restoration See the GSMM Volume 2, Section 4.22 for more information. Scenario 4: Site Reforestation/Revegetation & Soil Restoration See the GSMM Volume 2, Section 4.22 and 4.23 for more information. Check the box if a portion of the post-developed area employs site reforestation/revegetation and is protected by a conservation easement or equivalent form of protection. See the GSMM Volume 2, Section 4.22 and 4.23 for more information. Check the box if the same portion of the post-developed area employs site reforestation/revegetation and is protected by a conservation easement or equivalent form of protection. Area (ac) of development reforested/revegetated and protected by a conservation easement or equivalent form of protection. Area (ac) with restored soils in a reforested & revegetated area and protected by a conservation easement or equivalent form of protection. Area (ac) with restored soils in a reforested & revegetated area and protected Abox above is checked Area (ac) with restored soils in a reforested & revegetated area and protected Abox above is checked Abox abov									Rv	0.45	1		
Conservation Area Credits Check the box if a portion of the post-developed area is protected by a conservation easement or equivalent form of protection. Check the box if a portion of the post-developed area is protected by a conservation easement or equivalent form of protection. Area (ac) of development protected by a conservation easement or equivalent form of protection. Check the box if a portion of the post-developed area employs soil restoration and is protected by a conservation easement or equivalent form of protection. Area (ac) of development with restored soils and protected by a conservation easement or equivalent form of protection. Scenario 2: Site Reforestation/Revegetation *See the GSMM Volume 2, Section 4.22 for more information. Check the box if a portion of the post-developed area employs soil restoration above is checked Scenario 4: Site Reforestation/Revegetation & Soil Restoration													
Check the box if a portion of the post-developed area is protected by a conservation easement or equivalent form of protection. Area (ac) of development protected by a conservation easement or equivalent form of protection. Check the box if a portion of the post-developed area employs soil restoration and is protected by a conservation easement or equivalent form of protection. Note: The green cell will unlock if the Scenario 1 box above is checked Scenario 2: Site Reforestation/Revegetation *See the GSMM Volume 2, Section 4.22 for more information. Check the box if a portion of the post-developed area employs site reforestation/revegetation and is protected by a conservation easement or equivalent form of protection. Scenario 2: Site Reforestation/Revegetation *See the GSMM Volume 2, Section 4.22 for more information. Check the box if a portion of the post-developed area employs site reforestation/revegetation and is protected by a conservation easement or equivalent form of protection. Scenario 4: Site Reforestation/Revegetation & Soil Restoration *See the GSMM Volume 2, Section 4.22 and 4.23 for more information. Check the box if a portion of the post-developed area employs site reforestation/revegetation and is protected by a conservation easement or equivalent form of protection. Area (ac) of development reforestation of the post-developed area employs site reforestation/revegetation and is protected by a conservation easement or equivalent form of protection. Area (ac) of development reforested/revegetated and protected by a conservation easement or equivalent form of protection. Area (ac) of development reforested/revegetated and protected by a conservation easement or equivalent form of protection. Area (ac) of development in the post-developed area employs site reforestation/revegetation and is protected by a conservation easement or equivalent form of protection. Area (ac) of development in the form of protection. Area (ac) of development with restored soils in a reforested & revegetate							Po	tential Max Soil I	Retention, S _{post} (in)	3.21	1		
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Development Name: Cagle Mill	data in
Drainage Basin Name: A	calcula
	consta

Water Quality Goals

Target Runoff Reduction Storm (in) 1.00

Total Site Area for Water Quality Volume (acres)	70.36
Target Runoff Reduction Volume (cf)	114,243
Target Water Quality Volume (cf)	137,092

Select BMPs for Runoff Reduction and Water Quality

		Area E	raining to Each	n BMP	Storage Volume	RR Conveyance			Ru	noff Reduction	Calculations			WQ Cald	ulations
		On-site Pervious Area (acres)	On-site Impervious Area (acres)	Offsite Area (acres)	Provided by BMP (cf)	rovided by BMP Provided by BMP		RR Volume from Direct Drainage (cf)	RR Volume from Upstream Practices (cf)	Total RR Volume Received by BMP (cf)	Runoff Reduction %	RR Achieved (cf)	Remaining RR Volume (cf)	WQ _v from Direct Drainage (cf)	Effective TSS Removal %
BMP 1	Stormwater Pond	20.00	24.56		70,000		BMP 2	88,325	0	88,325	0%	0	88,325	105,990	80%
BMP 2	Stormwater Wetlands – Level 1		0.00			88,325		0	88,325	88,325	0%	0	88,325	0	80%
BMP 3	Bioretention Basin (w/o underdrain)		2.50					8,621	0	8,621	100%	0	8,621	10,346	100%
BMP 4	Bioretention Basin (w/o underdrain)		2.50					8,621	0	8,621	100%	0	8,621	10,346	100%
BMP 5	Regenerative Stormwater Conveyance	19.30	1.50					8,676	0	8,676	0%	0	8,676	10,411	80%
BMP 6	Select a BMP		0.00					0	0	0	N/A	0	0	0	N/A
BMP 7	Select a BMP							0	0	0	N/A	0	0	0	N/A
BMP 8	Select a BMP							0	0	0	N/A	0	0	0	N/A
BMP 9	Select a BMP							0	0	0	N/A	0	0	0	N/A
BMP 10	Select a BMP							0	0	0	N/A	0	0	0	N/A
	TOTAL UNTREATED AREA (acres)	39.30 0.00	31.06 0.00	0.00				114,243				0		137,092	

Target Runoff Reduction Volume (cf)	114,243
Target Achieved?	No
Remaining Runoff Reduction Volume (cf)	114,243

Target Water Quality Volume (cf)	137,092
% TSS Removal Achieved	95%
Target Achieved?	Yes!
Remaining TSS Removal %	0%

Georgia Stormwater Management Manual Stormwater Quality Site Development Review Tool, v2.2 Development Name: Cagle Mill data input cells Drainage Basin Name: A calculation cells constant values **Channel and Flood Protection Calculations** 1-yr, 24-hr 2-yr, 24-hr 25-yr, 24-hr 100-yr, 24-hr storm storm storm storm Target Rainfall Event (in) 2-yr, 24-hr storm 0.00 0.00 1-yr, 24-hr storm 25-yr, 24-hr storm 100-yr, 24-hr storm 0.00 0.00 0.00 Pre-Development Runoff Volume (in) Post Development Runoff Volume (in) with no BMPs Post-Development Runoff Volume (in) with BMPs 0.00 0.00 0.00 0.00 *See Stormwater Management Standards to Determine Detention Requirements. Comments

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											F	
Total Conservation Area Credit (acres) 0.00	201125. Talien depondent of equivalent form of protocion.						,			,		
Total Conservation Area Credit (acres) 0.00												
		Total Co	nservation Ar	ea Credit (acres)	0.00							

	 _
Development Name: Cagle Mill	data input cells
Drainage Basin Name: B	calculation cell
	constant value

Water Quality Goals

Target Runoff Reduction Storm (in) 1.00

Total Site Area for Water Quality Volume (acres) 67.44

Target Runoff Reduction Volume (cf) 137,138

Target Water Quality Volume (cf) 164,565

Select BMPs for Runoff Reduction and Water Quality

		Area [Draining to Each	1 ВМР	Storage Volume	RR Conveyance			Ru	noff Reduction	Calculations			WQ Cald	ulations
		On-site Pervious Area (acres)	On-site Impervious Area (acres)	Offsite Area (acres)	Provided by BMP (cf)		Down-stream BMP	RR Volume from Direct Drainage (cf)	RR Volume from Upstream Practices (cf)	Total RR Volume Received by BMP (cf)	Runoff Reduction %	RR Achieved (cf)	Remaining RR Volume (cf)	WQ _v from Direct Drainage (cf)	Effective TSS Removal %
BMP 1	Stormwater Pond	16.00	38.23	13.21				134,740	0	134,740	0%	0	134,740	161,688	80%
BMP 2	Regenerative Stormwater Conveyance	6.15	3.75					14,048	0	14,048	0%	0	14,048	16,858	80%
BMP 3	Select a BMP							0	0	0	N/A	0	0	0	N/A
BMP 4	Select a BMP							0	0	0	N/A	0	0	0	N/A
BMP 5	Select a BMP							0	0	0	N/A	0	0	0	N/A
BMP 6	Select a BMP							0	0	0	N/A	0	0	0	N/A
BMP 7	Select a BMP							0	0	0	N/A	0	0	0	N/A
BMP 8	Select a BMP							0	0	0	N/A	0	0	0	N/A
BMP 9	Select a BMP							0	0	0	N/A	0	0	0	N/A
BMP 10	Select a BMP							0	0	0	N/A	0	0	0	N/A
	TOTAL UNTREATED AREA (acres)	22.15 7.06	41.98 0.00	13.21				148,788				0		178,546	

137,138	Target Runoff Reduction Volume (cf)
No	Target Achieved?
137,138	Remaining Runoff Reduction Volume (cf)

Target Water Quality Volume (cf)	164,565
% TSS Removal Achieved	87%
Target Achieved?	Yes!
Remaining TSS Removal %	0%

Georgia Stormwater Management Manual Stormwater Quality Site Development Review Tool, v2.2 Development Name: Cagle Mill data input cells Drainage Basin Name: B calculation cells constant values **Channel and Flood Protection Calculations** 1-yr, 24-hr 2-yr, 24-hr 25-yr, 24-hr 100-yr, 24-hr storm storm storm storm Target Rainfall Event (in) 2-yr, 24-hr storm 0.00 0.00 1-yr, 24-hr storm 25-yr, 24-hr storm 100-yr, 24-hr storm 0.00 0.00 0.00 Pre-Development Runoff Volume (in) Post Development Runoff Volume (in) with no BMPs Post-Development Runoff Volume (in) with BMPs 0.00 0.00 0.00 0.00 *See Stormwater Management Standards to Determine Detention Requirements. Comments

Storing	valer	Luaiii	ly Site	Devei	opme	III Kev	riew i c	101, VZ	2		
Davidanment Name Carle Mill									data tanak anti-		<u> </u>
Development Name: Cagle Mill		data input cells									
Drainage Basin Name: C		calculation cells									
				·					constant values		
				Site Data	a						
Indicate Pre-Development Land Cover and Runoff Curve Numbers in the S	ite's Disturbe	d Area									
·	HSG* A				HSG C		HSG D			~ -	1
Cover Type	(acres)	CN	HSG B (acres)	CN	(acres)	CN	(acres)	CN	Total	% Cover	
Woods - Good Condition		30	6.07	55		70		77	6.07	100%	
Select a land cover type		0		0		0		0	0.00	0%	
Select a land cover type Select a land cover type		0		0		0		0	0.00	0% 0%	-
Select a land cover type		0		0		0		0	0.00	0%	1
Local Jurisdiction Input									0.00	0%	
Other Total	0.00		6.07		0.00		0.00		0.00 6.07	0% 100%	-
*HSG = hydrologic soil group	0.00		0.07		0.00		Impervious (ac)	0.00	6.07	100%	1
Tioo - Hydrologic soli group							Weighted CN	55	1		
					Po	otential Max Soil	Retention, Spre (in)	8.18	1		
Indicate Post-Development Land Cover and Runoff Curve Numbers in the	Sito's Disturb	od Area									
	HSG A				HSG C		HSG D				1
Cover Type	(acres)	CN	HSG B (acres)	CN	(acres)	CN	(acres)	CN	Total	% Cover	
Impervious	(33122)	98	1.52	98	(33133)	98	(3.3.2.5)	98	1.52	54%	
Open space - Fair condition (grass cover 50% to 75%)		49	1.30	69		79		84	1.30	46%	
Select a land cover type		0		0		0		0	0.00	0% 0%	
Select a land cover type Select a land cover type		0		0		0		0	0.00	0%	-
Local Jurisdiction Input		Ü		- J		Ü		Ü	0.00	0%	1
Other									0.00	0%	
Total	0.00		2.82		0.00		0.00 Impervious (ac)	1.52	2.82	100%	J
							Impervious (ac)	0.54	1		
							Weighted CN	85	1		
		Po	tential Max Soil R	Retention, S _{post} (in)	1.82	1					
			Conser	vation Are	ea Credits						
					1						
Scenario 1: Natural Conservation Area *See the GSMM Volume 2, Section 2.3.3.3 for	or more informa	tion.			Scenario 3: S	Soil Restoration	*See the GSMM	Volume 2, Sec	tion 4.23 for mo	re information.	
Check the box if a portion of the post-developed area is protected by a conse	ryation oacom	ont or oquivalo	nt form of		_	Chack the hey if	f a partian of the p	act davalanad	aroa omplove e	oil roctoration a	nd is protected by a
protection.	i valion cascino	sint or equivale	iit ioiiii oi				sement or equival			oli restoration a	ind is protected by a
F							·	•			
Area (ac) of development protected by a conservation easement or			f the Scenario 1 box						protected by a	conservation	Note: The green cell will unlock if the Scenario
equivalent form of protection.	above is checked	i				easement or equ	uivalent form of pro	otection.			3 box above is checked
					-+					*See the GSM	M Volume 2, Section 4.22 and 4.23 for
Scenario 2: Site Reforestation/Revegetation *See the GSMM Volume 2, Section 4.2	2 for more info	mation.			Scenario 4: S	Site Reforestation	on/Revegetation &	& Soil Restora	tion	more informati	
☐ Check the box if a portion of the post-developed area employs site reforestate	ion/revegetation	<u>n</u> and is protec	ted by a								restation/revegetation and
conservation easement or equivalent form of protection.						soil restoration,	and is protected by	y a conservation	on easement or	equivalent form	of protection.
Area (as) of dayolanment referented/revegetated and protected by a	Note: The green	cell will unlock it	f the Scenario 2 box			Area (ac) with re	estored soils in a re	eforested & rev	enetated area :	and protected	Note: The green cell will unlock if the Scenario
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2 addition of addition of protocolors						,			•		
	Total Co	nconvotion A-	ea Credit (acres)	0.00							
	lotal Co	nservation Ar	ea Gredit (acres)	0.00							

Development Name: Cagle Mill	data in
Drainage Basin Name: C	calcula
	constar

Water Quality Goals

Target Runoff Reduction Storm (in) 1.00 Total Site Area for Water Quality Volume (acres)

tal Site Area for Water Quality Volume (acres)

Target Runoff Reduction Volume (cf)

Target Water Quality Volume (cf)

6,573

Select BMPs for Runoff Reduction and Water Quality

		Starana Valuma	RR Conveyance		Runoff Reduction Calculations						WQ Calculations				
		On-site Pervious Area (acres)	On-site Impervious Area (acres)	Offsite Area (acres)	Storage Volume Provided by BMP (cf)		Down-stream BMP	RR Volume from Direct Drainage (cf)	RR Volume from Upstream Practices (cf)	Total RR Volume Received by BMP (cf)	Runoff Reduction %	RR Achieved (cf)	Remaining RR Volume (cf)	WQ _v from Direct Drainage (cf)	Effective TSS Removal %
BMP 1	Stormwater Wetlands – Level 1	1.30	1.52			5,478		5,478	0	5,478	0%	0	5,478	6,573	80%
BMP 2	Select a BMP							0	0	0	N/A	0	0	0	N/A
BMP 3	Select a BMP							0	0	0	N/A	0	0	0	N/A
BMP 4	Select a BMP							0	0	0	N/A	0	0	0	N/A
BMP 5	Select a BMP							0	0	0	N/A	0	0	0	N/A
BMP 6	Select a BMP							0	0	0	N/A	0	0	0	N/A
BMP 7	Select a BMP							0	0	0	N/A	0	0	0	N/A
BMP 8	Select a BMP							0	0	0	N/A	0	0	0	N/A
BMP 9	Select a BMP							0	0	0	N/A	0	0	0	N/A
BMP 10	Select a BMP							0	0	0	N/A	0	0	0	N/A
	TOTAL UNTREATED AREA (acres)	1.30 0.00	1.52 0.00	0.00				5,478				0		6,573	

Target Runoff Reduction Volume (cf)	5,478
Target Achieved?	No
Remaining Runoff Reduction Volume (cf)	5,478

Target Water Quality Volume (cf)	6,573
% TSS Removal Achieved	80%
Target Achieved?	Yes!
Remaining TSS Removal %	0%

Georgia Stormwater Management Manual Stormwater Quality Site Development Review Tool, v2.2 Development Name: Cagle Mill data input cells Drainage Basin Name: C calculation cells constant values **Channel and Flood Protection Calculations** 1-yr, 24-hr 2-yr, 24-hr 25-yr, 24-hr 100-yr, 24-hr storm storm storm storm Target Rainfall Event (in) 2-yr, 24-hr storm 0.00 0.00 1-yr, 24-hr storm 25-yr, 24-hr storm 100-yr, 24-hr storm 0.00 0.00 0.00 Pre-Development Runoff Volume (in) Post Development Runoff Volume (in) with no BMPs Post-Development Runoff Volume (in) with BMPs 0.00 0.00 0.00 0.00 *See Stormwater Management Standards to Determine Detention Requirements. Comments